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AASHTO Guide for the Planning, Design, and Operation of Bicycle Facilities



For Review and Comment by:

Subcommittee on Design
Subcommittee on Traffic Engineering
Technical Committee on Geometric Design
Technical Committee on Nonmotorized Transportation

**American Association of State Highway
and Transportation Officials**

February 2010

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CHAPTER 1: INTRODUCTION**1.1. DESIGN IMPERATIVE**

Bicycle travel has played an historic role in transportation. Even before the invention of the automobile, the League of American Wheelmen promoted improved traveled ways.

Bicycling is recognized by transportation officials throughout the United States as an important transportation mode. Over a quarter of the population in the U.S. over the age of 16 rides bicycles. (1) Nationwide, people are recognizing the convenience, energy efficiency, cost effectiveness, health benefits and environmental advantages of bicycling.

Local, state and federal agencies are responding to the increased use of bicycles by implementing a wide variety of bicycle-related projects and programs. The emphasis now being placed on bicycle transportation requires an understanding of bicycles, bicyclists and bicycle facilities. This guide addresses these issues and clarifies the elements needed to make bicycling an accessible mode of transportation.

All roads, streets and highways, except those where cyclists are legally prohibited, should be designed and constructed under the assumption that they will be used by bicyclists. (2) Therefore, bicycles should be considered in all phases of transportation planning, new roadway design, roadway reconstruction, operational and maintenance activities, capacity improvement, bridge and transit projects.

1.2. PURPOSE

Bicyclists can be expected to ride on almost all roadways, as well as on shared use paths, where permitted. Safe, convenient, well-designed and well-maintained facilities are essential to accommodate and encourage bicycling.

This guide provides information on how to accommodate bicycle travel and operations in most riding environments. It is intended to present sound guidelines that result in facilities that meet the needs of bicyclists and other highway users. Sufficient flexibility is permitted to encourage designs that are sensitive to local context and incorporate the needs of bicyclists, pedestrians and motorists. However, in some sections of this guide, suggested minimum dimensions are provided. These are recommended only where further deviation from desirable values could compromise safety.

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1.3. SCOPE

This guide provides information on the physical infrastructure needed to support bicycling. Facilities are only one of several elements essential to a community's overall bicycle program. Bicycle safety education and training, encouraging bicycle use, and enforcing the rules of the road as they pertain to bicyclists and motorists should be combined with engineering measures to form a comprehensive approach to bicycle use. Information on other elements of an overall bicycle program can be obtained from state or local bicycle coordinators and other publications.

The provisions for bicycle travel are consistent with, and similar to, normal highway engineering practices. Signs, signals and pavement markings for bicycle facilities are presented in the Manual on Uniform Traffic Control Devices (3), which should be used in conjunction with this guide. For construction of bicycle facilities, applicable state and local construction specifications should be used.

1.4. DEFINITIONS

BICYCLE - A pedal-powered vehicle upon which the human operator sits. The term "bicycle" for this publication includes three and four-wheeled human-powered vehicles, but not tricycles for children.

BICYCLE BOULEVARD - A street segment, or series of contiguous street segments, that has been modified to accommodate through bicycle traffic but discourage through motor traffic.

BICYCLE FACILITIES - A general term denoting improvements and provisions to accommodate or encourage bicycling, including parking and storage facilities, and shared roadways specifically designated for bicycle use.

BICYCLE LANE or BIKE LANE - A portion of a roadway which has been designated by pavement markings and, if used, signs, for the preferential or exclusive use of bicyclists.

BICYCLE LEVEL OF SERVICE (BLOS) – A model used to estimate bicyclists' average perception of the quality of service of a section of roadway between two intersections.

BICYCLE LOCKER or BIKE LOCKER – A secure, lockable container used for long-term individual bicycle storage.

BICYCLE PATH or BIKE PATH – A pathway that is exclusively used by bicyclists, where a separate, parallel path is provided for pedestrians and other wheeled users. Most pathways are shared between bicyclists and other users: see Shared Use Path.

BICYCLE RACK or BIKE RACK - A stationary fixture to which a bicycle can be securely attached.

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- 1 BICYCLE ROUTE— A roadway or bikeway designated by the jurisdiction having authority, either with a
2 unique route designation or with BIKE ROUTE signs, along which bicycle guide signs may provide
3 directional and distance information. Signs that provide directional, distance, and destination
4 information for cyclists do not necessarily establish a bicycle route.
- 5 BICYCLE NETWORK - A system of bikeways designated by the jurisdiction having authority. This system
6 may include bike lanes, bicycle routes, shared use paths, and other identifiable bicycle facilities.
- 7 BIKEWAY - A generic term for any road, street, path or way which in some manner is specifically
8 designated for bicycle travel, regardless of whether such facilities are designated for the exclusive use of
9 bicycles or are to be shared with other transportation modes.
- 10 HIGHWAY - A general term denoting a public way for purposes of vehicular travel, including the entire
11 area within the right-of-way.
- 12 RAIL-TRAIL - A shared use path, either paved or unpaved, built within the right-of-way of a former
13 railroad.
- 14 RAIL-WITH-TRAIL - A shared use path, either paved or unpaved, built within the right-of-way of an active
15 railroad.
- 16 RIGHT-OF-WAY - A general term denoting land, property or interest therein, usually in a strip, acquired
17 for or devoted to transportation purposes.
- 18 RIGHT OF WAY (ASSIGNMENT) - The right of one vehicle or pedestrian to proceed in a lawful manner in
19 preference to another vehicle or pedestrian.
- 20 ROADWAY - The portion of the highway, including shoulders, intended for vehicular use.
- 21 RECUMBENT BICYCLE - A bicycle with pedals at roughly the same level as the seat where the operator is
22 seated in a reclined position with their back supported.
- 23 RUMBLE STRIPS - A textured or grooved pavement treatment designed to create noise and vibration to
24 alert motorists of a hazard. Longitudinal rumble strips are sometimes used on or along shoulders or
25 center lines of highways to alert motorists who stray from the appropriate traveled way. Transverse
26 rumble strips are placed on the roadway surface in the travel lane, perpendicular to the direction of
27 travel.
- 28 SHARED LANE - A lane of a traveled way that is open to bicycle travel and vehicular use.
- 29 SHARED LANE MARKING - A pavement marking symbol that indicates an appropriate bicycle positioning
30 in a shared lane.

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- 1 SHARED ROADWAY - A roadway that is open to both bicycle and motor vehicle travel. This may be an
2 existing roadway, a street with wide curb lanes, or a road with paved shoulders.
- 3 SHARED USE PATH - A bikeway physically separated from motorized vehicular traffic by an open space or
4 barrier and either within the highway right-of-way or within an independent right-of-way. Shared use
5 paths may also be used by pedestrians, skaters, wheelchair users, joggers and other non-motorized
6 users.
- 7 SHOULDER - The portion of the roadway contiguous with the traveled way, for accommodation of
8 stopped vehicles, emergency use and lateral support of sub-base, base and surface courses, often used
9 by cyclists where paved.
- 10 SIDEWALK - That portion of a street or highway right-of-way, beyond the curb or edge of roadway
11 pavement, which is intended for use by pedestrians.
- 12 SIDEPATH - A shared use path located immediately adjacent and parallel to a roadway.
- 13 TRAVELED WAY - The portion of the roadway intended for the movement of vehicles, exclusive of
14 shoulders.
- 15 UNPAVED PATH - Path not surfaced with a hard, durable surface such as asphalt or Portland cement
16 concrete.

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WORKS CITED

1. **National Highway Traffic Safety Administration; Bureau of Transportation Statistics.** *National Survey of Pedestrian and Bicyclist Attitudes and Behaviors*. s.l. : U.S. Department of Transportation, 2002.
2. **AASHTO.** *A Policy on Geometric Design of Highways and Streets*. Washington, D.C. : American Association of State Highway and Transportation Officials, 2004.
3. **Federal Highway Administration.** *Manual on Uniform Traffic Control Devices*. Washington, D.C. : Federal Highway Administration, U.S. Department of Transportation, 2009.

CHAPTER 2: BICYCLE PLANNING

2.1. BACKGROUND

Bicycling is a healthy, low cost mode of travel that is available to nearly everyone. Bicycling is also the most energy-efficient form of transportation available. Since bicycling emits no pollution, requires no external energy source, and uses land efficiently, it effectively moves people from one place to another without adverse environmental impacts. For communities working to address a wide range of issues from traffic congestion to climate change, bicycling is a transportation solution that works at both local and global levels.

Surveys show that people support bicycling because it makes neighborhoods safer and friendlier, saves on motorized transportation costs, provides a way to routinely get physical activity, and reduces transportation-related environmental impacts, emissions, and noise. Bicycling increases the flexibility of the transportation system by providing additional mobility options, especially for short-distance trips that are too far to walk and too close to drive. Bicycle transportation is particularly effective in combination with transit systems, as when used together, each expands the range of the other mode.

2.2. WHY PLANNING FOR BICYCLING IS IMPORTANT

As communities throughout the U.S. face new challenges, bicycling provides a solution to many different concerns. Since the bicycle is an appropriate vehicle for many trips, it can play a significant role in sustainable land use planning, transportation, recreation, and economic development initiatives. Particularly in urban and suburban centers, where a large percentage of trips are shorter than two miles in length, bicycling can serve as part of a comprehensive approach to alleviate traffic congestion and provide flexible, convenient, and affordable travel options. Bicycling is also very compatible with transit system development, and can effectively expand the area served by each transit stop.

Like other users of the transportation system, bicyclists need access to jobs, goods and services, and recreational activities. Planning for existing and potential bicycle use should be integrated into and coordinated with the overall transportation planning process. Transportation improvements can provide an opportunity to enhance the safety and convenience of bicycle travel.

Improvements made for bicyclists often result in better conditions for other transportation users. For instance, paved shoulders, wide curb lanes, and bicycle lanes not only provide improved conditions for bicyclists, but also increase motorist comfort. Between intersections, bicycle lanes and paved shoulders result in more consistent separation between bicyclists and passing motorists. Bicycle lanes improve sight distance for motorists at driveways and provide a buffer area between sidewalks and traffic lanes,

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making streets more comfortable for pedestrians. Communities that have built bicycle networks have seen positive results for all users from modest investments.

Plans for implementing bicycle projects often require supportive policies in a community's general plan, master transportation plan, zoning ordinances, and subdivision regulations. These may need to be amended to support bicycle-compatible roadway design, encourage shared use path connections between neighborhoods, require bicycle parking, and create land-use policies that keep destinations closer to home and work.

Providing for bicycling touches on many different aspects of community planning, and a good bicycle plan reflects this dynamic. Depending on the community, a bicycle plan may involve many diverse aspects, such as signal timing and progression, safety education, building codes and parking facility design, land-use policies, school busing policies, social marketing to promote flexible transportation options, roadway maintenance and transit access, and many others.

2.3. TYPES OF BICYCLING

Many characteristics have been used to classify different types of bicycle riders. Among the most common are comfort level, physical ability, and trip purpose. These characteristics can be used to help develop generalized profiles of various bicycle user types. People will not fit neatly into a single category, and a rider's profile may change in a single day, for example, as a commuter switches to a parent who takes a child for a recreational ride. Still, these profiles provide a way to gauge approximate level of comfort on and preference for specific facility types.

2.3.1. TRIP PURPOSE

UTILITARIAN / NONDISCRETIONARY

Utilitarian or nondiscretionary trips are trips that are necessary as part of a person's daily activities. These commonly include commute trips to work or school, work-related non-commute trips, shopping and errands, or taking a child to school. Depending on the length of trip and quality of bicycle facilities provided, among other factors, bicycling trips can replace or seamlessly link with other transportation modes such as transit or motor vehicle trips.

In addition to people who choose to bicycle for transportation, utilitarian users may also include those who do not have access to an automobile or possess a driver's license, have no transit available, or are otherwise dependent upon bicycling.

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1 School trips are a special type of utilitarian trip that involve younger riders and require careful attention
2 to their characteristics. In neighborhoods with low volume, low speed streets, children who have been
3 taught basic bicycling skills can share the road with automobiles. On roadways with higher speeds and
4 volumes, bike lanes or separate pathways and safety improvements at intersections can accommodate
5 children with appropriate traffic skills.

6 RECREATION / DISCRETIONARY

7 Recreational and discretionary trips include trips made for exercise and/or leisure. Recreational users
8 cover all age groups from children to adults to senior citizens, and will have varying levels of comfort
9 when riding in traffic. Recreational trips can range from short trips within a neighborhood, to long rides
10 lasting several hours and covering many miles. Children will generally ride within their neighborhood,
11 with friends or parents, and on streets, sidewalks, or shared-use paths. Adult recreational trips cover a
12 wide range depending on the user's comfort and fitness level, with average adult users looking for
13 moderate to slow-paced riding on quiet streets or shared use paths. A smaller number of adult bicyclists
14 go on long-distance recreational trips, seeking out scenic and sometimes challenging terrain for sport
15 and fitness.

16 Mountain bicyclists fall into the category of recreational riders but are considered a unique and
17 independent group due to their regular use of natural surfaces in addition to paved surfaces. Mountain
18 bikes are generally designed for use on both types of surfaces. This guide will cover the use of mountain
19 bikes for recreational or utilitarian travel on paved surfaces but does not discuss mountain bike use on
20 narrow or single track natural surfaces.

21 UTILITARIAN VS. RECREATION

22 The line between utilitarian and recreational bicycling is blurry at best because the same transportation
23 system can be used for both purposes. Just as roads are designed for various motor vehicle trip
24 purposes, roads and pathways should be designed to facilitate various bicycle trip purposes and there is
25 very little difference between a bicycle network that is intended for recreational bicyclists versus one
26 that is designed for transportation trips.

27 People who use a bike for transportation get exercise they may not have otherwise had time for, or that
28 would have required additional time and expense, such as going to a fitness center. Unlike driving,
29 which is typically not viewed as a recreational activity but rather as a means to an end, many people
30 choose to bicycle because it achieves more than a single purpose, such as exercising while reaching a
31 destination. Bicycling is a multifaceted recreational activity for millions of people nationwide, young and
32 old, cutting across many socioeconomic and demographic categories. Some users may never go beyond
33 riding on a shared use path or low volume roads, while others may advance their skills and become
34 bicycle commuters. That is why understanding and planning for the needs and abilities of all bicycle
35 users is necessary to design successful bicycle networks.

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- 1 Exhibit 2.1 outlines common characteristics of recreational and utilitarian trips. The descriptions below
 2 provide a general idea of typical differences between trip types, however it should be noted that some
 3 trips combine purposes and do not fall into these distinct categories:

Recreational Trips	Utilitarian Trips
Directness of route not as important as visual interest, shade, protection from wind	Directness of route and connected, continuous facilities more important than visual interest, etc.
Loop trips may be preferred to backtracking; start and end points are often the same	Trips generally travel from residential to schools, shopping or work areas and back
Trips may range from under a mile to over 50 miles	Trips generally are 1-5 miles in length
Short-term bicycle parking is needed at recreational sites, parks, trailheads and other recreational activity centers	Short-term and long-term bicycle parking is needed at stores, transit stations, schools, workplaces
Varied topography may be desired, depending on the fitness and skill level of the bicyclist	Flat topography is desired
May be riding in a group	Often ride alone
May drive with their bicycles to the starting point of a ride	Use bicycle as primary transportation mode for the trip; may transfer to public transportation; may or may not have access to a car for the trip
Typically occur on the weekend or on weekdays before morning commute hours or after evening commute hours	Some trips occur during morning and evening commute hours (commute to school and work), but in general bicycle commute trips may occur at any hour of the day

4 ***Exhibit 2.1. Recreational Trips vs. Utilitarian Trips***

5 **RIDER AGE**

- 6 Adults do not have uniform cognitive and perceptual abilities. However, in comparison to children,
 7 adults generally can start and stop movement of their bicycle more quickly, are more visible to
 8 motorists, can interpret directionality of sounds with greater accuracy, and have a greater awareness of
 9 potential conflicts. In addition, most adults also operate motor vehicles and have the advantage of

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1 understanding the “rules of the road” as motorists; therefore, they are already familiar with riding in
2 traffic.

3 Seniors are a special type of adult rider who may ride at a slower pace and have longer reaction times
4 when faced with sudden hazards.

5 Children have a wide range of skills and cognitive capabilities. Generally, children are slower in
6 recognizing and responding to rapidly changing situations. This leads to possible dangers in common
7 situations that children face when riding bicycles, such as crossing streets. Children tend to:

- 8 • Have a relatively narrow field of vision.
- 9 • Have difficulties accurately judging the speed and distance of an approaching vehicle.
- 10 • Assume a vehicle can see them if they can see the vehicle.
- 11 • Have difficulty concentrating on more than one thing.
- 12 • Have difficulty understanding danger.
- 13 • Have difficulty determining the direction of auditory input.
- 14 • Have little experience with the rules of the road because they do not drive motor vehicles.

15 **2.3.2. LEVEL OF USER SKILL AND COMFORT**

16 Another way to look at user types is by comfort and skill level.

17 **EXPERIENCED AND CONFIDENT**

18 This group includes bicyclists who are comfortable riding on most types of bicycle facilities. This group
19 also includes utilitarian and recreational riders of many ages who are confident enough to ride on busy
20 roads and navigate in traffic when necessary to reach their destination. However, some may prefer to
21 travel on low-traffic residential streets or shared-use paths. Such bicyclists may deviate from the most
22 direct route to travel in their preferred riding conditions. Experienced bicyclists may include commuters,
23 long-distance road bicyclists, racers, and those who regularly participate in rides organized by bicycle
24 clubs.

25 **CASUAL AND LESS CONFIDENT**

26 This group includes a majority of the population, and includes a wide range of people: those who ride
27 frequently for multiple purposes; those who enjoy bicycling occasionally but may only ride on paths or
28 low-traffic streets in favorable conditions; those who ride for recreation, perhaps with children; and
29 those for whom the bicycle is a necessary mode of transportation. In order for this group to regularly
30 choose bicycling as a mode of transportation, a physical network of visible, convenient and well-
31 designed bicycle facilities is needed.

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- 1 People in this category may move over time to the ‘experienced and confident’ category.
- 2 Exhibit 2.2 outlines general characteristics of experienced versus casual bicyclists:

Experienced/Confident Riders	Casual/Less Confident Riders
Most are comfortable riding with vehicles on streets, and are able to negotiate streets like a motor vehicle, including using the full width of a narrow travel lane when appropriate and using left-turn lanes.	Prefer shared use paths, bike boulevards, or bike lanes along low-volume, low-speed streets.
While comfortable on most streets, some prefer on-street bike lanes, paved shoulders or shared use paths when available.	May have difficulty gauging traffic and may be unfamiliar with rules of the road as they pertain to bicyclists: may walk bike across intersections.
Prefer a more direct route.	May use less direct route to avoid arterials with heavy traffic volumes.
Avoid riding on sidewalks. Ride with the flow of traffic on streets.	If no on-street facility is available, may ride on sidewalks.
May ride at speeds up to 20 mph on flat ground, up to 45 mph on steep descents.	May ride at speeds around 8 to 12 mph.
May cycle longer distances.	Cycle shorter distances: 2 to 5 miles is a typical trip distance.

- 3 **Exhibit 2.2. Casual/Less Confident vs. Experienced/Confident Riders**

4 2.4. TYPES OF TRANSPORTATION PLANNING PROCESSES

- 5 The field of transportation planning has evolved over 20 years to reflect a growing body of experience,
- 6 literature, and lessons learned nationwide. Bicycling has been integrated into planning processes
- 7 throughout the country, in places large and small, and including both urban and rural areas. This section
- 8 of the Guide covers the following types of planning processes:

- 9
 - Comprehensive Transportation Plans
- 10
 - Bicycle Master Plans
- 11
 - Transportation Impact/Traffic Studies
- 12
 - Small-Area and Corridor-Level Planning

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- Project-Level Planning

2.4.1. COMPREHENSIVE TRANSPORTATION PLANS

Comprehensive or master transportation plans should include a bicycling component. These include Long-Range Transportation Plans, Highway Safety Plans, and Transportation Demand Management (TDM) Plans. The bicycle component of these plans should be of a similar level of detail as the motor vehicle components, for example, identifying specific short-term and long-term improvements, establishing funding priorities, and addressing policy issues. Public meetings for these plans should be designed to solicit input on bicyclists' needs and priorities, as well as input on all other modes. These plans should also provide recommendations for improving bicycle/transit connections.

In some cases, the bicycle element of the master transportation plan is a condensed version of a separate bicycle master plan (see below) and/or may incorporate the separate bicycle master plan by reference. Where this is the case, it is still important for the bicycle component to provide the same level of detail as the other modal elements of the plan.

2.4.2. BICYCLE MASTER PLANS

The purpose of a stand-alone bicycle plan is to identify the projects, policies, and programs that are needed in order to fully integrate bicycling as a viable mode of transportation within a community. Bicycle plans prepared by State DOT's are often more focused on policy issues, while bicycle plans that are completed by local or regional agencies may focus on bicycle network planning, as well as policies and design practices that support bicycling.

A good bicycle plan starts from each community's current stage - some communities may be "starting from scratch" while others may be at a more advanced stage. It should address policy, infrastructure, and programming. For a community that is embarking upon bicycle planning for the first time, the focus may be on winning support for initial projects that will generate significant use or result in visible safety improvements, and help to build momentum for subsequent projects. For a community that has already implemented a partial bikeway system and has a growing number of engaged and active bicyclists, the focus may be on how to move beyond the "low-hanging fruit" already implemented and tackle more challenging projects and programs. And for those communities in a more advanced stage, with well-developed bicycle infrastructure and significant bicycle use, well-defined policies, new education and outreach programs, and a focus on critical gaps in the network may be appropriate. All communities should address policies that encourage and support bicycle trips.

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A bicycle plan should be tailored to the unique conditions of the community which it serves. Bicycle plans for cities, suburbs, counties, regions, and states all differ significantly, depending on many factors including span of control (e.g. which roads or corridors are controlled or managed by the government entity), political support, available funding, and level of community engagement. Bicycle plans exist for every type of community: urban, suburban, rural, mountain, and resort. In fast-growing communities, bicycle plans may concentrate on policies, standards, and code language to guide future development, whereas plans for more built-out communities may be more concerned with the retrofitting of bicycle improvements at existing locations and analysis of potential off-street corridors.

A bicycle plan helps guide transportation departments to implement bikeways as part of their routine roadway maintenance and “3R” (resurfacing, restoration, or rehabilitation) activities. For example, a routine pavement overlay may provide a convenient opportunity to implement bike lanes. When signals are upgraded, it is a good time to add detectors or push buttons for bicyclists. A bicycle plan can and should deal with the immediate needs for short-term improvements, balanced with longer term projects that could be decades from realization.

PUBLIC PROCESS

To develop a plan that will enjoy community support, the process should include opportunities for the public, stakeholders, and other interest groups to participate and be heard. Public input should include a combination of strategies, such as public workshops, hearings, notices in the media, outreach events, and the formation of a Bicycle Advisory Committee. Effective committees welcome diverse viewpoints. Potential committee members may include health and/or safety advocates, educators, business leaders, law enforcement personnel, bike club members, people with disabilities, elderly, and people who are economically disadvantaged. Local officials (elected and staff) who are responsible for implementation should participate in the process.

Outreach should be conducted to target and draw out the opinion of a broad cross section of the community, including experienced, casual and novice bicyclists of all ages. These efforts could include a website, mailed surveys, school visits, or community bicycling audits.

COORDINATION WITH OTHER DOCUMENTS AND PLANNING PROCESSES

The plan should be coordinated with regional (county and MPO) and state transportation plans (modal plans, corridor plans, etc.). While bicycle transportation may not always be the primary focus of these plans, the bicycle mode should be taken into consideration and should be addressed in an appropriate level of detail. For example, the implementation of bicycle recommendations often requires revisions to land development regulations, roadway design standards, and standard design details. These documents are typically updated on a periodic basis and these updates should include bicycle provisions where appropriate and as recommended in the bicycle master plan. Coordination is also needed with

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funding programs (such as the annual capital improvements program), and planning documents of other agencies (such as transit, and parks and recreation).

PHASING OF INFRASTRUCTURE IMPROVEMENTS

A phasing plan sets forward a strategy for implementing bicycle facilities over time, reflecting political realities, future development, funding opportunities, and technical challenges. By identifying projects to be implemented in the short, medium and long term, jurisdictions can focus initially on projects that are low-cost or require minimal infrastructure work, while simultaneously starting to plan, design, and seek funding and support for longer-term, more complex projects.

Short-term projects: Short-term projects can help to create early success and show significant progress in plan implementation. These projects are generally low-cost and easy to implement. Examples include traffic signal timing adjustments or push buttons; restriping existing streets by narrowing lanes; removing travel lanes or parking and redistributing space to accommodate bike lanes; road repaving that includes bike lanes; or installation of wayfinding signage or shared lane markings. These strategies will be discussed in more detail in the design chapters.

Medium-term projects: Medium-term projects may require street repaving, facility reconstruction such as moving curbs, or funding as part of other capital improvement programs. These projects generally must undergo detailed infrastructure design study, are more complex to implement, and require time to secure funding. Medium-term projects may also be those that only occur with new facility construction or old facility rehabilitation.

Long-term projects: Long-term projects generally represent investments of major capital funds; these projects are complex from a design or political standpoint. Examples can include bicycle bridges, elevated crossings, or underpass-style tunnels. These projects can be developed through new facility construction or facility rehabilitation.

To develop a phasing plan, a number of issues should be considered:

- **Bicycle travel demand:** To what degree will the bikeway generate significant usage? How many trip generators are within close proximity of the project, such as residential areas, schools, parks, transit centers, employment and commercial districts, churches, etc? There are several methods for forecasting bicycle travel demand, as described in Section 2.6.
- **Route Connectivity and Directness:** To what degree does this alternative fill in a missing gap in the bicycle network, or make a critical connection to a transit facility or other key destination?
- **Crash/Conflict Analysis:** Does the proposed improvement have the potential to alleviate a safety problem, such as an intersection with a history of bicycle crashes or conflicts?

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- **Barriers:** How well does the alternative overcome a barrier in the current bicycle network? Barriers could include bridges, overpasses, interchanges, difficult intersections, waterways, etc.
- **Ease of Implementation:** How difficult will it be to implement this project? This criterion takes into account right-of-way, topographical, environmental, political, and economic constraints.

TYPICAL PLAN CONTENTS

A well-developed bicycle plan is comprehensive and should cover some if not all of the following topics (not necessarily in this order):

- **INTRODUCTION**

The introduction of the Plan lays a foundation and sets the context for the Plan. It should provide a brief overview of the history and current status of bicycling in the jurisdiction, may discuss current or previous planning efforts that support bicycling, provide data on current levels of bicycling (along with historical data if available), and any other information that is needed to lay a foundation for the Plan.

- **VISION, GOALS, AND OBJECTIVES**

This section establishes what the Plan hopes to accomplish. The vision statement should paint a picture of the jurisdiction in the future, once the goals and objectives have been fulfilled. Goals should be broad statements that address key focus areas, such as mobility, health, the environment, etc. Objectives identify more specific strategies for accomplishing the vision and goals.

- **BENCHMARKS OR PERFORMANCE INDICATORS**

Benchmarks should be set in such a way that jurisdictions can measure results. In order to set a baseline for performance measures, it may be necessary to collect initial data. Performance measures should be as simple as possible, and should be fairly easy to measure. In some cases, existing data collection processes (such as roadway inventories) can be adjusted to collect data relevant to bicycle performance measures (i.e. shoulder width and pavement condition).

Examples of benchmarks include the number of bikeway miles implemented, mode share percentage, rate of bicycle-motor vehicle crashes as compared to the number of bicycle trips, total number of bicycle-motor vehicle crashes, number of bike parking spaces, bike usage on a particular corridor, percentage of kids bicycling to schools, and others. Inclusion of outcome-oriented performance measures (such as usage counts and crash rates) is desirable to check effectiveness of current programs; purely inventory-oriented performance measures may not detect issues that need to be addressed.

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- **EXISTING CONDITIONS**

The overview of existing conditions should take stock of the transportation infrastructure. The existing conditions analysis should include a general assessment of streets, roads and highways by function, type, ownership, traffic volumes and speeds, width and condition, as well as an inventory of existing bikeways, including shared-use paths and trails outside the street system. Other items include bicycle parking conditions (quality and quantity), crash data, proposed developments that may have a significant impact on bicycling, bike-transit integration, and education, encouragement, and enforcement efforts.

- **RECOMMENDED BICYCLE FACILITIES**

This component is discussed in more detail in the next section. Recommendations should reflect the community's needs, as well as the feasibility of projects in specific roadway corridors. An opportunistic approach is wise – the majority of bike plans recommend new facilities in locations where other roadway projects (such as repaving) offer opportunities to implement bikeways less expensively. Projects should be identified in sufficient detail such that they can be integrated into a local capital improvement plan or advanced to a design phase. This should include, at a minimum, roadway name, beginning and end points, bikeway type, a description of the work needed, and the estimated cost. Bicycle parking needs can also be identified, as well as standards for placing bicycle parking facilities (see Chapter 6 for more information).

- **RECOMMENDED POLICIES/DESIGN GUIDELINES**

Recommendations for policy changes are a standard component of most bicycle master plans. This includes zoning and land development policies that support bicycling (such as higher densities of mixed-use development, neighborhood design that provides a high level of bicycle connectivity, bicycle parking ordinances, requirements for commuter support facilities such as showers, etc). Some bicycle plans also include design guidance that clarifies the jurisdiction's expectations in terms of bicycle facility design. This can be particularly helpful if the jurisdiction's current design guidelines do not address bicycle facilities, however ultimately the goal should be to integrate bicycle design standards into other existing documents that cover roadway design, local subdivision and development codes, or other appropriate sources.

- **RECOMMENDED EDUCATION AND ENCOURAGEMENT PROGRAMS**

This section of the Plan is very important, as there are typically many opportunities to improve conditions for bicyclists by improving behaviors. The education component should address issues such as bicycling-related information on appropriate jurisdictional websites, safety information messages for motorists and bicyclists, and bicyclist training programs for children, youth, and adults. The encouragement component can include commuter support programs and incentives, promotional activities oriented to neighborhoods and local business districts (e.g., a "shop by bike" program), campaigns to promote use of bicycles with transit, rides

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organized to introduce (or publicize benefits of) bicycling to a wider audience, and other activities to promote the more widespread practical application of bicycling.

- **IMPLEMENTATION PLAN**

This section should address short-, mid- and long-term recommendations, and should provide a phasing plan as described above. Short-term projects should include planning-level cost estimates, for budgetary purposes. Funding sources should be identified, such as local or state transportation improvement programs, special federal funding programs, local capital improvement budgets, grants, and others. All types of projects – both infrastructure and non-infrastructure (such as education and encouragement programs) should be included in the phasing plan. For some plans, it may also be desirable to identify the agencies that are responsible for implementing the recommendations.

2.4.3. TRANSPORTATION IMPACT/TRAFFIC STUDIES

Transportation impact studies attempt to disclose information to stakeholders about potential impacts and benefits of new development. Although many studies in the past focused exclusively on motor vehicle impacts, today agencies have access to resources that can be used to measure the impacts on bicyclists (see Section 2.6). The National Environmental Policy Act (NEPA), the federal law governing environmental analysis, and many state environmental laws require a full disclosure of transportation impacts, not just motor vehicle traffic impacts.

Thorough traffic studies evaluate impacts to all modes, including pedestrians, bicyclists and transit, in addition to a discussion of on-site circulation and support facilities. Impacts to bicyclists are considered significant if:

- **A project disrupts *existing* bicycle facilities.**

This can include adding new vehicular or bicycle traffic to an area experiencing safety concerns or a new development adjacent to an existing sensitive use, such as a school or park. Particular attention should be paid to on-street bicycle facilities on roadways with proposed driveways, and roadway widening or intersection improvements intended to augment motor vehicle capacity, which may reduce or eliminate shoulders or bike lanes.

- **A project interferes with *proposed* bicycle facilities.**

This includes failure to dedicate right-of-way for planned on- and off-street bicycle facilities included in an adopted bicycle master plan, or failure to contribute toward construction of planned bicycle facilities along the project's frontage. Another example is a new roadway that severs a planned pathway connection, particularly when grade separation is desirable but isn't planned for in advance.

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- **A project conflicts with adopted bicycle system plans, guidelines, policies or standards.**

This can include project designs that are in conflict with policy language, such as bicycle directness, connectivity, and network completeness.

Another consideration for bicycles in traffic studies is the evaluation of future off-site improvements to determine secondary impacts to bicycles. Impact studies typically include a set of improvements designed to reduce impacts to the transportation system. For example a project may require acceleration or deceleration lanes at a new driveway to benefit motor vehicle safety and/or capacity. Thorough transportation impact studies explicitly analyze and mitigate secondary impacts on bicycling.

2.4.4. SMALL-AREA AND CORRIDOR-LEVEL PLANNING

Transportation plans that focus on specific roadway corridors should incorporate the needs of bicyclists along with all other users. The presumption in preparing these plans is that the needs of bicyclists will be included as a routine matter, and the decision to *not* accommodate them should be the exception rather than the rule.

During the development of small-area plans and corridor plans, bicycle access along and across roadways should be planned. An opportunistic approach should be used to incorporate safety improvements for bicyclists along with other planned roadway improvements (see Section 2.5.2). In some cases, a roadway corridor or bridge replacement/reconstruction plan may create an opportunity to provide a new bicycle facility that does not necessarily connect to bikeways on either end of the corridor. However, bicycle accommodations should still be provided and should be designed with logical termini, because all bicycle networks must begin with incremental improvements that eventually result in a connected network.

2.4.5. PROJECT LEVEL PLANNING - APPROVALS

Once a specific project is identified, key considerations become the types of approvals needed or desired to move the project to construction. Approvals needed by affected government agencies, stakeholders and the general public should be identified early in the project development process. In some cases projects require approval at the national level under the National Environmental Policy Act (NEPA). There are several factors that trigger the need for NEPA approval, most commonly the use of federal funding or impacts to federal lands. In many instances, whether or not NEPA approval is needed, state and local environmental approvals as well as other permits may be required. Often times these approvals require regular updates to, and input from the general public and key stakeholders.

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During the project development and/or approval process, there is often a need to develop and evaluate design alternatives. In some cases, NEPA approval requires the evaluation of all practical alternatives that accomplish the purpose and need of the project. Analytical tools (see Section 2.6) can aid in evaluating alternatives by comparing relatively small differences in design and presenting them in a format that is relatively easy to understand.

2.5. PLANNING BICYCLE TRANSPORTATION NETWORKS

The core element of a bicycle plan will be the bicycle transportation network, composed of a connected, comprehensive system of paved shoulders, bike lanes, shared lanes, bicycle boulevards, bike routes, and shared-use paths. This section describes how to develop a bicycle network plan.

2.5.1. DECIDING WHERE IMPROVEMENTS ARE NEEDED

All roadways should be accessible by bicycle, except limited-access highways where bicycle travel is specifically prohibited. Whenever roads are reconstructed or constructed, appropriate bikeway facilities should be included to accommodate bicyclists' needs. However, technical, political, and financial realities may mean that not all roads can be immediately retrofitted or designed with the best or most appropriate bikeway. Thus, choices must be made regarding which improvements receive priority, and what level of accommodation each roadway will receive. Making these choices is both an art and a science. The science relies on use of standards, guidelines, and technical analysis tools, while the art integrates local knowledge, engineering judgment, and public input. Technical guidance on the design of different bikeway facilities is provided in Chapters 4 and 5 of this Guide.

Factors to consider when deciding where improvements are needed include:

- *User needs* - Balancing the full range of needs of current and future bicyclists.
- *Traffic volumes and speeds* — Motor vehicle traffic volumes and speeds should be considered along with the roadway width. Some bicyclists will avoid roadways with high speeds and heavy volumes of traffic, unless they are provided with a facility that offers some degree of separation from traffic. By contrast, people who regularly use a bicycle for transportation often use main roadways because their directness and higher priority at intersections typically make them more efficient routes. In many cases, the best approach is to improve the arterial roadway to accommodate bicycles, but to also provide a parallel route along streets with lower speeds and traffic volumes.
- *Overcoming barriers* — Overcoming constraints and physical barriers such as freeways or waterways should be a top priority when developing a bikeway network. A single major barrier

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(e.g., difficult intersection, bridge without sidewalks or bike lanes) can render an otherwise attractive bikeway corridor undesirable. Input from local bicyclists, along with a field analysis of major highway crossings, railroads, and river crossings, can help to identify major barriers.

- *Connection to land uses* – Bikeways should allow bicyclists to access key destinations. They should connect to employment zones, parks, schools, shopping, restaurants, coffee and ice cream shops, sports facilities, community centers, major transit connections, and other land uses that form the fabric of a community.
- *Directness of route* – A bikeway should connect to desirable locations with as few detours as possible. For example, does a bicyclist have to travel out of his or her way on a route with many turns to reach a safe freeway overpass? Multiple turns can disorient a rider and unnecessarily complicate and lengthen a trip.
- *Logical route* – Does the planned network make sense? A network should include facilities that bicyclists already use, or have expressed interest in using.
- *Intersections* – Bikeways should be planned to allow for as few stops as possible, as bicycling efficiency is greatly reduced by stops and starts. If bicyclists are required to make frequent stops, for example, along streets with stop signs every block, they may avoid the route or disregard traffic control devices. Signalized intersections with very short green times (such as those on low-priority streets) can lead to disregard for traffic control. At major streets, crossings should be carefully planned and managed to ensure maximum safety and flow.
- *Aesthetics*—Scenery is an important consideration along a facility, particularly for a facility that will serve a primarily recreational purpose. Trees can also provide cooler riding conditions in summer and can provide a windbreak. Bicyclists tend to favor roads with adjacent land uses that are attractive such as campuses, shopping districts, and those with scenic views.
- *Spacing or density of bikeways* – A bikeway network should be planned for maximum use and comfort, and thus should provide an appropriate density relative to local conditions. Some bicycle network plans have set a goal to provide a bicycle facility within one-fourth of a mile of every resident.
- *Overall feasibility*—Decisions regarding the location of new bikeways may also include an overall assessment of feasibility given physical or right-of-way constraints, as well as other factors that may impact the cost of the project. While funding availability may influence decisions, it is essential that a lack of funds not result in a poorly-designed or constructed facility. The decision to implement a bikeway plan should also be made with a conscious, long-term commitment to a proper level of maintenance. Facility selection should seek to maximize user benefit per dollar funded. Cost-benefit analysis is covered in Section 6.

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While every street will serve as a bicycle facility to some extent, focusing trips along specially treated corridors can help to attract new bicyclists and increase safety for all modes.

A context sensitive design approach is important in all aspects of roadway design. Simply applying standards, without understanding how they will function, the local context, or the future design intent, can lead to inappropriate and underused facilities. A core value of context sensitive solutions is to provide a safe facility for both the user and the surrounding community, and to ensure that the project is built in harmony with adjacent land uses, preserving important environmental, historic and aesthetic features of the area. Context sensitive designs should address the safety needs of bicyclists and should support measures that reduce the impact of motor vehicles on the environment.

2.5.2. PRACTICAL (OPPORTUNISTIC) APPROACH TO NETWORK PLANNING

Many of the most successful bike plans have been implemented through a pragmatic approach involving phasing of improvements and opportunistic partnerships with other projects and government departments/agencies. Examples of this type of approach include:

- Bike lane implementation as part of resurfacing, reconstruction, and routine maintenance overlays. Many communities have coordinated their bikeway plans and their street repaving programs to create bike lanes through the reallocation of street space during routine paving projects.
- “Complete Streets” policies: integration of bikeways in routine public works projects including highway and transit projects. Cost-effective improvements can be made by systematically including bikeways in projects as a matter of policy.
- Bikeway implementation via private sector development activity. New developments, including mixed-use projects, residential developments, and urban infill projects provide significant opportunities by including bikeways in the local planning process.
- Bikeway implementation in coordination with major capital projects. Bikeways can successfully be included in bridges, freeways, light rail projects, transit stations and other capital projects.
- Development of shared-use paths in corridors with utilities or other infrastructure improvements. Co-location of water, sewer, communications, power, and other utilities can create cost-sharing and revenue opportunities for bikeways.
- Rails-to-trails and rails-with-trails projects: Active, abandoned and rail-banked corridors are frequently used to create shared-use paths.
- Training for maintenance bureaus, planning boards, utility managers, school districts, and other agencies to ensure they are aware of the opportunity to implement bicycle facilities as part of their routine activities.

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CHOOSING AN APPROPRIATE FACILITY TYPE

Although incorporating bicyclists' needs into the design of major transportation corridors can be challenging, the reality of planning bikeways in built environments means that roadways constitute the majority of a bicycle network. Whenever streets are constructed or reconstructed, appropriate provisions for bicyclists should be included.

Technical information on the design of different bikeway facilities is provided in Chapters 4 and 5. The bikeway design options are:

- Shared lanes
- Paved shoulders
- Bike lanes
- Bike boulevards
- Shared use paths

Bike routes are not included in the list above because they represent a designation, rather than a facility type. See "Wayfinding for Bicycles" below.

CONSIDERATIONS

The best application of each of these facilities combines experience with data analysis, engineering judgment, and budget constraints. Across the nation, state and local guidelines vary considerably depending on local preferences, experience, and conditions. Thus, this Guide does not provide strict rules as to when to employ a bike lane versus a shared lane.

However, the urban centers in the U.S. that have seen the highest levels of bicycle use are those that have built a network of bike lanes and shared use paths as the backbone of their system. A very effective tool for encouraging bicycling is to provide a visible network of bikeways; it is harder (though not impossible) to attract people to use something not readily apparent.

Selection of an appropriate bikeway facility requires the following information:

- Road function (arterial, local, etc.)
- Traffic volume
- Speed
- Traffic mix (e.g. truck %)
- Expected users (e.g. is one type of user expected to dominate, such as children bicycling to school)
- Road conditions (lane widths, total roadway width, conditions at intersections and parking demand)

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- Frequency of driveways and side streets
- Topography
- Existing and proposed adjacent land uses

Bicycle quality of service tools (see Section 2.6) can be helpful in determining the appropriate facility choice, as they combine several of the factors listed above and can be used to determine the amount of lateral separation that is needed between bicycles and motor vehicles at increasing speeds. However, facility choice should also be appropriate given the type of street or corridor involved, and the potential for conflicts at intersections.

Exhibit 2.3 outlines general considerations for each facility type.

MULTIPLE FACILITY TYPES ON A SINGLE CORRIDOR

Corridors that effectively accommodate bicycles often combine multiple facility types, each type being used where appropriate. For example, a shared-use path can connect to a bicycle boulevard to create a continuous corridor. A corridor may start with bike lanes, travel along a bike boulevard, and then transition back to bike lanes. Throughout the network, transitions between facility types should be functional and intuitive.

As indicated in Exhibit 2.3, shared use paths can range from short inter-street connections to long corridor routes. Shared use paths can attract new users, and can be an asset in connecting neighboring jurisdictions and providing community cohesion. To be successful, access via the local street network is crucial, with appropriate bikeway facilities available on those connecting streets.

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Type of bikeway	Best use	Motor vehicle design speed	Traffic volume	Classification or intended use	Other considerations
Paved shoulders	Rural highways that connect town centers and other major attractors	Variable. Typical posted rural highway speeds (generally 40-55 mph)	Variable.	Rural roadways; inter-city highways	Provides more shoulder width for roadway stability. Shoulder width should be dependent on characteristics of the adjacent motor vehicle traffic, i.e. wider shoulders on higher-speed roads
Bike lanes	Major roads that provide direct, convenient, quick access to major land uses. Also can be used on collector roads and busy urban streets with slower speeds	Generally, any road where the design speed is more than 25 mph	Variable. Speed differential is generally a more important factor in the decision to provide bike lanes than traffic volumes	Arterials and collectors intended for major motor vehicle traffic movements	Where motor vehicles are allowed to park adjacent to bike lane, ensure width of bike lane sufficient to reduce probability of conflicts due to opening vehicle doors and other hazards. Analyze intersections to reduce bicyclist/motor vehicle conflicts. Sometimes bike lanes are left “undesigned” (i.e. bicycle symbol and signs are not used) in urban areas as an interim measure
Bike boulevard	Local roads with low volumes and speeds, offering an alternative to, but running parallel to, major roads. Still should offer convenient access to land use destinations	Use where the speed differential between motorists and bicyclists is typically 15 mph or less. Generally, posted limits of 25 mph or less	Generally less than 3,000 vehicles per day	Residential roadways	Typically only an option for gridded street networks. Avoid requiring bicyclists to make frequent stops. Use signs, diverters, and other treatments so that motor vehicle traffic is not attracted from arterials to bike boulevards

2 **Exhibit 2.3 – General Considerations for Different Bikeway Types**

1

Type of bikeway	Best use	Motor vehicle design speed	Traffic volume	Classification or intended use	Other considerations
Shared lanes (wide outside lanes)	Major roads where bike lanes are not selected due to space constraints or other limitations	Variable. Use as the speed differential between bicyclist and motorists increases. Generally any road where the design speed is more than 25 mph	Generally more than 3,000 vehicles per day	Arterials and collectors intended for major motor vehicle traffic movements	Explore opportunities to provide parallel facilities for less confident bicyclists
Shared lanes (shared lane markings)	Space constrained roads with narrow travel lanes, or road segments upon which bike lanes are not selected due to space constraints or other limitations	Variable. Use where the speed limit is 35 mph or less	Variable. Useful where there is high turnover in on-street parking to prevent crashes with open car doors	Collectors or minor arterials	May be used in conjunction with wide outside lanes. Explore opportunities to provide parallel facilities for less confident bicyclists. Where motor vehicles allowed to park along shared lanes, ensure marking placement reduces potential conflicts with opening car doors
Shared roadways (no special provisions)	Minor roads with low speeds and volumes, where bicycles can share the road with no special provisions	Speed differential between motorists and bicyclists is typically 15 mph or less. Generally, speed limits of 30 mph or less	Generally less than 1,000 vehicles per day.	Neighborhood or local streets	Can provide an alternative to busier streets in a gridded street network. On a non-grid network, may be circuitous or discontinuous

2 **Exhibit 2.3 – General Considerations for Different Bikeway Types (continued)**

1

Type of bikeway	Best use	Motor vehicle design speed	Traffic volume	Classification or intended use	Other considerations
Shared use path: independent corridor	Linear corridors in greenways, or along waterways, highways, active or abandoned rail lines, utility rights-of-way, unused rights-of-way. May be a short connection, such as a pathway connector between two cul-de-sacs, or a longer connection.	n/a	n/a	Provides a separated path for non-motorized users	Analyze intersections to anticipate and mitigate conflicts between path and roadway users. Design path with all users in mind, wide enough to accommodate expected usage. On-road alternatives may be desired for advanced riders who desire a more direct facility that accommodates higher speeds

2 **Exhibit 2.3 – General Considerations for Different Bikeway Types (continued)**3 **2.5.3. WAYFINDING FOR BICYCLES**

4 Developing a bicycle wayfinding system is a complex endeavor that requires the planner or designer to
5 carefully consider the routes that bicyclists prefer, balancing the need for good bicycling conditions with
6 the need for direct access to destinations. Input from local bicyclists can be very helpful when planning
7 new bicycle routes. In general, it is advisable to start with a single route, or a simple network, and then
8 build upon the network over time, rather than to attempt to implement an extensive network of
9 multiple, interconnecting routes all at one time.

10 To achieve a successful wayfinding system, the planner should conduct careful field work to identify
11 effective routes and determine where signs should be placed, so that cyclists following routes do not go
12 off course. It is very important for the route planner to approach the task from the perspective of the
13 bicyclist who will be following the signs to reach their destination.

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- 1 Chapter 9 of the Manual on Uniform Traffic Control Devices (MUTCD) (1) provides the basic guidelines
 2 for design of wayfinding signage systems for bikeways. This includes three types of bicycle route
 3 designation and guide signs (see Exhibit 2.4), which are discussed below.



D11-1c



M1-8



M1-9

Exhibit 2.4. Typical Wayfinding Signs**D SERIES ROUTE SIGNS**

The D series (green bike route sign and various destination plaques) includes the traditional green bike route sign (D11-1), as well as a newer alternative sign that replaces the words “BIKE ROUTE” with a destination or route name (D11-1c). Use of this alternative is preferred whenever possible, as it provides the rider with more useful information than the D11-1. Routes should be named with either a term that describes the corridor (for example, a route that generally follows a waterway or valley, or a route that follows or parallels a well-known street), or a destination, using a relatively well-known place reference that is at the end of that specific route.

A variety of plaques are now available to supplement the D11 sign. These plaques can be used independently or in combination with the D11 plaques. These plaques are beneficial because they provide more space for wayfinding information, such as destinations and mileage. Use of the D11 series and these plaques is covered in more detail in Chapter 4 of this guide.

M1-8 SERIES ROUTE SIGNS

The M1-8/M1-8a signs are appropriate for local and regional networks of numbered or lettered routes. Use of these signs almost always requires the production of a map or series of maps to aid the bicyclist in understanding what destinations are served by these routes. For this reason, they are generally more appropriate for longer distance routes, rather than shorter urban and suburban routes. When using numbered or lettered routes, it is important to use an organized system for designating the routes. For

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example a numbered route system could be set up to use even numbers for east-west routes and odd numbers for north-south routes.

M1-9 ROUTE SIGNS

The M1-9 sign is used for AASHTO-approved U.S. Bicycle Routes that typically extend through two or more states. To designate such a route, a coordinated submittal should be made to AASHTO by the affected states. AASHTO provides the U.S. Bicycle Route number designation.

Ideally, bike routes should be located on shared use paths and roads with favorable conditions for bicycling, including those with bicycle facilities, low motor vehicle volumes, low traffic speeds, or enough width for shoulders or appropriate lane sharing. Bicycle route designation or guide signs are useful for a variety of purposes including helping bicyclists navigate; however, the placement of wayfinding signs does not necessarily improve bicycle safety, because the signs do not alter the geometric design of the roadway. For this reason, it may be necessary to supplement bicycle wayfinding signs with other roadway improvements to accommodate bicycle travel, depending upon motor vehicle speeds and volumes along the route.

WHEN TO USE BICYCLE ROUTE AND GUIDE SIGNS

Bicycle route and guide signs can be used:

- To designate a system of routes in a city, county, region, or state that is likely to generate bicycle trips, because it connects important origins and destinations.
- To designate a continuous route, that may be composed of a variety of facility types and settings, or located wholly on local neighborhood streets.
- To provide wayfinding guidance and connectivity between two or more major bicycle facilities, such as a street with bicycle lanes and a shared use path.
- To provide guidance and continuity in a gap between existing sections of a bikeway, such as a bike lane or shared use path.
- To provide location-specific guidance for bicyclists such as:
 - How to access and cross a bridge.
 - How to navigate through an area with a complex street layout.
 - Where the route diverges from a way used by motorists.
 - How bicyclists can navigate through a neighborhood to an internal destination, or to a through route that would otherwise be difficult to find.
- To provide bicyclists wayfinding guidance along a shared use path or other bicycle facility.

Many communities find that a wayfinding system for bicycles is a component of a bicycle network that enhances other encouragement efforts, because it provides a visible invitation to new bicyclists, while also encouraging current bicyclists to explore new destinations. More information on wayfinding signs can be found in Chapter 4 of this Guide.

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2.6. TECHNICAL ANALYSIS TOOLS THAT SUPPORT BICYCLE PLANNING

A number of technical analysis tools exist to help with planning bikeways and bikeway networks. These will be discussed below, and include:

- Data collection: bike counts
- Quality of service tools
- Safety analysis
- Bicycle travel demand analysis
- GIS-based data collection/network planning
- Cost-benefit analysis

The models and tools described in this section provide planners and decision-makers with methods of synthesizing large amounts of complex information. They can also provide useful graphical tools to communicate conditions and opportunities. No one model or tool solves all problems or answers all questions; each can provide assistance to the planning effort in a different way.

2.6.1. DATA COLLECTION: BIKE COUNTS/FLOW ANALYSIS

Many of the demand projection techniques described below either require or would benefit from bicyclist count data. Cities routinely collect, analyze, and use various data on motor vehicle traffic (e.g. average daily volumes, peak hour volumes, turning movements, speed, etc.) to determine such items as number of travel or turn lanes, and signal timing. Similarly, bike-related data collection is an important part of understanding, planning, and operating a bikeway system. Bike counts and movement analysis can be used to:

- Identify corridors where current use and potential for increased use is high.
- Understand patterns of usage both before and after a facility is installed.
- Collect baseline data from which to make demand projections.
- Track bicycle use over time community-wide, on particular corridors, or in response to specific factors, such as increasing density of bikeway facilities (this can include bicycle counts on specific roadways, as well as tracking bike-on-bus boardings or bike parking usage).
- Project increases in bicycle use in future years.
- Analyze specific travel patterns, such as bicyclists' positioning or movements at intersections, sidewalk usage, use of hand signals, or interaction with motorists.
- Analyze trends, such as the wearing of helmets, use of front or back bike lights, bicyclists' stopping or yielding (or not stopping or yielding) at stop signs, yield signs and traffic signals,

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or use of hand signals. Such an analysis can be helpful in determining if a campaign to encourage helmet use, for example, was successful.

- Analyze demographic trends, such as male versus female or rider age.

By conducting counts over several years, event-specific spikes will be less likely to skew the results. Counts taken in multiple seasons can help to determine seasonal fluctuation.

Per the direction of the Institute of Transportation Engineers (ITE) National Bicycle and Pedestrian Documentation Project, a bicycle count methodology has been established that will give jurisdictions across the nation access to a rich dataset for analysis. For count forms and directions, refer to the National Bicycle and Pedestrian Documentation Project Website. (2)

2.6.2. QUALITY OF SERVICE (OR LEVEL OF SERVICE) TOOLS

Quality of service (or Bicycle Level of Service) tools can be used to inventory and evaluate existing bicycling conditions, or to forecast future conditions for bicycling under different roadway design scenarios. A variety of bicycle compatibility criteria have been developed since the early 1990s to quantify how compatible a roadway is for accommodating safe and efficient bicycle travel. More information on this topic can be found in the Highway Capacity Manual. Applications of these models include:

- Documenting current conditions on an existing roadway.
- Conducting a benefits comparison among proposed bikeway/roadway cross sections.
- Identifying roadway restriping or reconfiguration opportunities to improve bicycling conditions.
- Prioritizing and programming roadway corridors for bicycle improvements.
- Creating bicycle suitability maps.
- Documenting improvements in a corridor or system-wide bicycling conditions over time (typically requires that data be managed in a GIS environment).
- Determining impacts of proposed roadway projects on bicyclists.

Although the term Level of Service (LOS) implies similarity to the vehicular intersection delay rating system established in the Highway Capacity Manual, bicycle level of service evaluates bicyclists' perceived safety and comfort with respect to motor vehicle traffic while traveling in a roadway corridor. To evaluate bicycle LOS, a mathematical equation is used to estimate bicycling conditions in a shared roadway environment. This modeling procedure calculates a user comfort rating (A through F, A being the best and F, the worst), from such factors as curb lane width, bike lane widths and striping combinations, traffic volumes, pavement surface condition, motor vehicle speeds, presence of heavy vehicle traffic, and on-street parking.

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Bicycle LOS provides a score for each roadway that indicates how comfortable a “typical” adult bicyclist would feel while riding along that roadway during peak travel conditions. Some bicyclists may feel more or less comfortable than the bicycle LOS calculated for a roadway. A poor bicycle LOS grade does not mean that bikes should be prohibited on a roadway, rather it means that the roadway is a candidate for improvements to better accommodate bicyclists.

It is important to distinguish between a segment-based and intersection-based LOS. The models discussed above do not address intersection LOS. Intersections can be significant barriers to bicycling, and a corridor with relatively high bicycle LOS along its segments can be less suitable due to intersections that have a low bicycle LOS. Factors that impact intersection LOS for bicycles include lane widths, motor vehicle speeds, crossing distance, signal timing, and conflicts with turning vehicles.

The detailed knowledge of local bicyclists and bicycle planners should be used to corroborate bicycle LOS model results.

2.6.3. SAFETY ANALYSIS

Analysis of crash trends, particularly at intersections or along corridors where most bicycle-motor vehicle related crashes occur, is one of several factors that are helpful when selecting and designing appropriate bikeways (see Section 2.5.1). By analyzing crash data, planners seek to target specific areas, understand the combination of conditions that could be creating high crash rates, profile high-risk corridors, compare the characteristics of one bikeway or potential bikeway to another, and focus attention most effectively.

When using crash data to determine locations that need safety improvements, it is important to review at least three years of data in order to account for anomalies that might occur in a single year.

However, there are a number of limitations associated with crash data, as well as difficulties accessing data:

- Bicycle-related crashes are generally underreported, especially those resulting in only minor injuries. (3)
- Crash data fails to capture unsafe locations characterized by near-misses.
- Bicycle count and exposure data is often lacking so it is difficult to identify a crash rate.
- Crash databases typically only include bicycle-motor vehicle crashes; crashes that do not involve a motor vehicle are not reported (for example, single-bicycle crashes influenced by poor surface conditions).

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- Non-traditional data sources, such as hospital records, may help create a more comprehensive picture of crashes at a location or along a corridor, but are time consuming to collect and analyze. (3)
- Existing data can be difficult to interpret, is often scattered through different systems and departments, and does not always yield enough crashes at a single location to produce statistically reliable results.
- If the data has not been sorted and mapped (such as through the PBCAT tool described below), the process of analyzing data can involve significant effort.
- Depending upon the methods used to report bicycle crashes, it can be difficult to determine the actual location or cause of the crash, or to glean other helpful information (such as the age of the bicyclist, whether the bicyclist was wearing a helmet, etc).

PEDESTRIAN AND BICYCLE CRASH ANALYSIS TOOL

The Pedestrian and Bicycle Crash Analysis Tool (PBCAT) is a software product developed by the Federal Highway Administration that can be used to develop and analyze a database containing details associated with crashes between motor vehicles and pedestrians or bicyclists. (4) (5) The database is typically built using detailed crash reports, which are generated by law enforcement agencies. PBCAT is a valuable tool, because in addition to identifying crash locations, it identifies the crash type (among a list of common reasons for crashes) and recommended countermeasures. During project planning, PBCAT can help to identify specific locations where additional design measures may be needed to increase bicycle safety. More information on PBCAT can be found at the Pedestrian and Bicycle Information Center Website. (6)

INTERSECTION SAFETY INDEX

The Bicycle Intersection Safety Index can be used to evaluate individual intersection approaches and crossings. (7) This method helps determine which intersections or approach legs should be prioritized for further evaluation and may be helpful for prioritizing safety improvements. The safety index score is based on a number of measurable characteristics of the intersection (number of lanes, configuration of turn lanes, presence of bike lane, type of traffic control, and traffic volume among others). More information on the Bicycle Intersection Safety Index can be found at the Pedestrian and Bicycle Information Center Website. (8)

2.6.4. GIS-BASED DATA COLLECTION/NETWORK PLANNING

Geographic Information Systems (GIS) are a useful tool during the development of a bicycle network plan. GIS mapping enables the planner to combine a visual representation of a bicycle network with large quantities of background data that are needed for each individual roadway or pathway segment

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1 within the network. This enables a level of comprehensive analysis that is more efficient and enables
2 the planner to track progress over time as roadways are improved with new bicycle facilities.

3 GIS mapping is typically used to catalogue essential data that is collected either from other databases
4 (such as average daily traffic or traffic speeds), from aerial photography (such as presence of a shoulder
5 on the roadway), or through field data collection (such as pavement condition or lane widths). GIS
6 mapping can also be used to develop network maps that indicate the type of facility that is
7 recommended for each roadway segment, as well as the proposed method of accomplishing the
8 improvement (such as lane width reductions, addition of new pavement, etc). Analysis in a GIS-based
9 environment is required in order to apply systematic evaluation tools such as bicycle LOS. Crash data
10 can also be more efficiently analyzed in a GIS database that enables the planner not only to view the
11 locations of crashes on a map, but also the background information on each crash (fault, time of day,
12 age of bicyclist, etc).

13 2.6.5. BICYCLE TRAVEL DEMAND ANALYSIS

14 Understanding existing and potential levels of bicycling is important in bikeway planning, particularly if it
15 is necessary to prioritize among many potential capital investments in bicycle infrastructure. Measuring
16 demand is less important when opportunities arise to incorporate the needs of bicyclists in roadway
17 resurfacing and rehabilitation projects, since routine accommodations for bicycling should be a standard
18 operating procedure.

19 Evaluating bicycle travel demand shares some similarities to motor vehicle travel demand modeling.
20 Both forecast future needs based on objective data inputs. However, bicycle travel demand should also
21 account for *latent* demand (demand that is not apparent, but underlying) because existing conditions on
22 a roadway are often a significant deterrent to travel. Therefore, bicycle travel demand methods make
23 assumptions regarding how many people would choose to bicycle along a given corridor if conditions
24 were conducive to bicycling. This is, at best, a very inexact science due to the many other causal factors
25 involved in the decision to ride a bicycle, including the level of connectivity of the overall bicycle
26 network, the availability of bicycle parking, typical trip lengths, and seasonal variations.

27 Compared to the vast amount of data collected for motor vehicles, there are virtually no widely-
28 accepted sources of data available to evaluate demand for bicycling. The ITE Trip Generation Manual is
29 widely used for data on trip generation, distribution, and other motor vehicle considerations; however,
30 no such system exists for bicycles.

31 Choosing the correct tool to measure latent demand is dependent upon desired outcome, availability of
32 data, ease of analysis, required accuracy, sensitivity to design factors, and whether the target of the
33 evaluation is a single facility or an entire network. The tools vary in their qualitative versus quantitative
34 approach to bicycle travel demand. The former depends on logic, examples, public input, and

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experience, while numbers will drive the latter. The qualitative approach generally requires less time and little data collection, while a quantitative approach may require a high level of demographic data collection, user and household surveys, and proficiency with data and statistical analysis.

Types of travel demand analysis include:

- Comparison studies
- Sketch plan methods
- Market analysis/land use models
- Discrete choice survey models

COMPARISON STUDIES

This type of study involves comparing an existing facility with a proposed one. Adjustments for demographic and land use differences can refine the study. Steps include creating a list of comparable facilities and analyzing their similarities to the project location in terms of land use types, population density, income, availability of alternative routes, and presence of schools, parks, employment, transit availability, and network continuity. When the comparison facility is selected, counts conducted will determine the level of use. Adjusting for differences between the two locations completes the process. An ideal case study will have data taken before and after implementation to compare expected with actual increases in cycling. This method works well when similar facilities for comparison exist within the region or market.

SKETCH PLANS

Sketch plan methods depend on rules of thumb and simple calculations to derive a demand estimate. For example, many communities need a demand estimate for a proposed trail or bikeway corridor as part of a funding request. This method uses regional or national datasets including the National Census, Journey to Work data, or the National Household Trip Survey to establish a baseline of potential corridor users. Refinements are then made based on a variety of factors, such as percentage of students or youth within the corridor area, seasonal variations, bike-transit trips, or utilitarian trips. Sketch plan methods are typically less reliable than other methods, such as comparison studies or market analysis tools.

MARKET ANALYSIS/LAND USE TOOLS

Modeled after land-use projection tools, these GIS-based approaches analyze demographic and land-use conditions to evaluate existing conditions and project future potential bicycle demand across a zone or community. Factors analyzed include street connectivity, destination land uses, topography, barriers, crash statistics, demographic data, and bikeway network density and quality. By comparing these existing conditions to perfect or 'ideal' conditions practitioners can match improvements to areas with the highest potential demand.

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DISCRETE CHOICE MODELS

Discrete choice models rely on surveys to ask people to catalogue their trips or predict their travel behavior if conditions were to change. They can be used to measure mode split based on the cost of travel time, fiscal cost, and convenience and can feed into regional travel models.

2.6.6. COST-BENEFIT ANALYSIS

Planning agencies can use cost-benefit analysis to quantify the impacts of bicycle facilities and discuss them in easily understood terms. Costs are generally divided into one-time capital construction costs and ongoing annual operating costs. Application of a cost-benefit methodology to bicycle projects can allow comparison to motor vehicle and transit projects. A comparative cost-benefit analysis of planned bikeway facilities can help prioritize projects that will have a high benefit-to-cost ratio. A cost-benefit analysis tool for bicycle facilities can be found at the Pedestrian and Bicycle Information Center Website. (9)

2.6.7. KEY ROLE OF PUBLIC INPUT IN THE PROCESS

All of the tools described above contribute to the planning process. However, no tool is a substitute for public input. Bicyclists in the community have the best knowledge of current conditions as well as specific opinions on areas where new facilities are needed or where existing facilities need improvement. Opinions and feedback of interested users who do not ride extensively (or at all) should also be sought to provide input regarding which facilities or programs they need in order to start riding. It is therefore important to identify ways to gain feedback from both bicyclists and non-bicyclists in the community.

2.7. INTEGRATING BICYCLE FACILITIES WITH TRANSIT

The relative ease of access to transit often determines a traveler's decision whether or not to ride transit. Programs that educate the public about connections between bicycling and transit can promote both modes simultaneously. Linking bicycles with transit overcomes such barriers as lengthy trips, personal security concerns, poor weather, and riding at night or up hills.

Safe and convenient routes that serve bicyclists should be viewed as essential support strategies in increasing transit ridership. The "catchment area" for bicycle-to-transit trips is typically two to three miles. This is the area within which bicyclists will chose to bicycle to or from transit as a segment of a longer trip.

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There are four main components of bicycle-transit integration:

- Facilitating bicycle access on transit vehicles;
- Offering bicycle parking at transit locations;
- Improving bikeways to transit; and
- Promoting usage of bicycle and transit programs.

Bicycle transport on transit vehicles should include access at all reasonable hours with enough spaces to meet the demand. A number of parking and bicycle-on-transit storage systems are available and in use. Transit stations should allow easy access for cyclists; this may include installation of an elevator, retrofitting a staircase with a bicycle channel, or providing access by ramps.

On highways and streets, combined bicycle and transit facilities, such as shared lanes or bicycle lanes adjacent to transit corridors, sometimes create design challenges for practitioners. As the bus pulls into a conventional, sidewalk stop, it crosses the area where bicyclists are most likely to ride (whether there is a designated bicycle lane or not). Bicyclists then typically pass the bus on the left. Once the bus has completed on- and off-boarding passengers, it crosses into the travel lane and the cycle repeats itself at each subsequent stop. This “leap-frog” effect is a fact of urban bicycle travel and is sometimes impossible to avoid; however, effective countermeasures include proper pavement markings for bike lanes at bus stops, provision of bike lanes on the left-hand side of the roadway on one-way streets, combined bus/bike lanes, added training for bus drivers, and educational materials for bicyclists (which can be displayed on the outside of the bus itself).

Bicycle parking at transit stops and stations should be well promoted and secure, with enough spaces available to meet the demand. Ideally, parking will include both short-term and long-term facilities.

Bicycle and transit integration continues to expand. Other areas of potential growth in bicycle and transit integration include:

- Emerging ways of accommodating bicycles on transit, such as high-capacity, on-bus bicycle racks, bicycle-on-vanpool services, and new methods for storing bicycles on rail cars.
- Emerging techniques for storing bicycles at transit hubs, such as high-capacity bike parking at transit stations and full-service staffed bicycle parking.
- More on-road bicycle and transit facilities, such as shared bus/bicycle streets and lanes.
- New methods of bicycle and transit education, such as on-bus bicycle rack demonstrations for bicyclists and share-the-road training for bus drivers.
- More coordination with local jurisdictions to provide bicycle access improvements in areas around transit stops and including bicycle access information on transit maps.
- New performance measures for evaluating the effectiveness of bicycle services.

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- 1 Many transit agencies throughout the U.S. have participated in local bicycle planning efforts and
- 2 interface with bicycle advocacy organizations. Many view efforts to better accommodate bicyclists as
- 3 positive public marketing components and as a method of increasing the viability of transit (10).

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1. **Federal Highway Administration.** *Manual on Uniform Traffic Control Devices*. Washington, D.C. : Federal Highway Administration, U.S. Department of Transportation, 2009.
2. **Institute of Transportation Engineers.** National Bicycle and Pedestrian Documentation Project. [Online] <http://bikepeddocumentation.org/>.
3. **Stutts, J.C. and Hunter, W.W.** *Injuries to Pedestrians and Bicyclists: An Analysis Based on Hospital Emergency Department Data*. Washington, DC : Federal Highway Administration, 1997. FHWA-RD-99-078.
4. **Harkey, D.L., J. Mekemson, M.C. Chen, and K Krull.** *Pedestrian and Bicycle Crash Analysis Tool*. Washington, DC : Federal Highway Administration, 1999. FHWA-RD-99-192.
5. **Harkey, D.L., S. Tsai, L. Thomas, and W.W. Hunter.** *Pedestrian and Bicycle Crash Analysis Tool (PBCAT): Version 2.0 Application Manual*. Washington, DC : Federal Highway Administration, 2006. FHWAHRT-06-089.
6. **FHWA.** *Pedestrian and Bicycle Information Center*. [Online] University of North Carolina Highway Research Center. <http://www.walkinginfo.org/facts/pbcat/index.cfm>.
7. *Bicyclist Intersection Safety Index*. **Carter, D.L., W.W. Hunter, C.V. Zegeer, J.R. Stewart, and H.F. Huang.** Washington, DC : Transportation Research Record, 2007, Vol. No. 2031.
8. **FHWA.** *Pedestrian and Bicycle Information Center*. [Online] University of North Carolina Highway Research Center. <http://www.walkinginfo.org/library/details.cfm?id=2802>.
9. —. *Pedestrian and Bicycle Information Center*. [Online] University of North Carolina Highway Research Center. <http://www.bicyclinginfo.org/bikecost>.
10. **Program, Transit Cooperative Research.** *Synthesis 62: Integration of Bicycles and Transit: A Synthesis of Transit Practice* . Washington, D.C : Transportation Research Board, 2005.

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CHAPTER 3: BICYCLE OPERATION AND SAFETY**3.1. INTRODUCTION**

The purpose of this chapter is to provide the designer with a basic understanding of how bicyclists operate and how their vehicle influences that operation. Knowledge of these elements is essential in order to design appropriately for this mode. Due to the bicycle operator's physical exposure and the unique characteristics of their vehicle, bicyclists are susceptible to severe injury in even minor incidents. Understanding bicyclists' operating characteristics is therefore essential to design facilities that minimize the risk of injury. This chapter covers the following topics:

- Design Vehicle
- Traffic Principles for Bicyclists
- Causes of Bicycle Crashes

3.2. DESIGN VEHICLE

The physical dimensions and operating characteristics of bicyclists vary considerably. Some of this variation is due to differences in types and quality of bicycles, whereas other variations are due to differing abilities of bicyclists. For bikeways that are shared with other users, such as shared use paths, the bicycle may not always be the critical design user for every element of design. For example, most intersections between roads and pathways should be designed for pedestrian crossing speeds as they are the slowest user.

As with motor vehicles, there are multiple types of design bicyclists. Many of the design dimensions for bikeways presented in this guide are based on critical dimensions or characteristics of different types of bicyclists. For example, recumbent and hand bicyclists are the critical user for eye height; however, a bicycle with a trailer might be the critical user when designing a median refuge island at a shared use path-roadway intersection.

This guide therefore presents bikeway design dimensions that accommodate a range of bicyclists and other non-motorized users, as appropriate. Critical physical dimensions for upright adult bicyclists are shown in Exhibit 3.1. The minimum operating width of 4 feet (1.2 m), sufficient to accommodate forward movement by most bicyclists, is greater than the physical width momentarily occupied by a rider because of natural side-to-side movement that varies with speed, wind, and bicyclist proficiency. Additional operating width may be required in some situations, such as on steep uphill grades, and the figure does not include shy distances from parallel objects such as railings, tunnel walls, curbs or parked cars. In some situations where speed differentials between bicyclists and other vehicles are relatively

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- 1 small, cyclists may accept smaller shy distances. However this should not be used to justify designs that
 2 are narrower than recommended minimums. The operating height of 8.3 feet (2.5 m) can
 3 accommodate an adult bicyclist standing upright on the pedals. Other typical dimensions are shown in
 4 Exhibit 3.1. (1)

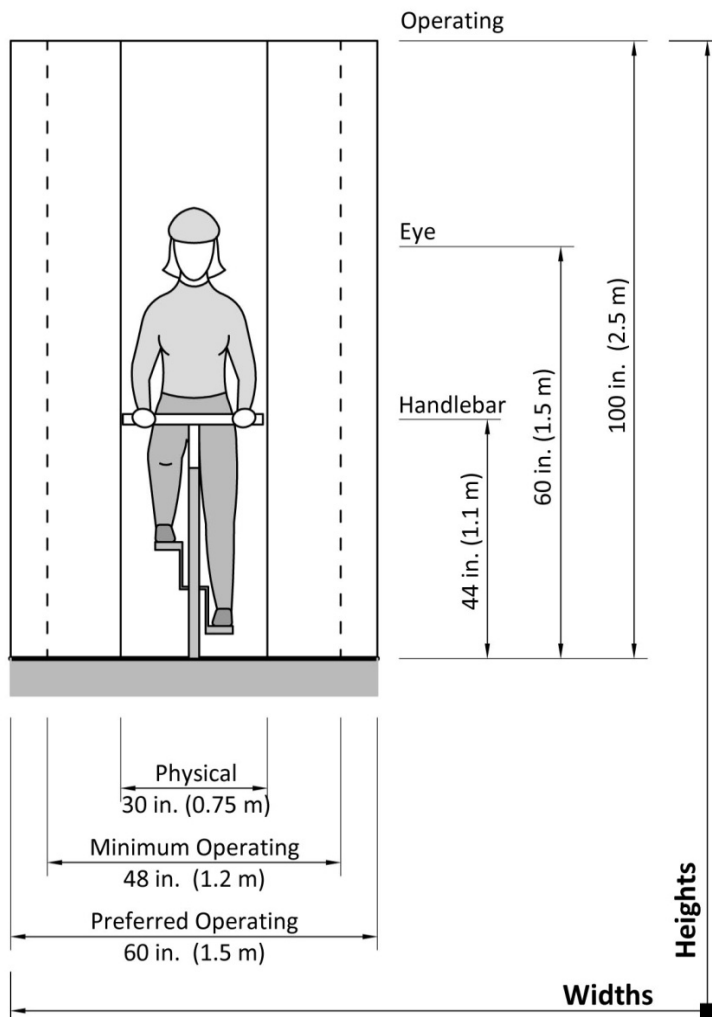
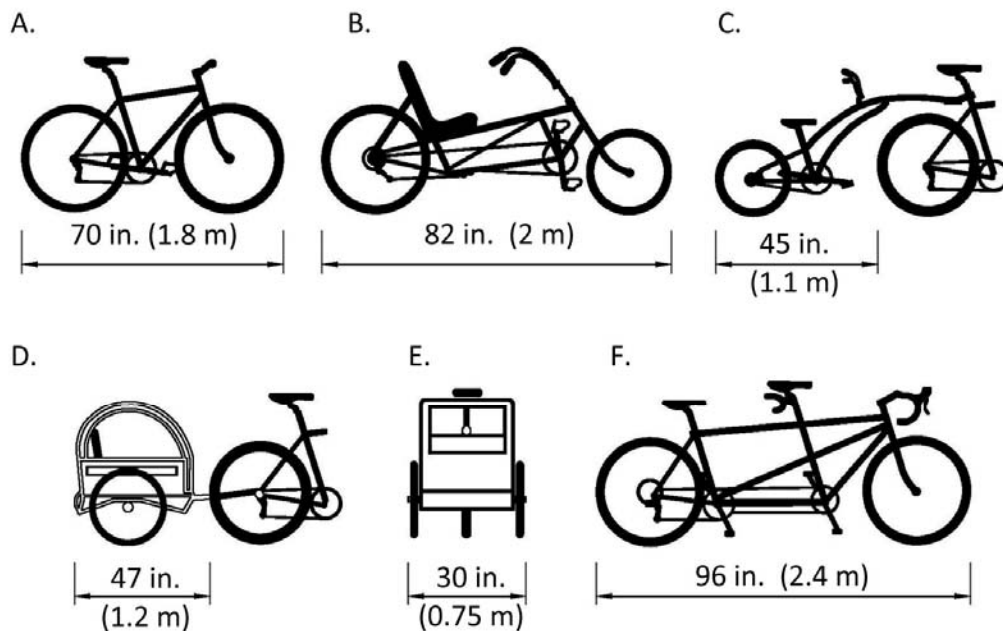


Exhibit 3.1. Bicyclist Operating Space

Exhibit 3.2 contains dimensions for several different types of bicycles including a typical bicycle, recumbent bicycle, tandem bicycle, and a bicycle with a child trailer. (1)

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A. Adult Typical Bicycle

B. Adult Single Recumbent Bicycle

C. Additional Length for Trailer Bike

D. Additional Length for Child Trailer

E. Width for Child Trailer

F. Adult Tandem Bicycle

1

2 **Exhibit 3.2. Typical Bicycle Dimensions**

3 Exhibit 3.3 lists various key dimensions for typical upright adult bicyclists, as well as key dimensions for
 4 other types of users including recumbent bicyclists, tandem bicyclists, bicyclists pulling a child trailer and
 5 in-line skaters. Unless otherwise noted, values associated with the 85th percentile of distribution are
 6 used to provide a conservative estimate that encompasses most bicyclists. (1) (2) (3)

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User Type	Feature	Dimension	
		US Customary	Metric
Typical upright adult bicyclist	Physical Width (95th percentile)	30 in	0.75 m
	Physical length	70 in	1.8 m
	Physical height of handlebars (typical dimension)	44 in	1.1 m
	Eye height	60 in	1.5 m
	Center of Gravity (approximate)	33-40 in	0.8-1.0 m
	Operating width (minimum)	48 in	1.2 m
	Operating width (preferred)	60 in	1.5 m
	Operating height (minimum)	100 in	2.5 m
	Operating height (preferred)	120 in	3.0 m
Recumbent bicyclist	Physical length	82 in	2.2 m
	Eye height	46 in	1.2 m
Tandem bicyclists	Physical length (typical dimension)	96 in	2.4 m
Bicyclist with child trailer	Physical width	30 in	0.75 m
	Physical length	117 in	3.0 m
Hand bicyclist	Eye height	34 in	0.9 m
Inline skater	Sweep width	60 in	1.5 m

Exhibit 3.3. Key Dimensions

As with bicycle dimensions, bicyclist performance can vary considerably based upon operator ability and vehicle design. Exhibit 3.4 lists various performance criteria for typical upright adult bicyclists as well as key performance criteria for other types of bicyclists. (1) (2) (3)

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Bicyclist Type	Feature	Value	
		US Customary	Metric
Typical upright adult bicyclist	Speed, paved level terrain	8-15 mph	13-24 km/h
	Speed, downhill	20-30 plus mph	32-50 plus km/h
	Speed, uphill	5-12 mph	8-19 km/h
	Perception reaction time	1.0-2.5 s	1.0-2.5 s
	Acceleration rate	1.5-5.0 ft/s ²	0.5-1.5 m/s ²
	Coefficient of friction for braking, dry level pavement	0.32	0.32
	Coefficient of friction for braking, wet level pavement	0.16	0.16
	Deceleration rate (dry level pavement)	8.0-10.0 ft/s ²	2.4-3.0 m/s ²
	Deceleration rate for wet conditions (50-80% reduction in efficiency)	2.0-5.0 ft/s ²	0.6-1.5 m/s ²
Recumbent bicyclist	Speed, level terrain	11-18 mph	18-29 km/h
	Acceleration rate	3.0-6.0 ft/s ²	1.0-1.8 m/s ²
	Deceleration rate	10.0-13.0 ft/s ²	3.0-4.0 m/s ²

Exhibit 3.4. Key Performance Criteria

Bicyclist speeds vary based on age and ability. Adults typically ride at 8-15 mph (13-24 km/h) on level terrain, while children ride more slowly. Experienced, physically fit riders can ride up to 30 mph (50 km/h); very fit riders can ride at speeds in excess of 30 mph (50 km/hr) but will typically only ride at such speeds on roads.

3.3. TRAFFIC PRINCIPLES FOR BICYCLISTS

This section describes the basic principles of operating a bicycle in traffic, including bicyclists' positioning on the road in a variety of different situations. A thorough understanding of these principles is necessary to plan and design bikeways and roadways open to bicycling, particularly in challenging design contexts.

Because some States' laws differ on the specifics of legal bicycle operation, this section will address basic principles that are fairly universal regardless of legal statute. Local traffic culture and physical design may influence bicycle operating patterns more than the details of State traffic codes, which are often not well known even to licensed motorists. Bicyclists tend to operate similarly in comparable traffic conditions, regardless of where they are riding.

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State traffic codes in the U.S. either explicitly define the bicycle as a vehicle or give the operator of a bicycle the rights and duties of an operator of a vehicle, with exceptions (e.g., bicycles may be ridden on sidewalks in some circumstances). The fact remains, however, that the bicycle has different physical dimensions and performance characteristics than a motor vehicle. A bicyclist is also more vulnerable in the event of a crash than a motorist.

The basic principles of bicycle operation in traffic include the following:

- **BICYCLISTS ON A TWO-WAY ROAD ORDINARILY RIDE ON THE RIGHT SIDE OF THE ROADWAY**

In the U.S., vehicle operators (including bicyclists) on a two-way road travel on the right side relative to their respective direction of travel. With only a few exceptions (such as when bike lanes are provided in both directions on an otherwise one-way street), bicyclists operating in the street ride with the flow of other traffic. Bicyclists may sometimes ride on the left side of a one-way street, typically if a bike lane exists on the left side, if there are markedly fewer conflicts on the left (e.g., no on-street parking and few turning conflicts), or if there is a major destination accessed from the left side.

- **BICYCLISTS OBEY STOP AND YIELD SIGNS AND OBSERVE YIELDING RULES**

Similarly to other vehicular traffic, a bicyclist on a minor road (including driveways and alleys, depending upon individual State laws) must yield to traffic on major roads. In this case yielding means proceeding only when it is safe to do so while obeying all traffic control devices.

- **BICYCLISTS YIELD WHEN CHANGING LANES**

A bicyclist who wants to move laterally on the roadway must yield to traffic in their new line of travel. In this case yielding means moving into the new line of travel only after ascertaining that the movement can be made safely and signaling the intended movement.

- **BICYCLISTS OVERTAKE OTHER VEHICLES ON THE LEFT**

A bicyclist overtaking another vehicle proceeding in the same direction must pass on the left of the vehicle being overtaken. This same basic operating principle applies to shared use paths, when bicyclists overtake pedestrians or other slower users. For bicyclists on roadways, there are several exceptions to this rule: 1) a bicyclist may pass on the right when in a bike lane; 2) a bicyclist may pass on the right when the vehicle to be overtaken is turning left or indicating a left turn; and 3) some States allow bicyclists to pass on the right when it is safe to do so.

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1 ○ BICYCLISTS' LATERAL POSITION ON THE ROADWAY IS DETERMINED BY SPEED AND
2 USABLE WIDTH

3 Bicyclists ride as far right as practical, which in on a typical roadway means that the bicyclist rides in (or
4 near) the right tire track. A bicyclist traveling at the same speed as other traffic, or in a travel lane too
5 narrow for a motor vehicle to safely pass without encroaching into the adjacent lane, travels in the
6 center of the lane (often referred to as "taking the lane"). The primary reason for taking the lane is to
7 encourage overtaking traffic to make a full lane change instead of squeezing past the bicyclist in the
8 same lane. The Uniform Vehicle Code and most State codes support bicyclists' right to take the lane if
9 necessary. Most vehicle codes also allow exceptions to the rightmost position on the road requirement
10 for reasons such as avoiding hazards, passing other cyclists and preparing for and making left turns.

11 Slower bicyclists travel to the right of faster bicyclists (and other vehicles). Like other vehicles,
12 emergency stops made by bicyclists must occur at the rightmost position on the road.

13 ○ BICYCLISTS APPROACH INTERSECTIONS IN THE RIGHTMOST LANE THAT PROVIDES
14 FOR THEIR MOVEMENT

15 Bicyclists approaching intersections typically position themselves in the rightmost lane that provides for
16 their desired movement. For example, a bicyclist traveling straight through at an intersection should not
17 position themselves to the right side of a dedicated right-turn lane, but rather in the rightmost through-
18 travel lane. Another exception occurs when a bicyclist makes a pedestrian-style left turn. This is
19 explained below.

20 ○ BICYCLISTS HAVE TWO OPTIONS FOR TURNING LEFT AT AN INTERSECTION

21 A bicyclist may make: 1) A vehicular-style left turn in which the bicyclist turns left from the left side of
22 the right half of the roadway, or from the rightmost left turn lane; or 2) a pedestrian-style left turn in
23 which the bicyclist travels in the right-most through lane across the intersection, stops at the far
24 crosswalk, makes a 90-degree turn, and then with the proper signal indication, either walks the bicycle
25 in the crosswalk or proceeds as if she were coming from the right (see Exhibit 3.5).

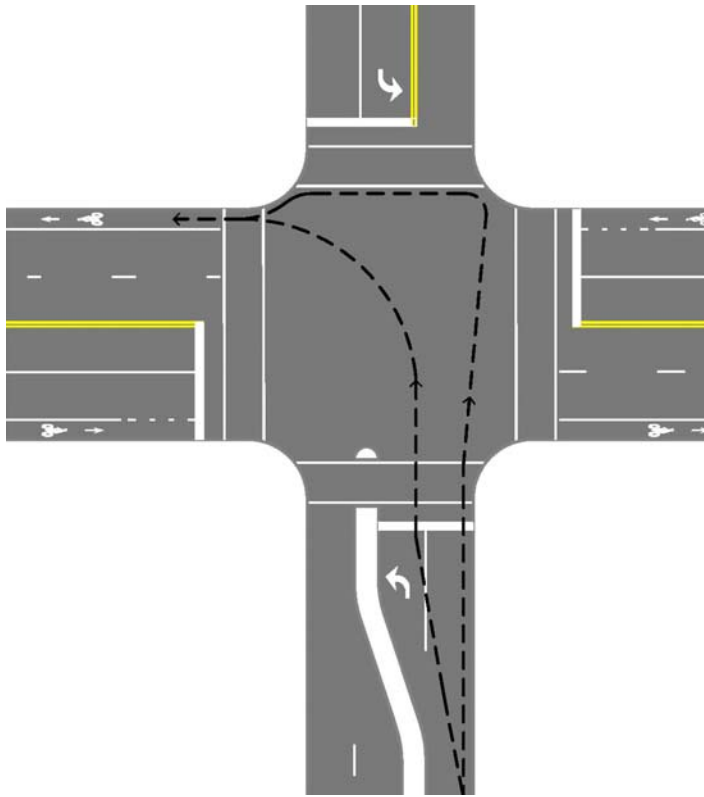
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Exhibit 3.5. A Bicyclist's Two Options for Turning Left at an Intersection

3.4. CAUSES OF BICYCLE CRASHES

By understanding the underlying causes of common bicyclist crashes, designers can more thoroughly comprehend the rationale behind many of the design principles set forth in this Guide. This section discusses common types of crashes that bicyclists experience, and how crashes relate to facility design.

3.4.1. BICYCLIST CRASH STUDIES

Numerous studies of bicycle crashes in the U.S. conducted over the past 40 years have produced very consistent results. This section summarizes common types of crashes and the factors that contribute to those crashes. Most information on bicyclist injury crashes comes from crashes with motor vehicles occurring in the public right-of-way, because reporting these crashes is mandatory in most states. Bicyclist-motor vehicle crashes that occur in non-roadway locations (paths, parking lots and driveways), as well as injury crashes that do not involve a motor vehicle, are usually not reported to State DOTs. Studies that examined hospital records have demonstrated that the majority (70-90%) of bicyclist crashes that are serious enough to warrant a trip to the emergency room are *not* the result of a collision

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with a motor vehicle. Most result from falls, crashes with fixed objects, and collisions with other cyclists. (4)

3.4.2. OVERALL FINDINGS

An examination of bicyclist-motor vehicle crashes in the aggregate yields less useful information than subdividing the results into the following broad categories: urban vs. rural, young vs. adult bicyclists, bicyclist vs. driver error, nighttime vs. daytime, and riding on the sidewalk vs. the roadway.

URBAN VS. RURAL

In urban areas, the majority of crashes occur at intersections and driveways. (5) These include bicyclists hit by motorists turning into and out of driveways and intersecting roadways, as well as bicyclists exiting driveways onto roadways. Left-turning motorists failing to yield to an oncoming bicyclist is a very common urban crash type. Hitting an open car door is estimated to represent between 3% and 6% of urban crashes; this percentage can be higher in cities with a high amount of on-street parking, lower in suburban areas with no on-street parking. (6) (7) (8) Overtaking or being struck from behind represents a small portion of crashes in urban areas, but a larger portion of crashes on rural roads. Overtaking crashes in urban areas often occur at night and are usually associated with poor lighting conditions. Overtaking crashes in rural areas are often associated with distracted drivers, or drivers driving too fast in areas with poor visibility (around curves or over the crest of a hill). (5) (9)

YOUTH VS. ADULT BICYCLISTS

Compared to their representation in the overall population, bicyclists under the age of 15 (particularly ages 10-14) are overrepresented in crashes with motor vehicles, while adults ages 25-44 and seniors (age 65+) are underrepresented. However, bicyclists older than age 44 are overrepresented with regard to serious and fatal injury. (5)

BICYCLIST VS. DRIVER ERROR

Bicyclists were judged to be solely at fault in about half of crashes with motor vehicles. Failure to yield, riding against traffic, and stop sign violations are the most common bicyclist contributing factors. Failure to yield is the most common contributing factor in crashes where motorists were at fault. The likelihood of a bicyclist being responsible for a crash is greater for young bicyclists; the likelihood of a motor vehicle driver being responsible is greater for crashes involving adult bicyclists. (5)

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NIGHTTIME VS. DAYTIME

The relatively high incidence of crashes that occur at night and dusk indicate that poor roadway lighting and a lack of proper lighting equipment appear to be contributing factors. (10) (11) The lack of supporting data on exposure makes it difficult to confirm this hypothesis, but bicyclists appear to be disproportionately struck at night, especially struck from behind; not being equipped with the required lighting appears to be a contributing factor.

RIDING ON THE SIDEWALK VS. THE ROADWAY

There is significantly higher incidence of bicyclist-motor vehicle crashes with cyclists riding on the sidewalk than with bicyclists operating in the roadway. The issue with sidewalk bicycle riding is primarily related to riding *against* the flow of adjacent traffic, as motorists crossing or turning at driveways and intersections usually do not look down the sidewalk in both directions for approaching bicyclists. (5)

3.4.3. CONTRIBUTING CAUSES OF BICYCLIST-MOTOR VEHICLE CRASHES AND RECOMMENDED COUNTERMEASURES

An understanding of the contributing causes of bicyclist-motor vehicle crashes can help decision makers choose appropriate engineering/design treatments, and implement meaningful education and enforcement programs. The following list of common behaviors includes recommended strategies to reduce the incidence of crashes due to these behaviors. The recommended engineering/design treatments are explained in further detail later in this guide.

WRONG-WAY RIDING

Riding in the direction that faces oncoming traffic puts bicyclists in a position where motorists (and other bicyclists) do not expect them, and for this reason is prohibited on the roadway. The attention of motorists who are entering the roadway is primarily directed to the left (to determine a suitable gap), and they may fail to notice bicyclists approaching from their right. Remedies for this behavior include education and enforcement, as well as engineering treatments that reinforce the correct direction of roadway travel. Bicycle lanes can reduce the incidence of wrong-way riding.

SIDEWALK RIDING

At driveways and intersections, motorists often drive onto the sidewalk area or crosswalk to get a better view of traffic, not looking for bicyclists approaching on the sidewalk (and especially unprepared to notice those riding against the direction of roadway traffic). The primary remedies for this behavior are education and enforcement in locations where riding on sidewalks is illegal. The most appropriate engineering measure to address this issue is to ensure that the roadway is designed to accommodate

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1 bicyclists, with techniques such as bike lanes on busy streets, and/or traffic-calming to reduce motor
2 vehicle speeds and/or volumes.

3 OTHER CRASHES AT DRIVEWAYS

4 Crashes also commonly occur at driveways in two other scenarios: 1) driver enters roadway from a
5 driveway and strikes a bicyclist riding in the street; and 2) driver turns off roadway into a driveway and
6 strikes a bicyclist on the sidewalk area. (5) Though the issue is motorist behavior, access control to limit
7 the number of driveways on bicycling corridors reduces these conflict points.

8 MOTORIST STRIKING BICYCLIST WITH VEHICLE DOOR (“DOORING”)

9 This type of crash occurs when a driver or passenger of a standing or parked motor vehicle opens a door
10 into traffic without making sure it is safe to do so and strikes a bicyclist traveling near the parked
11 vehicle. Remedies include educating motorists (training them to look for bicyclists before opening their
12 door) and bicyclists (training them not to ride too close to parked cars and to be on the lookout for
13 drivers opening their door, although the latter has become more difficult due to tinted windows and
14 taller vehicle design). Design treatments can help to reduce the likelihood of this type of crash. If a bike
15 lane is marked next to a parking lane, using a second stripe between the bike lane and parking lane
16 helps place cyclists further from parked cars. Some communities have used shared lane markings in
17 narrow lanes to encourage bicyclists to track over the symbol and away from parked cars.

18 BICYCLISTS RIDING OUT AT CONTROLLED INTERSECTIONS

19 The key behavior needed to avoid collision at intersections is yielding. Attempts to enforce "full stop"
20 compliance at stop-controlled junctions where most riders find they can safely yield without necessarily
21 making full stops are unlikely to be successful, given cyclists' strong counterincentive to minimize the
22 amount of energy needed to regain momentum after stopping or slowing. Signing bike routes on local
23 streets with many stop signs gives a conflicting message to riders: the streets may appear inviting, but a
24 requirement to stop at every block is discouraging. Developing bicycle boulevards (where through
25 bicycle movement with few stops is facilitated by design) is a better solution. Timing signals to better
26 accommodate typical urban cycling speeds may be helpful on arterial intersections.

27 MOTORISTS FAILING TO YIELD AT INTERSECTIONS

28 The most common crash type in this category involves the failure of a left-turning motorist to yield to an
29 oncoming bicyclist; the second most common involves a right-turning motorist who strikes a through
30 bicyclist (often referred to as a “right-hook” crash). (5) Measures that encourage bicyclist conspicuity
31 can be helpful, as can geometric modifications that limit vehicle turning speeds (e.g., reduced curb
32 radii). A bike lane provided along the left side of a dedicated right-turn lane can also help reduce the
33 incidence of such crashes. When there is insufficient width for a bike lane, shared lane markings can

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1 also be used to encourage proper positioning. Protected left-turn signal phases, where warranted, may
2 help reduce left-turn crashes.

3 BICYCLISTS STRUCK FROM BEHIND

4 While this crash type represents a small portion of urban crashes, it represents a significant portion of
5 rural crashes, especially fatalities. (5) Adding paved shoulders to narrow rural roads with high traffic
6 volumes is an effective countermeasure.

7 NIGHTTIME BICYCLE RIDING

8 About a third of bicyclist crashes occur between the hours of 5 pm and 9 pm; about a third of bicycling
9 fatalities occur between 6 pm and midnight. An additional 5 percent of crashes occur at dusk. (10) (11)
10 This is an educational and enforcement issue as all States require use of lighting equipment after sunset
11 (headlights in front, rear reflectors usually, and tail lamps as well in some states).

12 BICYCLE CRASHES INVOLVING CHILDREN

13 Children under the age of 16 tend to be overrepresented in crashes where the bicyclist was at fault.
14 Crash types where this group is overrepresented include disobeying stop signs, riding out at driveways,
15 turning or merging in front of traffic without yielding, and non-roadway crashes (parking lots and
16 driveways). (5) Some of these are behavioral issues related to lack of experience, where bicyclist
17 education and police enforcement (primarily warnings) could help, coupled with motorist education
18 regarding awareness of children's limitations. Creating a bicycle-friendly roadway environment where
19 motorists drive more slowly will also help reduce the number and severity of crashes involving children.

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WORKS CITED

1. **Landis, B.W, Petrisch, T.A and Huang, H.F.** *Characteristics of Emerging Road and Trail Users and Their Safety*. Washington, DC : Federal Highway Administration, 2004. FHWA-HRT-04-104.
2. **Florida Department of Transportation.** *Florida Bicycle Facilities Planning and Design Handbook*. Tallahassee : Florida Department of Transportation, 2000.
3. **Vermont Agency of Transportation.** *Vermont Pedestrian and Bicycle Facility Planning and Design Manual*. Montpelier : Vermont Agency of Transportation, 2002.
4. **Stutts, J.C and Hunter, W.W.** *Injuries to Pedestrians and Bicyclists: An Analysis Based on Hospital Emergency Department Data*. Washington, DC : Federal Highway Administration, 1997. FHWA-RD-99-078.
5. **Hunter, W.W, Stutts, J.C, Pein, W.E and Cox, C.L.** *Pedestrian and Bicycle Crash Types of the Early 1990's*. Washington, DC : Federal Highway Administration, 1996. FHWA-RD-95-163.
6. **Plotkin, W. and Komornick, A.** *Bicycle-Motor Vehicle Accidents in the Boston Metropolitan Region*. Boston, MA : Metropolitan Area Planning Council, 1984.
7. **Goodno, M.** *Bicycle Collisions in the District of Columbia*. Washington, DC : District Department of Transportation, 2004.
8. **NYC Departments of Health and Mental Hygiene, Parks and Recreation, Transportation, and the New York City Police Department.** *Bicyclist Fatalities and Serious Injuries in New York City: 1996-2005*. New York, NY : s.n., n.d.
9. **North Carolina Department of Transportation.** North Carolina Bicycle Crash Data. *Pedestrian and Bicycle Information Center*. [Online] http://www.pedbikeinfo.org/pbcat/types_bike.cfm.
10. **Administration, National Highway Traffic Safety.** *Traffic Safety Facts 2005*. Washington, DC : National Highway Traffic Safety Administration, (n.d.). DOT HS 810 631.
11. **National Highway Traffic Safety Administration.** *Traffic Safety Facts 2003*. Washington, DC : National Highway Traffic Safety Administration, (n.d.). DOT HS 809 775.

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CHAPTER 4: DESIGN OF ON-ROAD FACILITIES**4.1. INTRODUCTION**

This chapter provides an overview of designs that facilitate safe and convenient travel for bicyclists on roadways. Bicyclists have similar access and mobility needs as other users of the transportation system and use the street system as their primary means of access to jobs, services, and recreational activities. As the previous chapter discusses, bicycles and bicyclists have many unique features and characteristics that must be understood in order to design successfully for this mode.

Unlike the operator of a motor vehicle, whose primary responsibility is navigation and operation, the bicyclist must also provide the power to propel the vehicle and must maintain the balance necessary to keep the vehicle upright. When traffic is not congested, bicyclists usually travel more slowly than other vehicular operators on the roadway. The speed at which bicyclists can travel is limited by the relative physical strength and fitness of the operator, the terrain and geometry of the roadway, and the gearing and condition of the individual bike. Two tandem wheels make the bicycle inherently more maneuverable than an automobile, but a bicyclist is significantly more vulnerable to injury in the event of a crash. While motor vehicle operators must reach a certain age before being eligible for a license to operate on the public way, bicyclists are subject to no age limitations. All of these factors make proper bicycle facility design critical.

The guidance provided in this chapter is based on established practice supported by relevant research where available. The treatments described reflect typical situations; local conditions may vary and engineering judgment should be applied.

4.2. ELEMENTS OF DESIGN

To some extent, basic geometric design guidelines for motor vehicles will result in a facility that accommodates on-street bicyclists. If properly designed for motor vehicles, roadway design elements such as stopping sight distance, horizontal and vertical alignment, grades, and cross slopes will meet or exceed the minimum design standards required for cyclists. For example, with the exception of recumbent bicyclists, most adult bicyclists have an eye height that is higher than the standard motorist eye height which is used to determine stopping sight distance.

Surface condition significantly affects bicycle rideability. Pavement smoothness is important to bicyclist control and comfort. Gravel roads, loose material, cracks, bumps, and potholes on a paved roadway can

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pose severe steering and stopping limitations for bicyclists. Therefore, it is important to ensure that the roadway surface is in good repair -- resurfacing or reconstructing if necessary -- when establishing bike lanes or routes.

Chip-sealed surfaces are particularly difficult to ride on and should be avoided when possible. Where used, chip seals should be limited to the travel lanes on roads and highways with paved shoulders – the shoulders should not be chip-sealed. On roads with no shoulders (where bicyclists ride in the travel lanes), chip seals should use a fine mix and be covered with a fog or slurry seal.

4.3. SHARED LANES

Bicycles may be operated on all roadways except where prohibited by statute or regulation. In most instances, bicyclists and motor vehicles share the same travel lanes. Shared lanes exist everywhere; on local neighborhood streets, on city streets, and on urban, suburban, and rural highways.

There are no bicycle-specific designs or dimensions for shared lanes or roadways, but various design features can make shared lanes more compatible with bicycling, such as good pavement quality, adequate sight distances, roadway designs that encourage lower speeds, and bicycle-compatible drainage grates, bridge expansion joints, and railroad crossings. Appropriate signal timing and detector systems that respond to bicycles also make shared lanes more compatible with bicycling. If such features are not present, improvements or retrofits should be implemented. Other sections of this chapter address bicycle-compatible design features in more detail.

Generally speaking, roadways that carry low volumes of traffic, and/or where traffic typically operates at low speeds, may be suitable as shared lanes in their present condition. These roads often provide an enjoyable and comfortable bicycling experience with no need for bike lanes or any other special accommodations to be compatible with bicycling.

Various geometric and operational factors affect the comfort level of bicyclists in shared lanes. Models have been developed that quantify how various geometric and operational factors affect bicyclists. The Bicycle Level of Service (BLOS) model includes factors such as roadway lane width, lane use, traffic speed and volume, on-street parking, and surface condition in order to grade a roadway's relative comfort for bicyclists. This model can be used to determine to what extent shared lanes will adequately accommodate bicyclists given roadway conditions that exist today, or that are forecasted in the future. See Chapter 2 for a more detailed description of the use and application of the BLOS model.

DRAFT FOR AASHTO COMMITTEE REVIEW AND COMMENT**4.3.1. SHARED LANES ON MAJOR ROADWAYS (WIDE CURB/OUTSIDE LANES)**

Lane widths of 13 feet (4.0 m) or less require most motor vehicles to be driven at least part way into the next lane to pass a bicyclist with an adequate and comfortable clearance (usually 3 ft [0.9 m] or more depending on the speed of the passing vehicle). Lane widths of 14 feet (4.3 m) or greater enable motorists to pass bicycles without encroaching into the adjacent lane. The usable lane width is normally measured from the center of the edge line to the center of the traffic lane line, or from the longitudinal joint of the gutter pan to the lane line. The gutter should not be included in the measurement as usable width, as bicyclists will typically ride well to the left of the joint.

On sections of roadway where bicyclists may need more maneuvering space, the outside lane may be marked at 15 feet (4.6 m) wide. This width may be appropriate on sections with steep grades or on sections where drainage grates, raised delineators, or on-street parking effectively reduces the usable width. However, lane widths that continuously exceed 14 feet (4.3 m) may encourage the undesirable operation of two motor vehicles side by side in more congested or built-up areas. The provision of wide outside lanes should also be weighed against the likelihood that motorists will travel faster in them and that heavy vehicles (where present) will prefer them to inside lanes, resulting in decreased level of service for bicyclists and pedestrians. When sufficient width is available to provide bike lanes or paved shoulders, they are the preferred facilities on major roadways.

Roadways with shared lanes narrower than 14 feet (4.3 m) may still be designated for bicycles with bicycle guide signs and/or shared lane markings, per the guidance in this chapter.

4.3.2. SIGNS FOR SHARED ROADWAYS

A “Share the Road” sign assembly (W11-1 + W16-1P) (see Exhibit 4.1) is intended to alert motorists that bicyclists may be encountered and that they should be mindful and respectful of them. However, the sign is not a substitute for appropriate geometric design measures that are needed to accommodate bicyclists. The sign should not be used to address reported operational issues, as the addition of this warning sign will not significantly improve bicycling conditions. The sign may be useful under certain limited conditions, such as at the end of a bike lane, or where a shared use path ends and bicyclists must share a lane with traffic. The sign may also be useful during construction operations, when bicyclists may need to share a narrower space than usual on a travelway. This sign should not be used to indicate a bike route. A fluorescent yellow-green background can be used for this sign.

Another sign that may be used in shared lane conditions is the BICYCLES MAY USE FULL LANE sign (R4-11) (see Exhibit 4.2). This sign may be used on roadways without bike lanes or usable shoulders where travel lanes are too narrow for cyclists and motorists to operate side by side within a lane.

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Exhibit 4.1. SHARE THE ROAD Sign Assembly

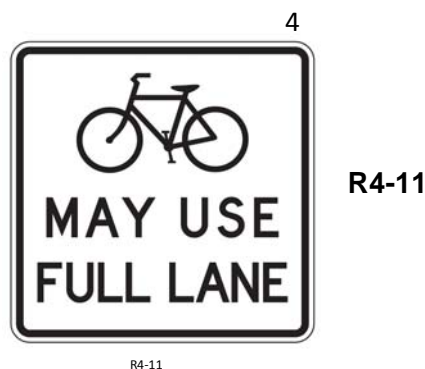
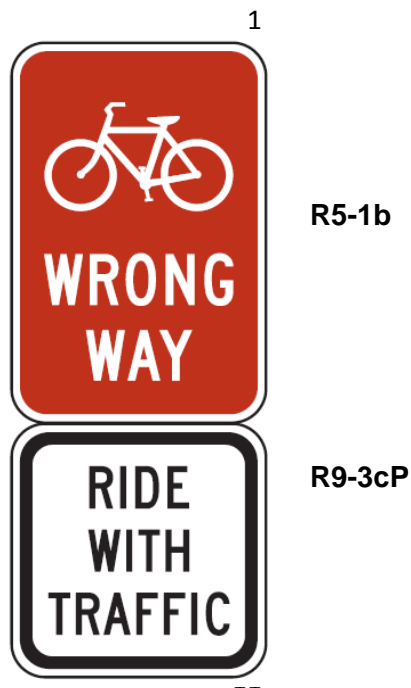


Exhibit 4.2. Bicycles May Use Full Lane Sign

Where wrong-way riding by cyclists is a frequent problem, the MUTCD (1) provides a bicycle WRONG WAY sign and RIDE WITH TRAFFIC plaque (R5-1b and R9-3cP) that can be mounted back-to-back with other roadway signs (such as parking signs) to reduce sign clutter and minimize visibility to other traffic (see Exhibit 4.3). This sign assembly can be used in shared lane situations, as well as on streets with bike lanes and paved shoulders.

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12 *Exhibit 4.3. Wrong Way - Ride with Traffic Sign Assembly*

13 4.4. MARKED SHARED LANES

14 In situations where it is desirable to provide a higher level of guidance to bicyclists and motorists, shared
15 lanes may be marked with a pavement marking symbol (see Exhibit 4.4). The symbol, known as the
16 shared lane marking, is useful in locations where there is insufficient width to provide bike lanes. The
17 marking also alerts road users to the lateral position bicyclists are likely to occupy within the traveled
18 way, therefore encouraging safer passing practices (including changing lanes, if necessary). Shared lane
19 markings may also be used to reduce the incidence of wrong-way bicycling.

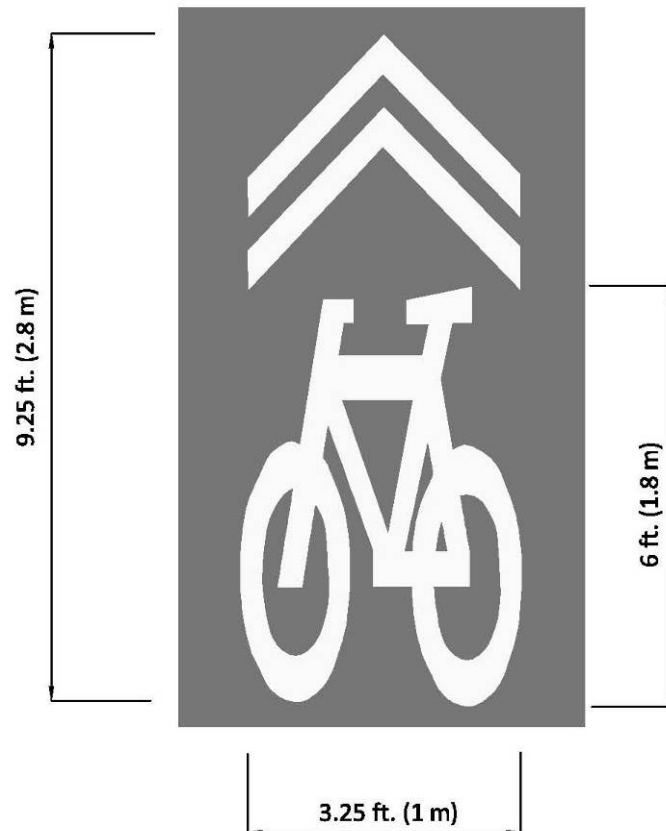
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Exhibit 4.4. Shared Lane Marking

Shared lane markings may be applicable in the following scenarios:

- In a shared lane with adjacent on-street parallel parking, to assist cyclists with lateral positioning that reduces the chance of a bicyclist impacting the open door of a parked vehicle.
- On wide outside lanes, to indicate safer positioning away from the curb or edge of roadway.
- On a section of roadway with shared lanes, to fill a gap between two sections of roadway that have bike lanes, or to fill a gap between a shared use path and a nearby destination, or other similar connections.
- On a section of roadway where the lanes are too narrow for a bicyclist and motorist to travel side by side in the lane.
- On a downgrade section of roadway where there is room for only one bike lane. In these situations, a bike lane should be used on the upgrade section due to the bicyclist's slower operating speed moving uphill.
- At multi-lane intersections where there is insufficient width to provide a bike lane, and conflicts make it desirable to indicate proper positioning.

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Shared lane markings are not appropriate on paved shoulders or in bike lanes, and should not be used on roadways that have a speed limit above 35 mph (50 km/h). Shared lane markings should be placed immediately after an intersection and spaced at intervals not greater than 250 feet (76 m) thereafter.

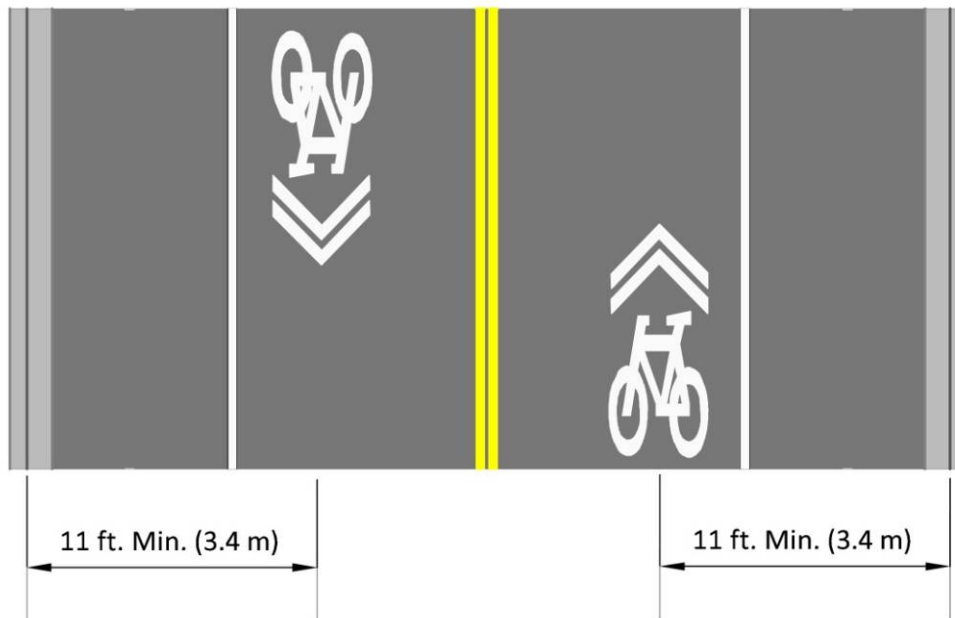
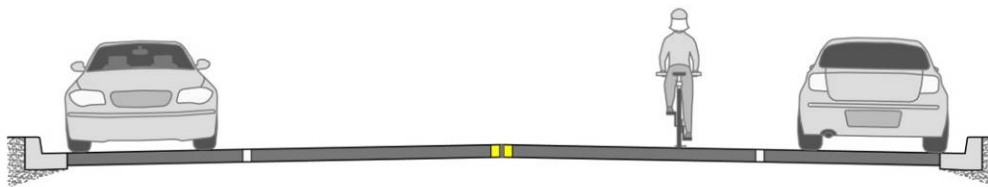
Shared lane markings should be marked on an alignment that represents a practical path of bicycle travel under typical conditions. For some streets, this may be the center of a shared travel lane. On a one-way street designated as a bicycle route, where the bicycle route makes a left turn, it may be appropriate to place shared lane markings on both the outside right and left lanes of the street.

The following provides guidance on shared lane marking placement (all values given are to the center of the marking):

- On streets with on-street parallel parking, shared lane markings should be placed at least 11 feet (3.4 m) from the face of curb (inclusive of gutter), or edge of pavement where there is no curb (see Exhibit 4.5).
- On streets without on-street parallel parking, shared lane markings should be placed at least 4 feet (1.2 m) from the face of curb (inclusive of gutter), or edge of pavement where there is no curb (see Exhibit 4.6).

The MUTCD (1) contains further guidance on shared lane markings.

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1

2 *Exhibit 4.5. Typical Shared Lane Marking Cross Section on Street with Parking*

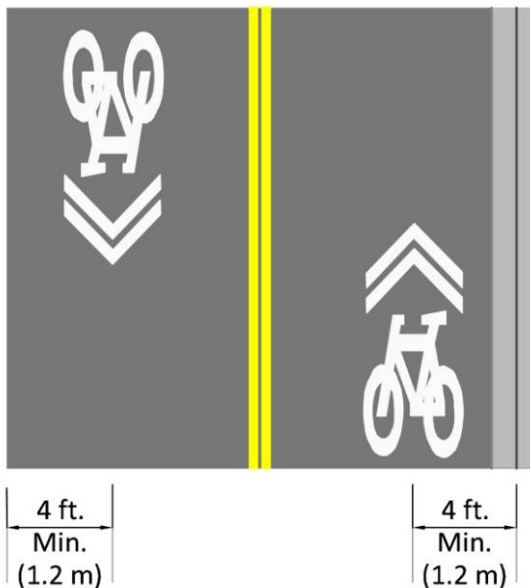
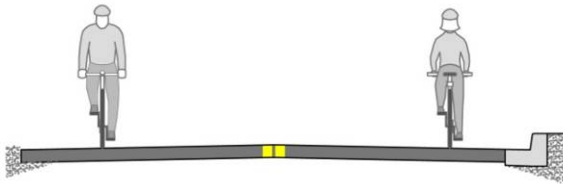
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Exhibit 4.6. Typical Shared Lane Marking Cross Section on Street with No On-Street Parking

4.5. PAVED SHOULDERS

Adding or improving paved shoulders can greatly improve bicyclist accommodation on roadways with higher speeds or traffic volumes, as well as benefit motorists (as described in the AASHTO Green Book). As described in Chapter 2, paved shoulders are most often used on rural roadways. Paved shoulders extend the service life of the road by reducing edge deterioration, and provide space for temporary storage of disabled vehicles. Paved shoulders can benefit pedestrians as well by providing a place for them to walk in locations where there is no sidewalk and the roadside is unsuitable for walking.

It is important to understand the differences between paved shoulders and bike lanes, particularly when a decision must be made as to which facility is more appropriate for a given roadway. Bike lanes are travel lanes, whereas in many jurisdictions, paved shoulders are not (and can therefore be used for parking). Paved shoulders typically stay to the right of right turn lanes at intersections, whereas bike

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lanes are placed on the left side of right-turn lanes in order to encourage merging movements in advance of the intersection. To avoid conflicts on roadways with paved shoulders that approach right-turn lanes, some jurisdictions introduce a bike lane only at the intersections, and then transition back to a paved shoulder.

For any given roadway, the determination of the appropriate shoulder width should be based on the roadway's context and conditions in adjacent lanes. On uncurbed cross sections with no vertical obstructions immediately adjacent to the roadway, paved shoulders should be at least 4 feet (1.2 m) wide to accommodate bicycle travel. Shoulder width of at least 5 feet (1.5 m) is recommended from the face of a guardrail, curb, or other roadside barrier to provide additional operating width, as cyclists will shy from a vertical face. It is desirable to increase the width of shoulders where higher bicycle usage is expected. Additional shoulder width is also desirable if motor vehicle speeds exceed 50 mph (80 km/h), if use by heavy trucks, buses, or recreational vehicles is considerable, or if static obstructions exist at the right side of the roadway. The BLOS model may be used to determine the appropriate shoulder width (see Chapter 2: Planning).

It is preferable to provide paved shoulders on both sides of two-way roads. In constrained locations where pavement width is limited, it may be preferable to provide a wider shoulder on only one side of the roadway, rather than to provide a narrow shoulder on both sides. This may be beneficial in the following situations:

- On uphill roadway sections, a shoulder may be provided to give slow-moving bicyclists additional maneuvering space, thereby reducing conflicts with faster moving motor vehicle traffic.
- On roadway sections with vertical or horizontal curves that limit sight distance, it can be helpful to provide shoulders over the crest and on the downgrade of a vertical curve, and on the inside of a horizontal curve.

For information on retrofitting paved shoulders onto existing roadways, see Section 4.9.

Where an unpaved driveway meets a roadway or pathway, it is advisable to pave some portion of the driveway approach to prevent loose gravel from spilling onto the travel way or shoulder. Paving at least 10 feet (3 m) on (low-volume) driveway connections, and 30 feet (9 m) or to the right-of-way line, whichever is less, on unpaved public road connections, can mitigate the worst effects of loose gravel. Where practical, the paved section of the approach to the highway should be sloped downward away from the highway to reduce the amount of loose material tracked into the shoulder.

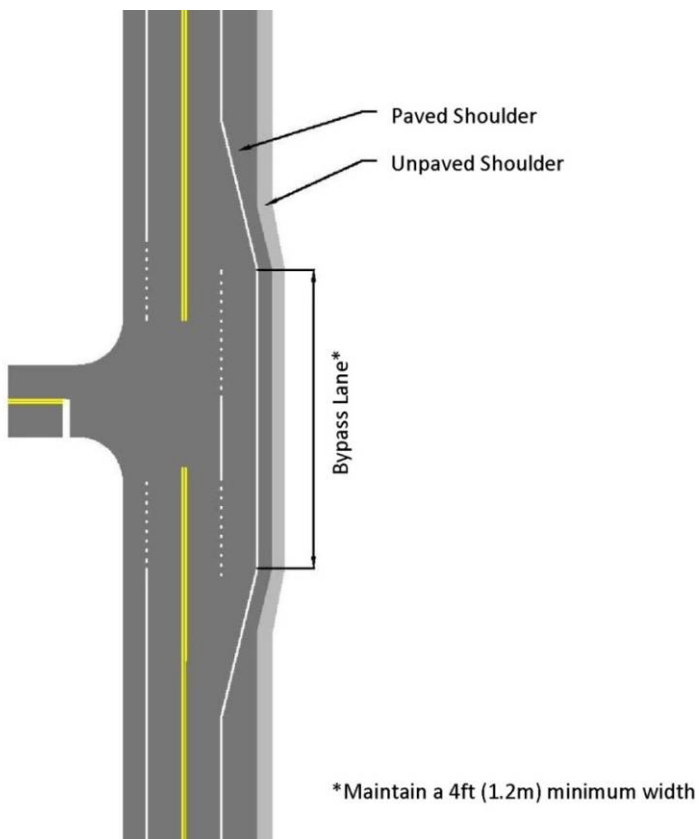
Raised pavement markers (also known as pavement reflectors) can have a detrimental effect on bicycling when placed along a shoulder or bike lane line, as they can deflect a bicycle wheel, causing a loss of control. If pavement markers are required, consideration should be given to installing the

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- 1 markers on the travel lane side of the edge line, and the marker should have beveled or non-abrupt
2 edges.

3 4.5.1. SHOULDER BYPASS LANES

- 4 It is becoming a common design practice to incorporate bypass lanes at T-intersections of two-lane
5 roadways, so as to facilitate the passing of motorists stopped to make left turns onto side roads. Where
6 this is done on a highway with paved shoulders, at least 4 ft (1.2 m) of shoulder pavement should be
7 carried through the intersection along the outside of the bypass lane. This is especially critical on
8 roadways with high volumes and operating speeds. An example of a preferred bypass lane treatment
9 with a continuous shoulder usable by bicyclists is shown in Exhibit 4.7.



11 **Exhibit 4.7. Shoulder Bypass Lane**

DRAFT FOR AASHTO COMMITTEE REVIEW AND COMMENT**4.5.2. RUMBLE STRIPS**

Longitudinal rumble strips can provide a safe and inexpensive way to reduce run-off-road crashes for motorists on high-speed roadways. However, they can be hazardous for bicyclists and can render popular and useful bicycle routes unridable. The effect of some rumble strip designs on bicyclists can be significant; they cause the bicycle to shudder violently, and therefore bicyclists avoid them. If rumble strips are located along the right edge of a roadway with a narrow shoulder or no shoulder space, cyclists will be required to share the travel lane with motorists.

Rumble strips are not recommended on shoulders used by bicyclists unless there is a minimum clear path of 4 feet (1.2 m) from the rumble strip to the outside edge of paved shoulder, or 5 feet (1.5 m) to the adjacent curb or other obstacle. If existing conditions preclude achieving the minimum desirable clearance, the width of the rumble strip may be decreased or other alternative solutions considered. Placing a rumble strip under the edge line is one way to reduce its impact on the adjacent shoulder, while providing the additional advantage of increasing the visibility of the edge line under dark conditions.

Periodic gaps in rumble strips should be provided to allow bicyclists to move across them as necessary (e.g., to avoid debris in the shoulder, pass other cyclists, make left turns, etc.). Gaps spaced at intervals of 40 to 60 feet (12 to 18 m) provide such opportunities. A gap length of at least 12 feet (3.7 m) will allow most bicyclists to leave or enter the shoulder without crossing the rumble strip, as shown in Exhibit 4.8.

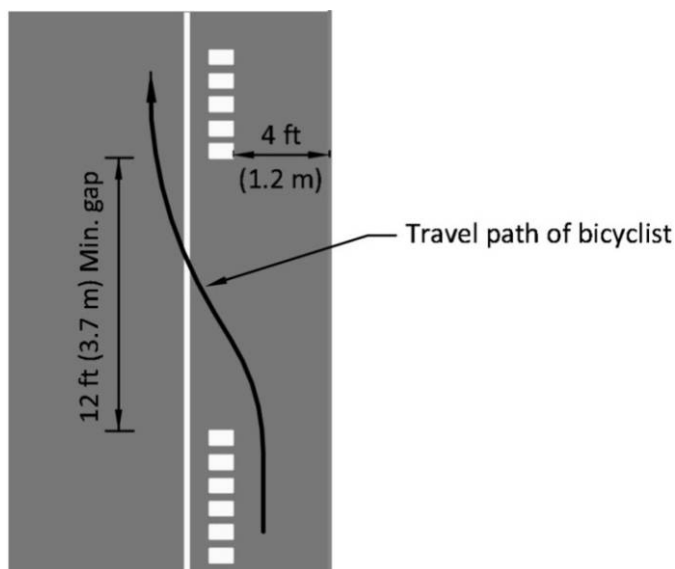


Exhibit 4.8. Rumble Strips

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In addition to periodic gaps, rumble strips should be milled and their dimensions should be adjusted to provide a more bicycle-tolerable design, as follows:

- Width: 5 inches (127 mm),
- Depth: 0.375 inches (10 mm), and
- Spacing: 11 to 12 inches (280 to 305 mm) (2)

Where it is necessary to limit the length of rumble strips to ensure adequate shoulder space for bicyclists, the length can be reduced to a minimum of 6 inches (152 mm). (2) In areas not prone to snow removal activity, an inverted profile (audible-vibratory) edge line marking can also be used as a more bicycle-friendly alternative to rumble strips.

Centerline rumble strips are used to prevent head-on collisions; however their presence is problematic for bicyclists. On a two-lane highway without paved shoulders, they discourage motorists from crossing the centerline to pass bicyclists with appropriate clearance. If use of centerline rumble strips is deemed necessary due to a history of head-on collisions, the dimensions for shoulder rumble strips described above should be used. In addition, the use of an inverted-profile (audible-vibratory) centerline marking is more conducive should motorists need to cross the centerline to pass bicyclists.

4.6. BICYCLE LANES

4.6.1. GENERAL CONSIDERATIONS

Bicycle lanes are a portion of the roadway designated for preferential use by bicyclists. They are one-way facilities that typically carry bicycle traffic in the same direction as adjacent motor vehicle traffic. Bike lanes are the appropriate and preferred bicycle facility for thoroughfares in both urban and suburban areas. Where desired, or where there is a high potential for bicycle use, bike lanes may be provided on rural roadways near urban areas. Paved shoulders can be designated as bike lanes by installing bike lane symbol markings (see Exhibit 4.9); however, a shoulder marked as a bike lane will still need to meet the criteria listed elsewhere in this chapter.

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Exhibit 4.9. Example of Paved Shoulder Designated as Bicycle Lane (photo by Michael Moule)

Bicycle lanes are used to delineate available road space for preferential use by bicyclists and to facilitate more predictable movements by bicyclists and motorists. Bike lanes enable bicyclists to ride at their preferred speed, even when adjacent traffic speeds up or slows down. Bike lanes also encourage bicyclists to ride on roadways in a position where they are more likely to be seen by motorists entering or exiting the roadway than they would be while riding on sidewalks. Properly designed bike lanes encourage bicyclists to operate in a manner consistent with the legal and safe operation of all vehicles. Bike lanes should follow travel paths that lawfully operating bicyclists would take to travel in their intended direction within the roadway cross section. Bike lanes are not intended to accommodate all bicycle use on a roadway; bicyclists may leave a bike lane to pass other bicyclists, make left turns or right turns, avoid debris or other hazards, or to pass buses momentarily stopped in the bicycle lane.

Raised pavement markings, raised curbs, and other raised devices can cause steering difficulties for bicyclists and should not be used to separate bike lanes from adjacent travel lanes.

Bike lanes should have a smooth riding surface. Utility covers should be adjusted flush with the surface of the lane. Bike lanes should be provided with adequate drainage (bicycle-safe drain grates) to prevent ponding, washouts, debris accumulation, and other potential hazards for bicyclists. In addition, other roadway features should be compatible for bicycling. See Section 4.12 for more information on this topic.

State laws should be considered when implementing bike lanes, as they may have an impact on bike lane design, such as the placement of dashed lane lines. Motorists are prohibited from using bike lanes for driving and parking, but many state vehicle codes allow or direct drivers to use bike lanes while turning or merging, maneuvering into or out of parking spaces, and for emergency avoidance maneuvers or breakdowns. Some state codes also allow buses, garbage collectors, and other public vehicles to use bike lanes temporarily.

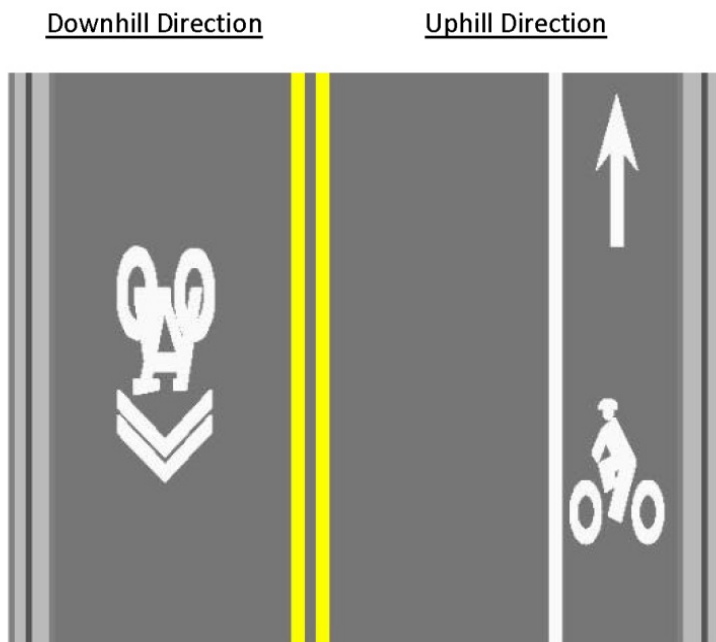
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1 For information on retrofitting bike lanes onto existing streets, see Section 4.9.

2 **4.6.2. BICYCLE LANES ON TWO-WAY STREETS**

3 In most cases, bike lanes should be provided on both sides of two-way streets. A bicycle lane provided
4 on only one side may invite wrong-way use.

5 Exceptions can be made on streets with an appreciable grade. On streets where downhill grades are
6 long enough to result in bicycle speeds similar to typical motor vehicle speeds, then a bicycle lane may
7 be provided only in the uphill direction, with shared lane markings in the downhill direction (see Exhibit
8 4.10). This design can be especially advantageous on streets where fast downhill bicycle speeds have
9 the potential to increase the likelihood of crashes with fixed objects, particularly in locations with on-
10 street parking.



11

12 ***Exhibit 4.10. Shared Lane Marking and Bike Lane on Steep Street***

DRAFT FOR AASHTO COMMITTEE REVIEW AND COMMENT**4.6.3. BICYCLE LANES ON ONE-WAY STREETS**

On one-way streets, bike lanes should normally be on the right-hand side of the roadway. A bicycle lane may be placed on the left if there are a significant number of left turning bicyclists or if a left-side bike lane decreases conflicts, for example those caused by heavy bus traffic, heavy right-turn movements (including double right-turn lanes), deliveries, or on-street parking.

Bike lanes should typically be provided on both streets of a one-way couplet in order to provide facilities in both directions and discourage wrong-way riding. If width constraints or other conditions make it impracticable to provide bike lanes on both streets, shared lane markings should be considered on the constrained street. This provides a more complete network and encourages bicyclists to travel with the flow of other traffic.

On streets designated for one-way operation, it is sometimes desirable to provide an exception for bicyclists by marking a contra-flow bicycle lane on the appropriate side, separated by a yellow centerline marking. This may be considered in situations where it would provide substantial savings in out-of-direction travel and/or direct access to high-use destinations, and/or where there will be fewer conflicts when compared to a route on other streets. This design is best used where there are few intersecting driveways, alleys, or streets on the side of the street with the contra-flow lane, and where bicyclists can safely and conveniently make transitions at the termini of the contra-flow lane (see Exhibit 4.11).

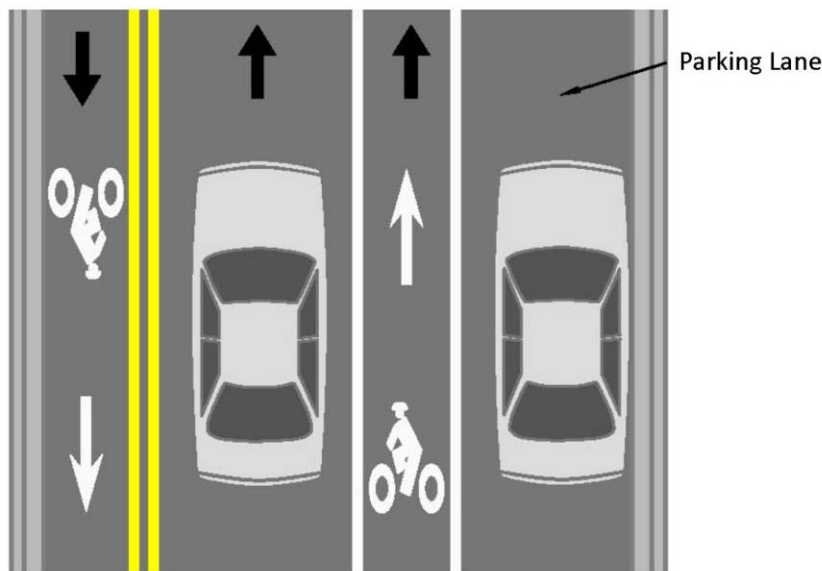


Exhibit 4.11. Typical Markings for One Way Street Designed for Two Way Bicycle Travel

For a bicycle lane to function as intended when built against the dominant flow of traffic on a one-way street, the following features should be incorporated into the design:

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- The bicycle lane should be placed on the correct side of the roadway (i.e. the right-hand side, from the perspective of the bicyclist traveling in the contra-flow direction; or on the left-hand side from the motorist's perspective).
- A bike lane should be provided for bicyclists traveling in the same direction as motor vehicle traffic. If there is insufficient room to provide a bike lane in the dominant-flow direction of the street, shared lane markings should be considered to emphasize that bicyclists must share the travel lane on this side of the street.
- Whether on-street parallel parking can be provided on the side of the street with the contra-flow bike lane depends on the nature of the street and of the parking. Given their position on the left side of the vehicle, motorists leaving a parking space will have difficulty seeing oncoming bicyclists in the contra-flow bicycle lane, as sight lines may be blocked by other parked vehicles. Where traffic volumes and parking turnover is light, and traffic speeds low, on-street parking may not cause conflicts. If a parking lane is provided on the side of the street with the contra-flow bike lane, bike lane lines should be provided on both sides of the bike lane.
- Bike lane symbols and directional arrows should be used on both the approach and departure of each intersection, to remind bicyclists to use the bike lane in the appropriate direction, and to remind motorists to expect two-way bicycle traffic.
- Appropriate separation must be placed between the two directions of traffic to designate travel lanes in both directions:
 - Pavement markings are the simplest form of separation and should consist of two solid yellow lines, the standard centerline marking where passing (across the centerline) is prohibited in both directions.
 - Medians or traffic separators provide more separation between motorists and bicyclists traveling in opposing directions. This treatment should be considered in situations with higher speeds or volumes. If medians or traffic separators are used, the contra-flow bike lane width should be at least 7 feet (2.1 m).
- At intersecting streets, alleys, and major driveways, DO NOT ENTER signs and turn restriction signs should include supplemental plaque that says EXCEPT BICYCLES, to establish that the street is two-way for bicyclists and to remind motorists to expect two-way bicycle traffic.
- At traffic signals, signal heads should be provided for contra-flow cyclists, as well as suitable bicycle detection measures. A supplemental plaque that says BICYCLE SIGNAL may be needed beneath the signal to clarify its purpose.

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4.6.4. BICYCLE LANE WIDTHS

Bike lane widths should be determined by context and anticipated use. The speed, volume, and type of vehicles in adjacent lanes significantly affect bicyclists' comfort and desire for lateral separation from other vehicles. Bicycle lane widths should be measured from the center of the bicycle lane line. The appropriate width should take into account design features at the right edge of the bike lane, such as the curb, gutter, on-street parking lane, or guardrail.

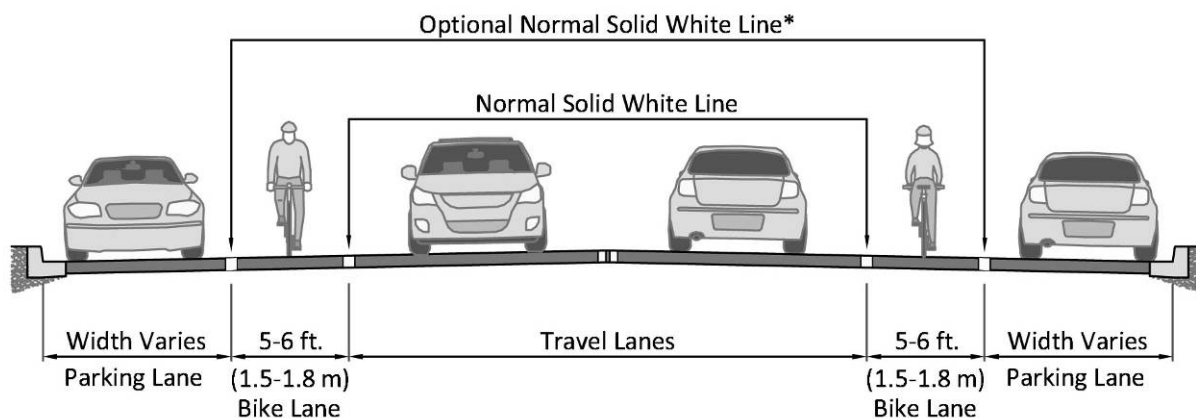
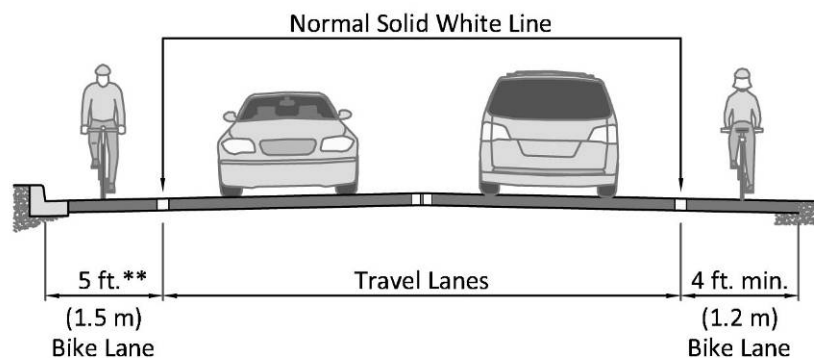
Exhibit 4.12 shows two typical locations for bike lanes in relation to the rest of the roadway, and the widths associated with these facilities.

As discussed in the previous chapter, a bicyclist's preferred operating width is 5 feet (1.5 m). Therefore, under most circumstances the recommended width for bike lanes is 5 feet (1.5 m). Wider bicycle lanes may be desirable under the following conditions:

- Adjacent to a narrow parking lane (7 feet [2.1 m]) with high turnover (such as those servicing restaurants, shops, or entertainment venues), a wider bicycle lane (6-7 feet or 1.8-2.1 m) provides more operating space for bicyclists to ride out of the area of opening vehicle doors.
- In areas with high bicycle use, a bike lane width of 6 to 8 feet (1.8-2.4 m) makes it possible for bicyclists to ride side-by-side or pass each other without leaving the lane.
- On high-speed (greater than 45 mph [70 km/h]) and high-volume roadways, or where there is a substantial number of heavy vehicles, a wide bicycle lane provides additional lateral separation between motor vehicles and bicycles to minimize wind blast and other effects.

When wider bike lanes are provided, adequate marking or signing should be used so the lanes are not mistaken for motor vehicle travel lanes or parking areas.

For roadways with no curb and gutter and no on-street parking, the minimum width of a bike lane is 4 feet (1.2 m).

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* The optional normal (4 in--6 in/100-150 mm) solid white line may be helpful even when no stalls are marked (because parking is light), to make the presence of a bicycle lane more evident. Parking stall markings may also be used.

** On extremely constrained, low-speed roadways with curbs but no gutter (e.g. in locations with stone curbs), where the preferred bike lane width cannot be achieved despite narrowing all other travel lanes to their minimum widths, a 4-foot (1.2 m) wide bike lane can be used.

1

2 **Exhibit 4.12. Typical Bike Lane Cross Sections**

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For roadways where the bike lane is immediately adjacent to a curb, guardrails, or other vertical surface, the minimum bike lane width is 5 feet (1.5 m), measured from the face of a curb or vertical surface to the center of the bike lane line. There are two exceptions to this:

- In locations with higher motor vehicle speeds where a 2-foot (0.6 m) wide gutter is used, the preferred bike lane width is 6 feet (1.8 m), inclusive of the gutter.
- On extremely constrained, low-speed roadways with curbs but no gutter (e.g. in locations with stone curbs), where the preferred bike lane width cannot be achieved despite narrowing all other travel lanes to their minimum widths, a 4-foot (1.2 m) wide bike lane can be used.

Drain inlets and utility covers are sometimes built so they extend past the longitudinal gutter joint. Drain inlets and utility covers that extend into the bike lane may cause bicyclists to swerve, and have the effect of reducing the usable width of the lane. This is a particular problem if the minimum operating width of the lane falls below 3 feet (0.9 m). Therefore the width of the bike lane should be adjusted accordingly, or else the structure should be removed or relocated (see Section 4.12.8).

4.6.5. BICYCLE LANES AND ON-STREET PARKING

When on-street parking is permitted, the bicycle lane should be placed between the parking lane and the travel lane (see Exhibit 4.13). The recommended bicycle lane width in these locations is 6 feet (1.8 m) and the minimum bicycle lane width is 5 feet (1.5 m). Care should be taken when providing wider bike lanes in areas where parking is scarce or otherwise in demand, as wider bicycle lanes may result in more double parking.

Bike lanes at the same level as the street and without physical separation should generally not be placed between the parking lane and the curb. Such placement reduces visibility at driveways and intersections, increases conflicts with opening car doors, complicates maintenance, and prevents bicycle lane users from making vehicular left turns.

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Exhibit 4.13. Example of Bike Lane Adjacent to Parallel Parking (photo by Toole Design Group)

PARALLEL PARKING

Where bicycle lanes are installed adjacent to parallel parking, the recommended width of a marked parking lane is 8 feet (2.4 m), and the minimum width is 7 feet (2.1 m). Where parallel parking is permitted but a parking lane line or stall markings are not utilized, the recommended width of the shared bicycle and parking lane is 13 feet (4 m). A minimum width of 12 feet (3.7 m) may be satisfactory if parking usage is low and turnover is infrequent.

In general it is the legal responsibility of motorists to check for oncoming traffic before opening a car door into the traveled way. In some urban areas, bicyclists have been seriously injured in crashes with car doors that are suddenly swung open by inattentive drivers and passengers. This type of crash is more prevalent in locations with high parking turnover, such as main streets, commercial streets with restaurants and retail businesses, or similar areas. Bicyclists can avoid this type of crash by riding on the left side of a bicycle lane, outside the range into which opened doors of parked vehicles could extend. Several communities employ markings to encourage cyclists to ride further from parked cars, such as providing a wider parking lane, a wider bike lane, or a striped buffer between the parking lane and the bike lane. Parking “Ts” extending into the bike lane and slightly narrower bike lane symbols placed on the left side of the bike lane may encourage bicyclists to ride in a safer location.

DIAGONAL PARKING

In areas with high parking demand and sufficient street width, diagonal parking is sometimes used to increase parking capacity and reduce travel speeds on streets that are excessively wide. Bicycle lanes should normally not be placed adjacent to conventional front-in diagonal parking, since drivers backing out of parking spaces have poor visibility of bicyclists in the bicycle lane.

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Exhibit 4.14. Example of Bike Lane Adjacent to Back-in Diagonal Parking (photo by Toole Design Group)

The use of back-in diagonal parking (see Exhibit 4.14) can help mitigate the conflicts normally associated with bike lanes adjacent to angled parking. There can be numerous benefits to back-in diagonal parking for all roadway users:

- Improved sight distance between exiting motorists and other traffic compared to parallel parking or front-in angled parking.
- No conflict between bicyclists and open car doors.
- Easier loading/unloading of vehicles.
- Passengers (including children) are naturally channeled toward the curb when alighting. Loading and unloading of the trunk occurs at the curb, not in the street.

When bike lanes are placed adjacent to back-in diagonal parking spaces, parking bays should be long enough to accommodate most expected vehicles.

4.7. BICYCLE LANE SIGNS AND MARKINGS

Bicycle lanes are designated for preferential use by bicyclists with a normal solid white line (4 to 6-inch or 100-150mm wide) and one of the (two) standard bike lane symbol markings, which may be supplemented with the directional arrow marking. Optional bike lane signs may be used to supplement the pavement markings.

Standards and guidance for applying these elements can be found in the MUTCD (1). Supplemental guidance is provided below.

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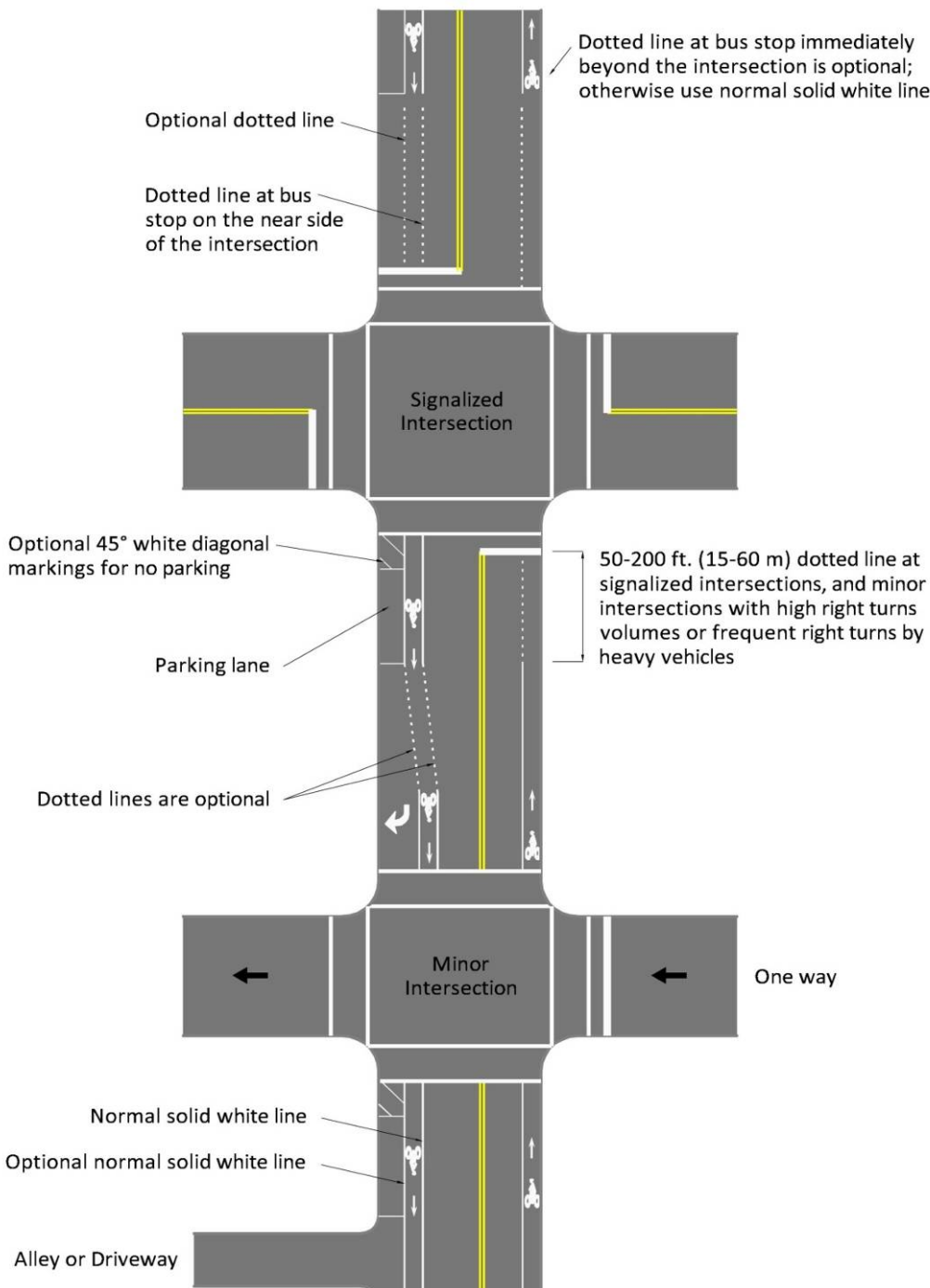
4.7.1. BICYCLE LANE LINES

A bike lane should be delineated from the motor vehicle travel lanes with a normal solid white line. Bike lane lines can be dotted at locations where there will be frequent merging activity by bicyclists or motorists across the lane line. Details about using dotted lines at intersections are provided in Section 4.8. Bike lanes can also be dotted at bus stops or bus pullouts. Bicycle lane lines should remain solid and not dotted at unsignalized driveways and alleys (see Exhibit 4.15).

Raised pavement markers, curbs, posts, or barriers should not be used to separate bicycle lanes from adjacent travel lanes. Raised devices are hazardous to bicyclists because they are fixed objects immediately adjacent to the travel path of the bicyclist. In addition, raised devices can discourage or prevent right-turning motorists from merging into the bicycle lane before turning. Raised devices can also make it more difficult to maintain the bicycle lane.

A normal solid line can be used to indicate the outside edge of the bike lane in locations with no curbs or where the edge of the roadway is poorly defined.

Where a bicycle lane is adjacent to a parking lane, the parking area should be defined by parking space “T” markings or a normal solid white line. Such markings encourage parking closer to the curb and can help make clear, during times of low parking usage, that the parking lane and bicycle lane are not a travel lane. More information on bike lanes adjacent to on-street parking can be found in Section 4.6.5.

DRAFT FOR AASHTO COMMITTEE REVIEW AND COMMENT***Exhibit 4.15. Typical Bike Lane Pavement Markings***

Striped buffers may be used to provide increased separation between a bike lane and another adjacent lane that may present conflicts, such as a parking lane with high-turnover or a higher speed travel lane. The benefits of additional lateral separation should be weighed against the disadvantages; a buffer

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1 between the bike lane and the adjacent motor vehicle travel lanes places cyclists further from the
2 normal sight lines of motorists, who are primarily looking for vehicles in the normal travel lanes, and
3 buffers between the travel lane and bike lane reduce the natural “sweeping” effect of passing motor
4 vehicles, potentially requiring more frequent maintenance.

4.7.2. BICYCLE LANE MARKINGS

6 As detailed in the MUTCD (1), a bike lane should be marked with standard bike lane markings to inform
7 bicyclists and motorists of the restricted nature of the bike lane. Markings should be placed after each
8 intersection or signalized driveway. Supplementary markings may also be placed in a visible location on
9 a bike lane that is entering the intersection (prior to the crosswalk), to remind bicyclists not to enter the
10 bike lane on the wrong side of the road. However, in urban areas with short block lengths, this may
11 result in an overabundance of bicycle lane markings. In general, due to the complexity of urban streets,
12 flexibility is necessary in placing bicycle lane markings.

13 Additional markings may be placed at periodic intervals on bicycle lanes, to remind motorists of the
14 potential presence of bicyclists, especially in areas where motorists are expected to cross bike lanes. In
15 suburban areas with long distances between intersections and little roadside activity, bike lane symbols
16 can be as far apart as 1000 feet (305 m) or more. In urban areas where motorists make parking
17 maneuvers across bike lanes or where there is significant driveway density, it may be appropriate to
18 space the symbols as often as every 100 feet (30 m).

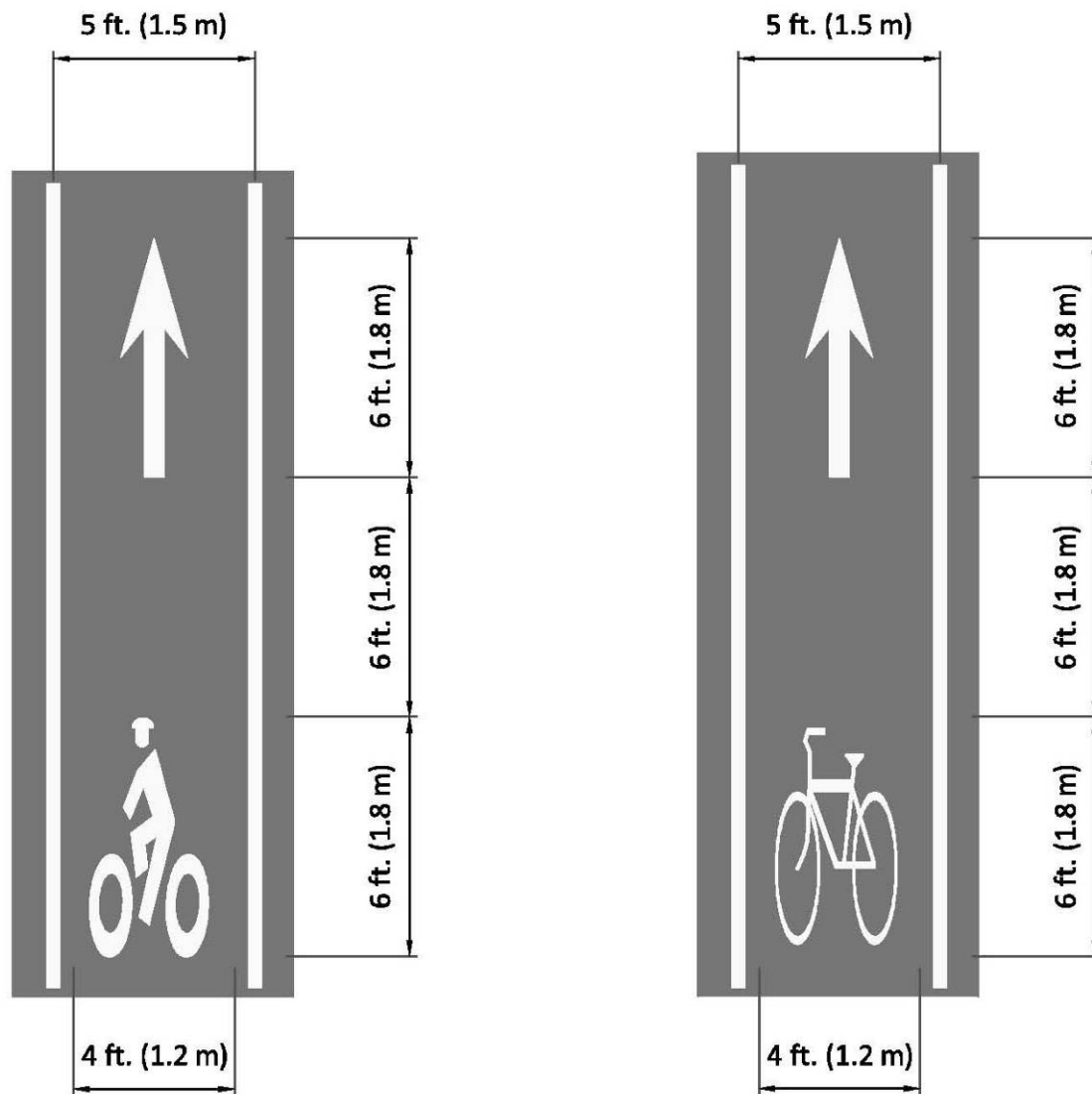
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Exhibit 4.16. Bike Lane Symbol Markings

The MUTCD (1) allows one of the two standard bicycle lane symbol markings (or the words “BIKE LANE”) and a directional arrow as shown in Exhibit 4.16. All bicycle lane markings should be white and retroreflective.

Care should be taken to avoid placing symbols in areas where turning motor vehicles would damage or obliterate the markings, e.g. at driveways and the area immediately adjacent to an intersection (Exhibit 4.17).

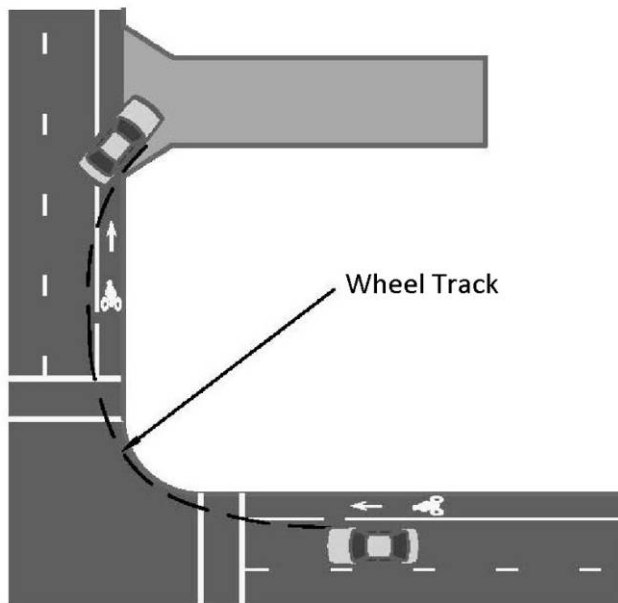
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Exhibit 4.17. Example of Symbol Placement to Avoid Premature Wear

4.7.3. BICYCLE LANE SIGNS

Due to the cluttered nature of the roadside in most urban areas, which reduces the effectiveness of signs, bicycle lane markings are typically the primary indication to motorists and bicyclists of the restricted nature of bike lanes. Signs may be used to supplement bicycle lane lines and markings; however they are less effective on streets with on-street parking.

The standard BIKE LANE (R3-17) sign (see Exhibit 4.18) with the AHEAD (R3-17aP) plaque may be placed in advance of the start (upstream end) of a bicycle lane. The BIKE LANE sign with the ENDS (R3-17bP) plaque should be placed at a sufficient distance to give warning to the bicyclist that the lane is ending. The BIKE LANE ENDS sign should not be used where a bike lane changes to an unmarked shoulder, for example at the urban or suburban fringe, or at temporary interruptions in a bike lane.

BIKE LANE signs may also be placed as needed at periodic intervals along a bicycle lane. Spacing of the sign should be determined by engineering judgment based on prevailing speed of bicycle and other traffic, block length, distances from adjacent intersections, and other considerations. Bike lane markings are typically used more frequently than BIKE LANE signs. Where the BIKE LANE sign is used, however, it should generally be placed adjacent to a bike lane pavement marking.

DRAFT FOR AASHTO COMMITTEE REVIEW AND COMMENT**Exhibit 4.18. Bike Lane Sign**

If the installation of signs is necessary to reduce the instances of parking, standing, or stopping in a bicycle lane, the NO PARKING BIKE LANE signs (R7-9 or R7-9a) or other signs restricting parking or stopping should be installed.

4.8 BICYCLE LANES AT INTERSECTIONS

Most conflicts between bicyclists and motor vehicles occur at intersections and driveways. The risk of crossing-path conflicts is increased because bicyclists are generally less conspicuous than motor vehicles and tend to ride along the periphery of the main traffic paths on which motorists concentrate their attention while navigating intersections.

Good intersection design clearly indicates to bicyclists and motorists how they should traverse the intersection and generally adheres to the following principles:

- Free-flowing and high-speed turning movements by motor vehicles should be avoided.
- Adequate lighting should be provided to illuminate all users.
- The design should enable the bicyclist's route through the intersection to be direct, logical, and similar to the path of motor vehicle traffic.
- Actuated signals should be designed to detect the presence of bicycles.
- Signal green intervals and clearance intervals should be sufficient to allow cyclists to reach the far side of the intersection.
- Signals should be timed so they do not impede bicyclists with excessively long waits.
- Access management practices should be used to remove excessive conflict points.

Guidance on signal timing and bicycle detection is provided in Sections 4.12.3 and 4.12.4. Bike lanes are not normally striped through the middle of intersections; however, where extra guidance is needed it may be appropriate to use a dotted extension line to guide bicyclists through an undefined area.

Compact intersections where roads meet at (or nearly at) right angles are most functional for cyclists. Acute-angle intersections with three or four legs are less desirable, because some turning movements

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can be made at higher speeds, which creates conflicts with bicyclists traveling straight. Also, trucks turning on obtuse angles have blind areas on their right sides. However, the presence of an acute-angle intersection along a candidate bicycle route should not disqualify it from designation if no convenient alternative route is available. Acute-angle intersections are often found in older built-up areas where diagonally intersecting streets often provide the most direct and practical bicycle access to destinations.

Various practices are used to improve the functionality of acute-angle intersections:

- Approaches can be realigned, as described in the AASHTO Green Book.
- An intersection with more than four legs can be reconfigured so that only two roads cross, by closing a minor approach or by offsetting it to a new nearby minor intersection.
- Dotted bicycle lane extension lines can be used to guide bicyclists through long, undefined areas at large, skewed, or multi-leg intersections.
- A complex intersection can sometimes be converted to a roundabout.

4.8.1. RIGHT TURN CONSIDERATIONS

Right turns are relatively easy for bicyclists, since they typically ride on the right side of the roadway. On approaches to intersections that do not have right-turn only lanes, bike lane lines are either solid or dotted (see Exhibit 4.15). The choice between solid or dotted lines should be based on several factors, including the volume of right-turning motor vehicles, the speed of motor vehicle traffic, the types of vehicles that typically use the intersection, and the context of the surrounding area (e.g. urban vs. suburban, etc.). For example, dotted lines are more important where there are more right-turning vehicles, or where heavy vehicles frequently turn right. The dotted line is intended to provide a reminder that merging movements can be expected in this area.

State vehicle codes should be consulted as well, as the presence of a solid bicycle lane line at the approach to an intersection may discourage motorists from merging before turning right, as required by law in some states. This can result in conflicts when motorists turn across the path of cyclists.

If a dotted line is used, it should begin 50 to 200 feet (15 to 60 m) prior to the crosswalk (or edge of the intersection if no crosswalk exists). The bike lane line should resume with a solid line on the far side of the intersection (outside crosswalk area).

An intersection designed with large corner radii allows motorists to turn at higher speeds, thus making it more difficult for bicyclists to safely merge left. Corner radii should be as small as practical, but should be large enough to accommodate large vehicles (buses or heavy trucks) that frequently turn right at the intersection.

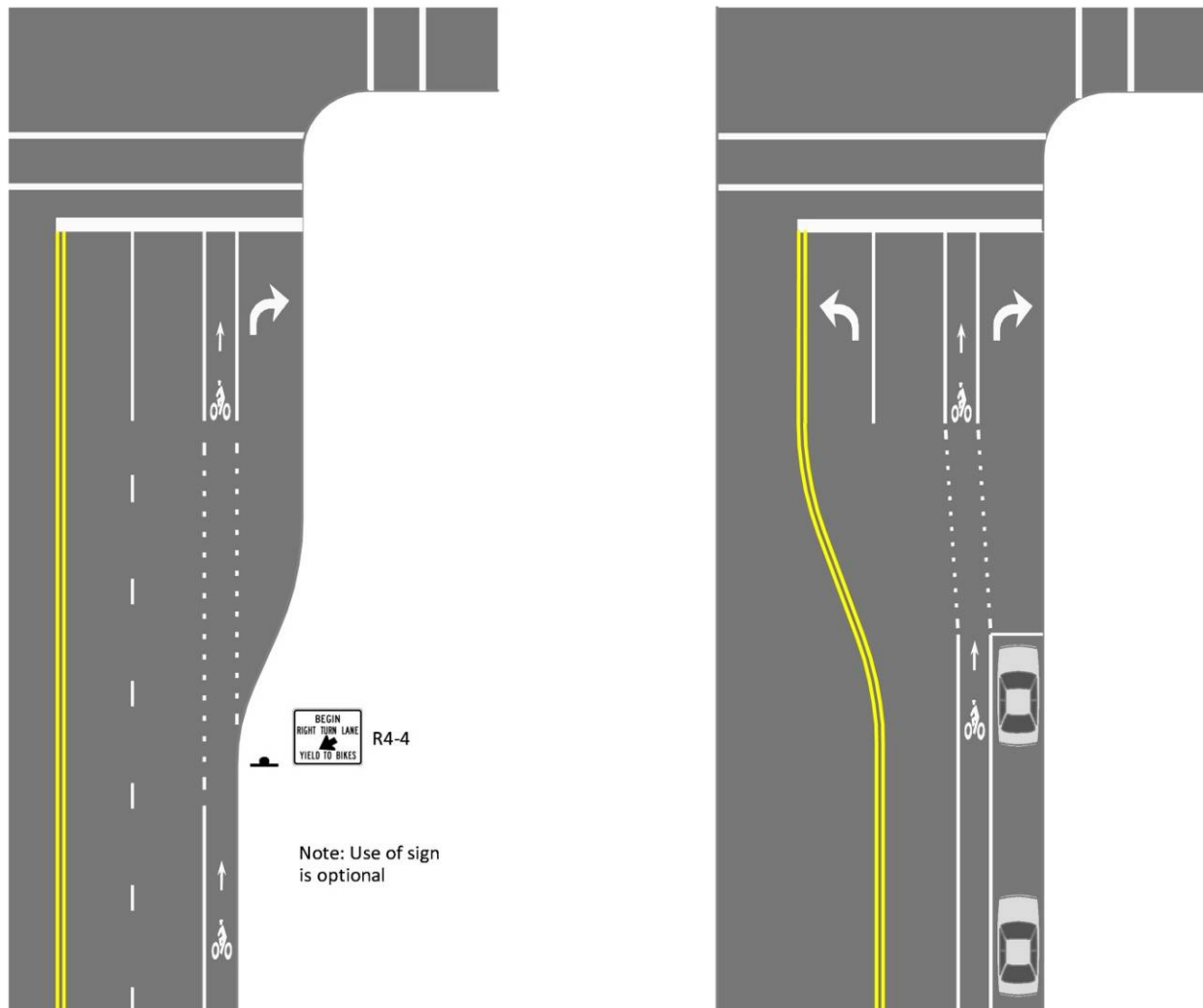
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Exhibit 4.19. Examples of Bike Lanes Approaching Right-Turn Only Lane (with and without parking)

Right-turn only lanes are often used where high volumes of right-turning motor vehicle volumes warrant an exclusive right-turn lane to improve traffic flow. The correct placement of a bike lane is on the left of an exclusive right-turn lane, as shown in Exhibit 4.19. The through bicycle lane should be a minimum of 4 feet (1.2 m) wide, however 5 feet (1.5 m) is preferable to provide comfortable operating space, and to allow use of a full-size bicycle symbol. Bike lane lines should be used on both sides of the lane, per Section 4.7.2.

Incorporating the bike lane to the left of the right-turn only lane enables bicyclists and right-turning motorists to sort their paths by destination in advance of the intersection, avoiding last-moment conflicts and providing the following benefits:

- Bicyclists are encouraged to follow the rules of the road: through vehicles (including bicyclists) proceed to the left of right-turning vehicles.

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- Merging movements occur away from the intersection, and are often easier to manage for bicyclists and other road users than a turning conflict.

Motorists are required to yield to bicyclists at the entrance to the right-turn only lane. The BEGIN RIGHT TURN LANE YIELD TO BIKES (R4-4) sign may be used to remind motorists entering the turn lane of their obligation to yield to bicyclists who are continuing through the intersection in the bike lane (because of the road rule that an operator leaving his lane yields to an operator on a path being entered or crossed).

In situations where a through travel lane becomes a right-turn only lane (see Exhibit 4.2.), bicyclists need to move laterally to weave across the travel lane. Therefore, the bike lane along the curb should be dropped, and a bicycle lane should be introduced on the left side of the right-turn lane. The bike lane line should not be striped diagonally across the travel lane, as this inappropriately suggests to bicyclists that they do not need to yield to motorists when moving laterally. This scenario is the least preferred option and should be avoided where practicable. In this situation, the BEGIN RIGHT TURN LANE YIELD TO BIKES sign should not be used, since bicyclists are the users who need to yield as they are weaving across the path of motor vehicle traffic.

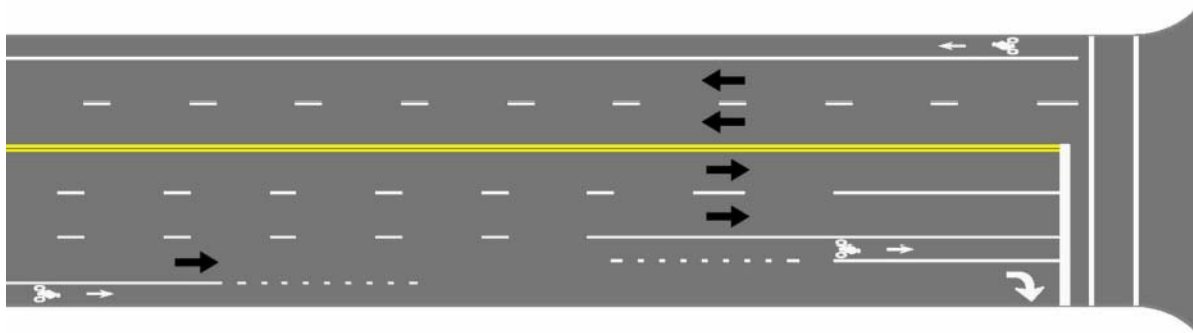


Exhibit 4.20. Example of Bike Lane with Through Lane Transitioning to Right-Turn Only Lane

The use of dual right-turn only lanes should be avoided on streets with bike lanes unless absolutely necessary to accommodate heavy right turn volumes. Where there are dual right-turn only lanes, the bicycle lane should be placed to the left of both right-turn lanes, in the same manner as where there is just one right-turn only lane. On one-way streets with dual right turn lanes, a bike lane on the left-hand side of the road may reduce conflicts and should therefore be considered (see Section 4.6.3).

An optional through right-turn lane next to a right-turn only lane should not be used where there is a through bicycle lane. If a capacity analysis indicates the need for an optional through right-turn lane, the bicycle lane should be discontinued at the intersection approach. It may be possible to eliminate the through right option lane by using other methods of handling the right-turn traffic volume (e.g. two right-turn only lanes as described above, or signal timing and phasing changes like additional green time or a right-turn overlap). An engineering analysis will be needed in order to determine the feasibility of

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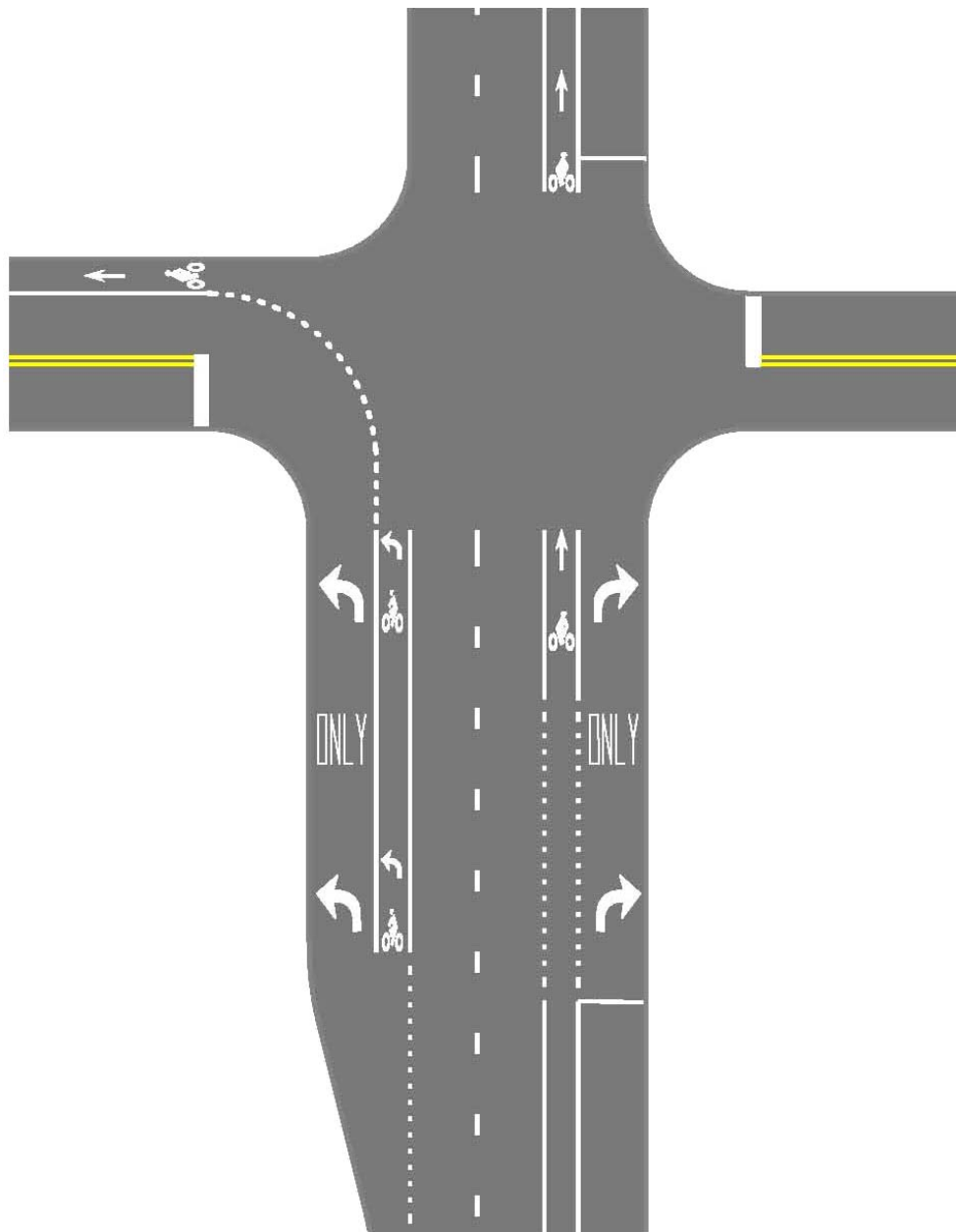
these options. If the lane assignment cannot be changed, shared lane markings may be placed in the center of the through right option lane to provide additional guidance to cyclists who wish to proceed straight.

At locations with heavy right-turn bicycle volumes, it may be appropriate to include a bicycle right-turn lane on the right side of the general right-turn lane. This design should only be considered where turning vehicles will not encroach into the turning bicyclist's path. Wayfinding signage should be provided in advance of the turn lane, so bicyclists can select the appropriate lane. The receiving street should be compatible for bicycling. A through bike lane or shared lane marking should also be included to guide bicyclists who want to continue straight (assuming this is a legal movement).

4.8.2. LEFT TURN CONSIDERATIONS

As described in Chapter 3, there are two methods for bicyclists to make left turns (see Exhibit 3.5). In the first method, the bicyclist merges left in advance of the intersection to turn from the same location as other left-turning vehicles. In the second method, the bicyclist proceeds straight through the intersection, stops on the far side of the intersection (at the corner) and turns the bicycle to the left, and then proceeds across the intersection again on the cross street, or as a pedestrian in the crosswalk. This method is more common in locations with high volumes of motor vehicles, and/or where there are high speeds, because it is more difficult for bicyclists to merge left.

Where there are considerable volumes of left-turning bicyclists, or where a designated or preferred bicycle route makes a left turn, it may be appropriate to provide a separate bicycle left-turn lane (see Exhibit 4.21).



1

2 *Exhibit 4.21. Example of Bike Left-Turn Only Lane*

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1 Separate bicycle left-turn lanes may also be appropriate at intersections of shared-use paths with
2 streets, or at other locations where left turns are allowed for bicyclists but not motorists (e.g. onto a
3 bicycle boulevard). At these locations, bicyclists wanting to turn left from the street system onto the
4 path or bicycle boulevard would otherwise be required to wait for oncoming traffic to clear in the
5 leftmost through travel lane, which is an exposed location that is uncomfortable for bicyclists on busy
6 streets.

7 As described in Section 4.6.3, it is sometimes appropriate to place a bike lane on the left side of a one-
8 way street. In this situation, where a left-turn only lane is provided on an approach, the bike lane should
9 be continued along the right side of the left-turn lane, analogous to the treatment for bike lanes with
10 right-turn only lanes described above.

11 As a general rule, bike lanes should be terminated in advance of roundabouts. Design measures for
12 bicyclists at roundabouts are described in Section 4.12.10.

13 **4.9 RETROFITTING BICYCLE FACILITIES ON EXISTING STREETS AND** 14 **HIGHWAYS**

15 Existing streets and highways can be retrofitted to improve bicycle accommodations by either widening
16 the roadway or by reconfiguring the existing roadway. On busier or higher-speed rural roads, paved
17 shoulders can be added to improve comfort for bicyclists. On urban (curbed) roadways, it may be
18 possible to accommodate bicycle lanes by reconfiguring travel lanes or, where that is not practical, to
19 make other adjustments that better accommodate cyclists.

20 Roadway retrofits for bicycle facilities are best accomplished as part of a repaving or reconstruction
21 project. This provides a clean slate for the new marking pattern, eliminating traces of the old lines that
22 remain visible when pavement markings are either painted over or ground off the roadway surface.
23 Where a retrofit requires road widening, completing the retrofit during a repaving project eliminates the
24 potential for rough joints, and reduces costs since the construction crew is already mobilized, and larger
25 material quantities typically result in better prices. Agencies may find it beneficial to systematically
26 review upcoming resurfacing projects to identify opportunities for bike lane and/or shoulder retrofits.

27 When retrofitting roads for bicycle facilities, the width guidelines for bike lanes and paved shoulders
28 (see Sections 4.5 and 4.6.4) should be applied. However, undesignated paved shoulders can improve
29 conditions for bicyclists on constrained roadways where obtaining the preferred shoulder widths is not
30 possible. In these situations, a minimum of 3 feet (0.9 m) of operating space should be provided
31 between the edge line and the edge of pavement (where there is no curb), the gutter joint (where curb
32 and gutter is used), or the curb face (where curb is used without a gutter).

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For example, in a retrofit situation where the total width of the outside lane is 14 feet (4.3 m), it would be preferable to instead provide a 10-11 foot (3.0 -3.4 m) travel lane and a 3-4 foot (0.9-1.2 m) shoulder. Re-striping a 14 feet (4.3 m) travel lane as a 12-foot (3.7 m) lane and a 2-foot (0.6 m) shoulder is not recommended. Since the paved shoulder would not accommodate bicycle operating width, and trying to avoid or repeatedly crossing an edge stripe is uncomfortable, bicyclists would need to ride in the travel lane instead. Even if a bicyclist manages to ride (partly or mostly) on such a narrow paved shoulder, this design may convey a misleading impression of adequate width to a motorist overtaking the bicyclist in the adjacent travel lane, when in fact it would be necessary for the motorist to be driven at least part way into the next lane in order to pass the bicyclist with adequate clearance.

4.9.1. RETROFITTING BICYCLE FACILITIES BY WIDENING THE ROADWAY

Where right-of-way is adequate, or where additional right-of-way can be obtained, roads can be widened to provide paved shoulders or bike lanes. The decision to widen the road should be weighed against the likelihood that vehicle speeds will increase, which will have adverse impacts on bicyclists and pedestrians. In urban and suburban areas with sidewalks or foreseeable pedestrian use, the goal of improving bike accommodation should be balanced with the goal of maintaining a high quality pedestrian environment, as well.

Where the pavement is being widened to provide paved shoulders or bike lanes, and no overlay project is scheduled, the following techniques can be used ensure that a rough joint is not placed in the shoulder where bicyclists ride:

- A saw cut located at the proposed edge line provides the opportunity to construct an even and tight joint. This eliminates a ragged joint at the edge of the existing pavement.
- Feathering the new asphalt onto existing pavement works if a fine mix is used, and the feather does not extend across the area traveled by bicyclists.
- Where there is already some shoulder width and thickness available, a pavement grinder can be used to make a clean cut at the edge of travel lane, with these advantages:
 - Less of the existing pavement is wasted.
 - The existing asphalt acts as a base.
 - There will not be a full-depth joint between the travel lane and the shoulder.
 - The grindings can be recycled as base for the widened portion.

4.9.2. RETROFITTING BICYCLE FACILITIES WITHOUT ROADWAY WIDENING

In many areas, especially built-out urban and suburban areas, physical widening is impractical, and bicycle facility retrofits must be done within the existing paved width. There are three methods of modifying the allocation of roadway space to improve bicyclist accommodation:

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- Reduce or reallocate the width used by travel lanes.
- Reduce the number of travel lanes.
- Reconfigure or reduce on-street parking.

In most cases, travel lane widths can be reduced without any significant changes in levels of service for motorists. An operational study may be necessary to evaluate the impact of a specific lane reconfiguration. One benefit is that bicycle level of service will be improved. Creating shoulders or bike lanes on roadways can improve pedestrian conditions as well by providing a buffer between the sidewalk and the roadway.

Other improvements on the outside portion of the roadway may also be needed during retrofit projects, including:

- Repairing rough or uneven pavement surfaces.
- Replacing unsafe drainage grates with a design that is compatible with bicycle use (see Section 4.12.8).
- Raising (or lowering) existing drainage grates and manhole or utility covers so they are flush with the pavement.
- Widening the roadway at spot locations to obtain adequate road width.

Where addition of bike lanes is planned as a retrofit project, there may be a portion of the roadway where there is insufficient width, resulting in a gap. Shared lane markings can be used on short segments of narrower roadway to provide better continuity. In these situations, efforts to reduce traffic speeds will make the shared roadway condition more comfortable for bicyclists. If the constrained segment is more than a few blocks long, it may be advisable to improve an alternate route for cycling; the alternate route should provide access to the same destinations.

REDUCING TRAVEL LANE WIDTH

In some cases, the width needed for bicycle lanes or paved shoulders can be obtained by narrowing travel lanes. Lane widths on many roads are greater than the minimum values described by the AASHTO Green Book (3) and, depending on conditions, may be candidates for narrowing.

The AASHTO Green Book (3) contains criteria for determining appropriate lane widths and provides significant flexibility to use travel lanes as narrow as 10 feet (3.0 m) in a variety of situations. Evaluation of safety effects of travel lane widths of 10 to 12 feet (3.0 to 3.7 m) on arterial roadways has found no general indication that the use of narrower widths within this range increases crash rates. (4) However, engineering judgment should be applied. Factors to be considered include operating speeds, volumes, traffic mix, horizontal curvature, use of on-street parking, and street context, among others.

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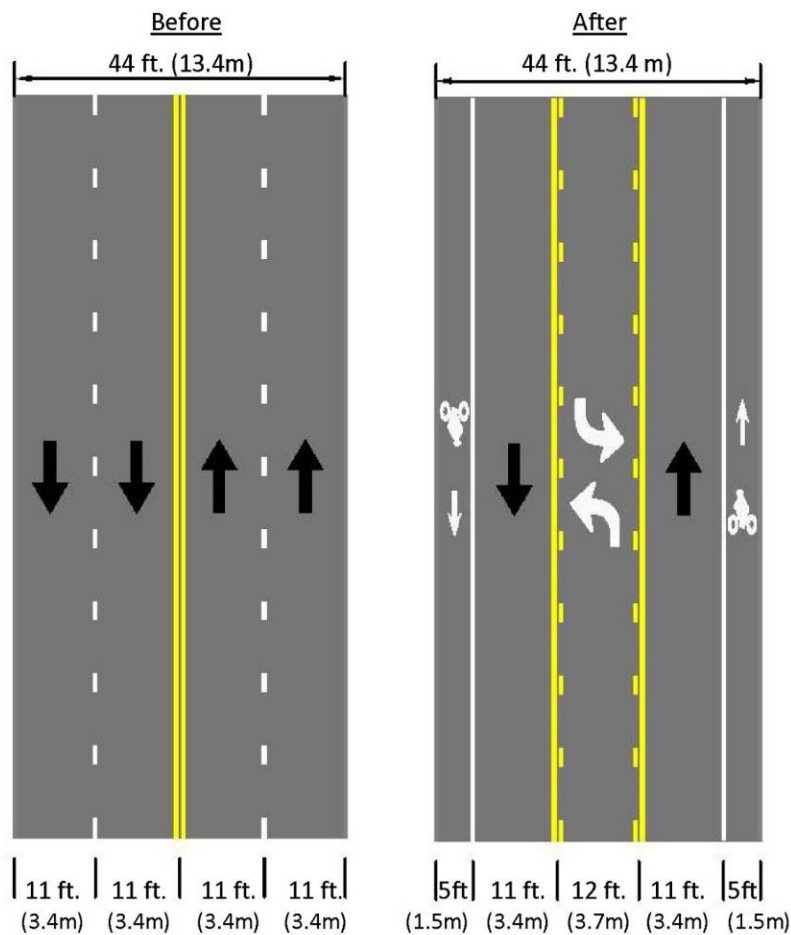
1 REDUCING THE NUMBER OF TRAVEL LANES

2 Reducing the number of motor vehicle travel lanes is often referred to as a “road diet” and is one
3 method that can be used to integrate bike lanes on existing roadways. This is a strategy that can be used
4 on streets with excess capacity (more travel lanes than necessary to accommodate the existing or
5 projected traffic volumes). This may be because the streets were built to accommodate a projected
6 volume that never materialized, or because traffic volumes have decreased due to population changes,
7 or due to changes in the transportation system.

8 Before implementing a road diet, a traffic study should be conducted to evaluate potential safety
9 benefits, to evaluate motor vehicle capacity and level of service, to evaluate bicycle level of service, and
10 to identify appropriate signalization modifications and lane assignment at intersections.

11 Road diets have many benefits, often improving safety, operations and livability for pedestrians,
12 bicyclists, adjacent residents, businesses, and motorists. A common lane reduction treatment is to
13 convert an undivided four-lane (two-way) roadway to a three-lane roadway (central two-way left-turn
14 lane -see Exhibit 4.22). Benefits of this type of road diet include:

- 15 • The additional space gained by removing one lane can be used to provide bike lanes or
16 shoulders on both sides of the road.
- 17 • With one travel lane in each direction, top-end travel speeds are moderated by those who are
18 following posted speed limits, which improves safety for all users.
- 19 • It may be feasible to include a raised median or small refuge islands at some pedestrian crossing
20 locations, making it easier and safer for pedestrians to cross the street.
- 21 • The reduction from two lanes to one in each direction virtually eliminates the risk of "multiple
22 threat" crashes (where a driver in one lane stops to yield, but the driver in the adjacent lane
23 continues at speed) for pedestrians and left-turning motorists and bicyclists.
- 24 • Left-turn lanes provide a place for motorists and bicyclists to wait to make a left turn, reducing
25 the incidence of left turn rear-end crashes.
- 26 • Sideswipe crashes are reduced since motorists no longer need to change lanes to pass a vehicle
27 waiting to turn left from the leftmost through lane.
- 28 • Less traffic noise (due to reduced speeds) and greater separation from traffic for pedestrians,
29 residents, and businesses.

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* Dimensions are illustrative.

Exhibit 4.22 Example of Road Diet

These four-lane to three-lane conversions can have potential operational benefits as well, particularly on streets with high numbers of left-turning vehicles, which impede traffic in the leftmost through lane of a four-lane undivided street. Four-lane undivided streets with traffic volumes less than 15,000 vehicles per day are candidates for four-lane to three-lane conversion; streets with higher volumes usually require a more detailed engineering study that includes recommendations for signal timing changes and other enhancements at intersections. There are many examples of four-lane to three-lane conversions with 15,000 to 20,000 vehicles per day and a few examples where converted streets are carrying over 20,000 vehicles per day. (5)

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Exhibit 4.23 Road Diet – Before and After (Photo by Jennifer Selby)

One-way streets may offer opportunities to install bike lanes through lane reductions. Many one-way couplets were originally two-way streets, and in the conversion, all available space was converted to one-way travel lanes. As a result, many one-way streets operate well below their capacity. Since only one bike lane is needed on a one-way street, removing a travel lane can provide additional space for other features such as on-street parking or wider sidewalks. As mentioned earlier in this chapter, both legs of a one-way couplet should include bike lanes.

REDUCING ON-STREET PARKING

On-street parking has both positive and negative effects on various road users and neighbors. On-street parking may serve residents, help keep traditional street-oriented businesses viable, provide a buffer for pedestrians, and help keep traffic speeds down. But on-street parking can also create conflicts for bicyclists and motorists, and uses road width that might otherwise be used for bicycle lanes. Removing or reducing on-street parking to install bike lanes requires careful negotiation with the affected businesses and residents. It may be possible to accommodate more parking on side streets, or to consolidate it in newly created parking bays or in shared (off-street) parking. A parking study can be conducted to determine if these (and other) solutions are feasible.

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REMOVING PARKING ON ONE SIDE

On most streets with parking on both sides, removal of all on-street parking is not necessary. One strategy is to remove parking from one side of a street, combined with minor additional lane narrowing. Typically, it is best to remove parking on the side of the street with fewer residences or businesses, or the side with residences rather than businesses. It is not necessary to retain parking on the same side of the road through an entire corridor. Alternating parking from one side to the other can create a traffic calming effect as well.

CONVERTING DIAGONAL PARKING TO PARALLEL PARKING

Another strategy to add bicycle lanes is to convert diagonal parking to parallel parking. It is usually sufficient to convert only one side of a street to parallel parking, thereby reducing parking by less than one-fourth. To be compatible with bike lanes, any remaining diagonal parking should be converted to back-in diagonal parking (see Section 4.6.5).

4.10. BICYCLE BOULEVARDS

A bicycle boulevard is a local street or series of contiguous street segments that have been modified to function as a through street for bicyclists while discouraging through automobile travel. Local access is maintained.

Bicycle boulevards create favorable conditions for bicycling by taking advantage of local streets and their inherently bicycle-friendly characteristics: low traffic volumes and operating speeds. However, without some improvements, local streets are usually not continuous enough to be used for long trips. For example, where they intersect a busy thoroughfare, it can be difficult for bicyclists to find adequate gaps to cross. Therefore, a series of physical and operational changes are needed to ensure bicyclists can travel along a bike boulevard with relative ease.

Bicyclists riding on bike boulevards typically share the roadway with other traffic. Some segments may be on busier roads with bike lanes. In locations where street segments do not connect, short sections of paths may be used to connect cul-de-sacs and dead-end streets. Bicycle boulevards should be long enough to provide continuity over a distance typical of an average urban bicycle trip (2-5 miles), but they can also be used for shorter distances when needed to connect path segments in constrained environments, or as a short segment on a route between a neighborhood and a school.

A bicycle boulevard incorporates several design elements to accommodate bicyclists. These may include, but are not limited to:

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- Traffic diverters at key intersections to reduce through motor vehicle traffic while permitting passage for through bicyclists;
- At two-way stop-controlled intersections, priority assignment that favors the bicycle boulevard, so bicyclists can ride with few interruptions;
- Neighborhood traffic circles and mini-roundabouts at minor intersections that slow motor vehicle traffic but allow bicyclists to maintain momentum;
- Other traffic-calming features to lower motor vehicle speeds where deemed appropriate;
- Wayfinding signs to guide bicyclists along the way and to key destinations;
- Shared lane markings where appropriate to alert drivers to the path bicyclists need to take on a shared roadway;
- Crossing improvements where the boulevard crosses major streets. Techniques for this purpose include, but are not limited to:
 - A traffic signal, where warranted, or a crossing beacon. To ensure that cyclists can activate the signal, bicycle-sensitive loop detectors (with detector pavement markings), or push-buttons that do not require dismounting are needed.
 - Median refuges wide enough to provide a refuge (8 feet [2.4 m] min) and with an opening wide enough to allow bicyclists to pass through (6 feet [1.8 m] min).
 - Curb extensions on a crossed thoroughfare with on-street parking, so as to allow approaching bicyclists an opportunity to pull past parked cars to get a better view of approaching traffic.

Not all bicycle boulevards will require all the treatments listed above. A local street may already have many of the desired characteristics and may only need wayfinding signs for continuity; other streets will need varying levels of treatment.

4.11. BICYCLE GUIDE SIGNS/WAYFINDING

Bicycle guide signs can help bicyclists navigate within and between a variety of destinations in urban, suburban, and rural areas. Considerations for planning bicycle wayfinding systems are discussed in detail in Chapter 2. The MUTCD (1) provides standards and guidelines for the design and placement of bicycle guide signs. This section provides supplemental information regarding these sign systems.

As described in Chapter 2, there are several types of bicycle guide signs that can be used.

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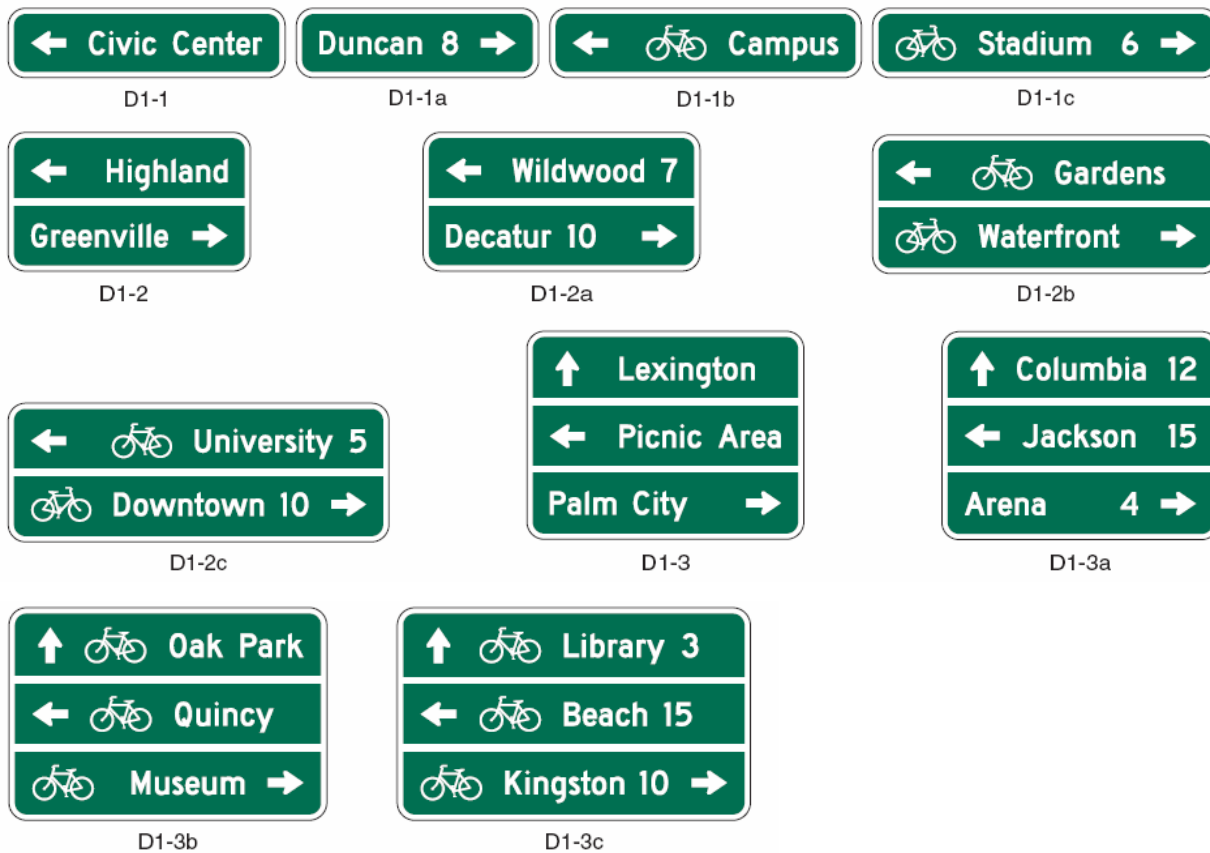
- 1 The D series (green bike route sign and various destination plaques) includes the green BIKE ROUTE sign
2 (D11-1), as well as an alternative sign that replaces the words “BIKE ROUTE” with a destination or route
3 name (D11-1c) (see Exhibit 4.24).

4



6 **Exhibit 4.24. D Series Signs**

- 7 A variety of wayfinding destination sign options can be used either in conjunction with the D11 sign, or
8 independently. D1 signs (see Exhibit 4.25) provide a combination of destination names, arrows, and
9 mileage information that can be very helpful to bicyclists. These signs can be stacked for up to three
10 destinations in different directions and include a directional arrow and a bicycle symbol, plus a
11 destination name (D1-1b, D1-2b, D1-3b), or a destination and a mileage (D1-1c, D1-2c, D1-3c). D1 signs
12 intended for bicyclist guidance should include the bicycle symbol as shown in the MUTCD, unless the
13 sign assembly already incorporates a D11 sign that contains a bicycle symbol.

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1

2 **Exhibit 4.25. D1 Wayfinding Signs**

3 Use of D-1 signs can eliminate the need for multiple D11 signs and supplementary plaques at bikeway
 4 intersections or direction changes and can greatly simplify the signing at these locations. The D11 sign is
 5 still appropriate as a confirming route destination sign beyond the intersection or directional change.

6 The M1-8/M1-8a signs are appropriate for local and regional networks of numbered or lettered routes,
 7 and the M1-9 sign is reserved for U.S. Bicycle Routes that have been designated by AASHTO. Chapter 2
 8 contains additional information on these sign types.

9 Bicycle guide signs must be visible to bicyclists and oriented so bicyclists have sufficient time to
 10 comprehend the sign and change their course if necessary. When appropriate, bicycle guide signs may
 11 be placed on existing posts and light poles to reduce sign and post clutter. However, the MUTCD
 12 prohibits displaying certain types of signs on the same post and should therefore be consulted. (1)

13 Guide signs should be placed at locations where a bike route turns at an intersection, where bike routes
 14 cross one another, and where bike routes cross major roadways (see Exhibit 4.26). Directional arrows
 15 are typically horizontal or vertical, however a sloping arrow may be used if it conveys a clearer indication

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- 1 of the direction bicyclists should travel. At large or complex intersections, it may be appropriate to place
2 signs at both the near and far side or at multiple locations. In rural areas, guide signs should be placed
3 at intersections with major roads and at a maximum spacing of 3 miles (5 km) in sections with no
4 intersections.

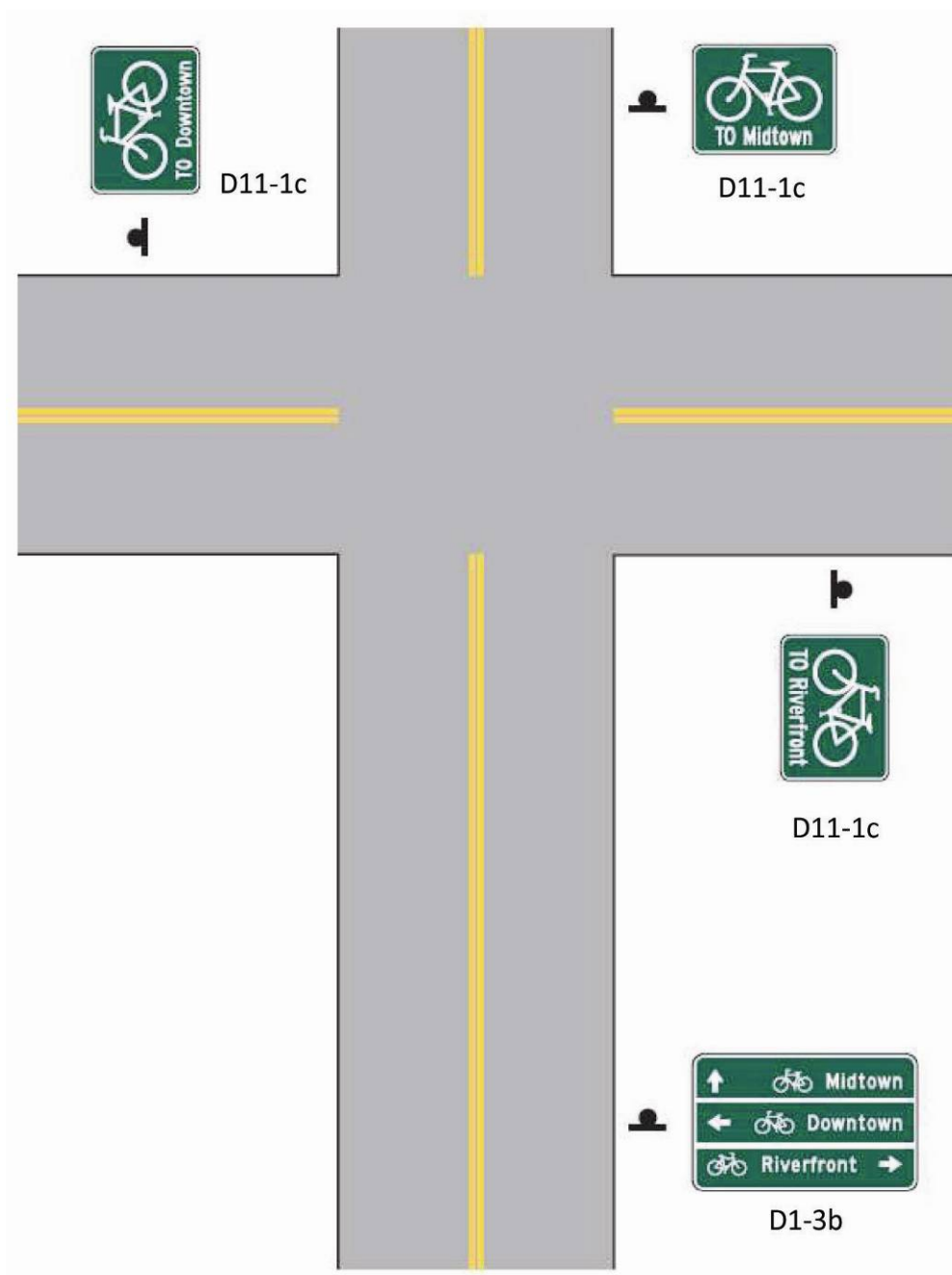


Exhibit 4.26. Typical Bicycle Guide Signage Layout

DRAFT FOR AASHTO COMMITTEE REVIEW AND COMMENT**4.12. OTHER ROADWAY DESIGN CONSIDERATIONS****4.12.1. RAILROAD CROSSINGS**

Railroad tracks that cross roads or shared-use paths on a diagonal can cause steering difficulties for bicyclists. Depending on the angle of the crossing, the width and depth of the flangeway opening, and pavement unevenness, a bicycle wheel may be turned from its course. By improving smoothness and flange opening, the angle may be less critical. The following is a more detailed discussion of these issues.

- **Crossing Angle**

The risk of a fall is kept to a minimum where the roadway or shared use path crosses the tracks at 90°. If the skew angle is less than 45°, special attention should be given to the bikeway alignment to improve the angle of approach, preferably to 60° or greater, so bicyclists can avoid catching their wheels in the flange and losing their balance (see Exhibits 4.27 and 4.28).

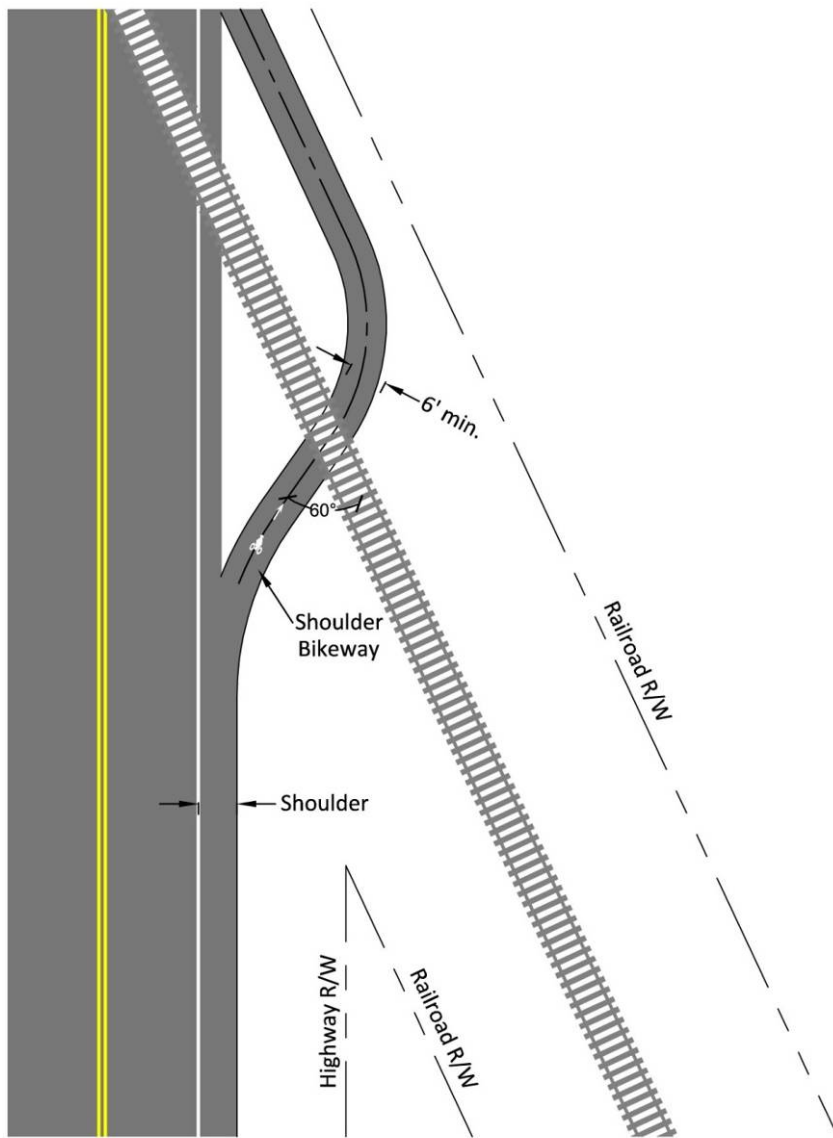
Efforts to create a right-angle crossing at a severe skew can have unintended consequences, as the reversing curves required for a right-angle approach can create other problems for bicyclists. It is often best to widen the roadway, shoulder, or bike lane to allow bicyclists to choose the path that suits their needs the best. On extremely skewed crossings (30° or less), it may be impracticable to widen the shoulders enough to allow for 90° crossing; widening to allow 60° crossing or better is often sufficient. It may also be helpful to post a warning sign at these locations.

- **Crossing Surfaces**

The four most common materials used at railroad crossings are concrete, rubber, asphalt, and timber. Concrete performs best, even under wet conditions, as it provides the smoothest ride. Rubber crossings are quite rideable when new, but they are slippery when wet and degrade over time. Asphalt is smooth when first laid, but can heave over time and must be maintained to prevent a buildup next to the tracks. Timber wears down rapidly and is slippery when wet.

- **Flange Opening**

The open flange area between the rail and the roadway surface can catch a bicycle wheel, causing the rider to fall. Flange width should be minimized when practical. Light rail and trolley lines require only a narrow flange, whereas heavy rail requires a wider flange. There are flangeway filler products that can be used on heavy rail lines with occasional low-speed rail traffic, such as on spur lines. These rubber fillers are depressed by the rail wheels as they ride over the filler; the filler rises again after the train has passed by to keep the flangeway opening limited.

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1

2

Exhibit 4.27. Correction for Skewed Railroad Crossing – Separate Pathway

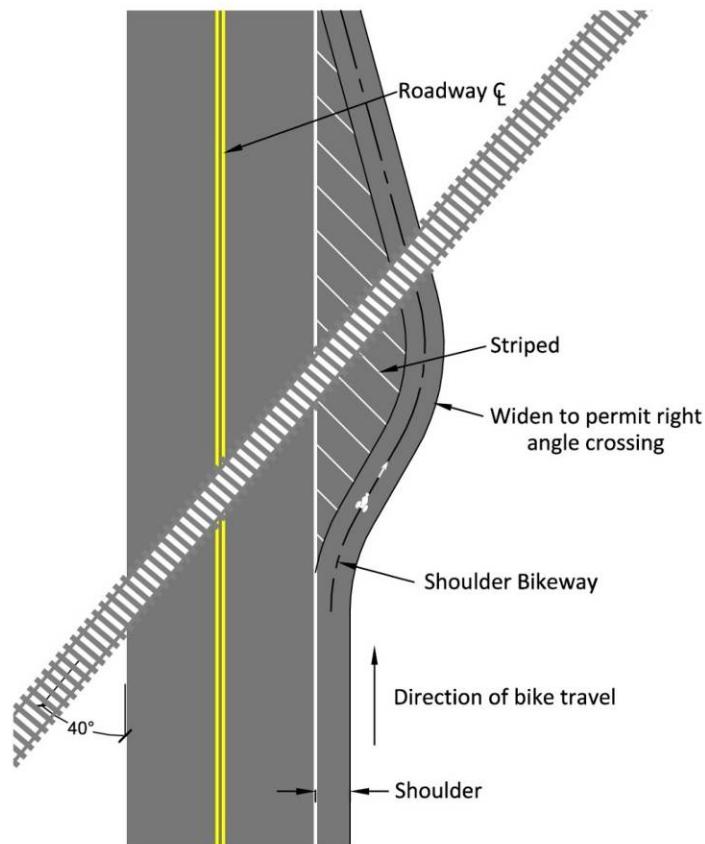
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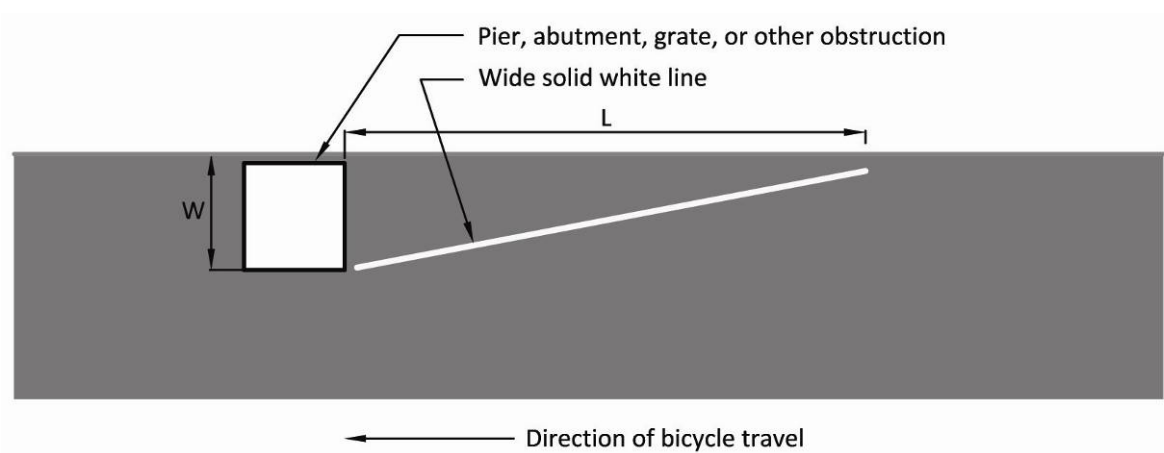
Exhibit 4.28. Correction for Skewed Railroad Crossing – Widened Shoulder

4.12.2. OBSTRUCTION MARKINGS

Barriers and obstructions, such as abutments, piers, rough grates, and other features constricting a bikeway should be clearly marked to gain the attention of approaching bicyclists. This treatment should be used only where the obstruction is unavoidable, and should not substitute for good bikeway design; removing the obstruction is preferred. An example of an obstruction marking is shown in Exhibit 4.29. Equation 4-1 provides the formula for determining the taper length.

Signs, reflectors, diagonal yellow markings, or other treatments may also be appropriate to alert bicyclists to potential obstructions.

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For metric units:
 $L = 0.6 WS$, where S is bicycle approach speed in kilometers per hour

For English units:
 $L = WS$, where S is bicycle approach speed in miles per hour

Exhibit 4.29. Obstruction Marking

US Customary	Metric
$L = WV$	$L = 0.62WV$ (4-6)
where: L = taper length (ft) W = offset width (ft) V = bicycle approach speed (mph)	where: L = taper length (m) W = offset width (m) V = bicycle approach speed (km/h)

Equation 4-1. Formula for Determining Taper Length for Obstruction Markings

4.12.3. TRAFFIC SIGNALS

Traffic signals assign right of way to various traffic movements at intersections. Traditionally, signal design has been determined by the operating characteristics of motor vehicles. Bicyclists typically use the same travelled way and signal displays as motorists. Bicyclists however have significantly different operating characteristics, and it is therefore advisable to adjust signal operations for bicyclists. Although non-motorized users of various types may cross at an intersection, this section addresses only the requirements of bicyclists.

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SIGNAL CONSIDERATIONS FOR BICYCLISTS

The differences in operating characteristics of bicyclists have an impact on some signal design elements. Important factors to consider are the speeds and behaviors of bicyclists. Experienced bicyclists on higher classification roadways (major streets) are typically comfortable entering intersections in the mid to late green due to longer greens available for major thoroughfares. However bicyclists on cross streets tend to slow down approaching the intersection even when approaching on a green, in order to start at the beginning of green. Most bicyclists tend to stop at the onset of yellow in the traffic signal. Children and senior bicyclists often use crosswalks and pedestrian push buttons to cross, therefore these facilities should be accessible to bicyclists who may wish to proceed through the intersection in this manner. These behaviors and preferences have an impact on the selection of signal timing parameters suitable for bicyclists. It is therefore important to evaluate bicycle needs at a traffic signal by considering the scenarios of a stopped bicycle and a rolling bicycle.

The signal parameters that should be modified to accommodate bicyclists, when appropriate, are the minimum green interval, all-red interval, and extension time:

- Minimum green is intended to safely clear a vehicle through the intersection from a stopped position. Bicycles require a longer minimum green than automobiles. Some controllers have a bicycle minimum green parameter which can be used to service bicyclists.
- The all-red interval is used to provide time for crossing vehicles to approach or pass beyond the far side of an intersection.
- Extension time or passage time is the time a detected automobile or bicyclist needs to extend the green indication to provide enough time to clear the intersection before a green indication is displayed to conflicting traffic.

The yellow interval is based on the approach speed of the automobiles and is usually between 3 and 6 seconds in duration. Generally, yellow change intervals calculated for automobiles using commonly accepted formulas are adequate for bicycles.

In some instances it may be appropriate to indicate that a signal head is intended for the exclusive use of bicyclists. A sign can be added near the signal head that states "BICYCLE SIGNAL". This may be appropriate where bicyclists share a signal phase with pedestrians or have their own phase. It may also be appropriate at some path crossings of roadways.

STOPPED BICYCLIST

When an approach receives a green indication, a stopped cyclist needs enough time to react, accelerate and cross the intersection before traffic on the crossing roadway enters the intersection on its green. This is referred to as standing bicycle crossing time, and is used to determine the bicycle minimum green

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- 1 (BMG) time. Intersection crossing time for a cyclist who starts from a stop and attains crossing speed V
 2 within the intersection is given by:

$BCT_{standing} = PRT + \frac{V}{2a} + \frac{(W + L)}{V}$	$BCT_{standing} = PRT + \frac{V}{2a} + \frac{(W + L)}{V}$
where: $BCT_{standing}$ = bicycle crossing time (s) W = intersection width (ft) L = typical bicycle length = 6 ft (see chapter 3 for other design users) V = attained bicycle crossing speed (ft/s) PRT = perception reaction time = 1 s a = bicycle acceleration (1.5 ft/s ²)	where: $BCT_{standing}$ = bicycle crossing time (s) W = intersection width (m) L = typical bicycle length = 1.8 m (see chapter 3 for other design users) V = attained bicycle crossing speed (m/s) PRT = perception reaction time = 1 s a = bicycle acceleration (m/s ²)

3
 4 **Equation 4-2. Standing Bicycle Crossing Time**

5 Most cyclists can accelerate at a rate of at least 1.5 ft/s² (0.5 m/s²) and can obtain a speed of at least 10
 6 mph (14.7 ft/s) [16 km/h (4.5 m/s)].

7 Bicyclists who begin crossing an intersection from a standing start on a new green take more time to
 8 cross than rolling cyclists who enter on green, since they have to accelerate. This time is usually more
 9 critical for cyclists on minor road approaches, since minor-road crossing distance is ordinarily greater
 10 than major-road crossing distance. Bicycle minimum green is determined using the bicycle crossing time
 11 for standing bicycles and clearance time as follows:

12

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US Customary	Metric
$BMG = BCT_{standing} - Y - R_{clear}$ $BMG = PRT + \frac{V}{2a} + \frac{(W + L)}{V} - Y - R_{clear}$	$BMG = BCT_{standing} - Y - R_{clear}$ $BMG = PRT + \frac{V}{2a} + \frac{(W + L)}{V} - Y - R_{clear}$
where: BMG = bicycle minimum green time (s) = bicycle crossing time (s) Y = yellow change interval (s) R _{clear} = all-red (s) W = intersection width (ft) L = typical bicycle length = 6 ft (see chapter 3 for other design users) V = bicycle speed crossing an intersection (ft/s) PRT = perception reaction time = 1 s a = bicycle acceleration (ft/s ²)	where: BMG = bicycle minimum green time (s) = bicycle crossing time (s) Y = yellow change interval (s) R _{clear} = all-red (s) W = intersection width (m) L = typical bicycle length = 1.8 m (see chapter 3 for other design users) V = bicycle speed crossing an intersection (m/s) PRT = perception reaction time = 1 s a = bicycle acceleration (m/s ²)

Equation 4-3. Bicycle Minimum Green Time Using Standing Bicycle Crossing Time

Some controllers have a built-in feature to specify and program a bicycle minimum green. Hence, if appropriate bicycle detection exists, and a bicycle is detected stopped at the intersection, the controller will provide the bicycle minimum green instead of the normal minimum green. If this type of controller is not used, and if the minimum green needed for local cyclists is greater than what would otherwise be used, minimum green time should be increased. However, as with all calculated signal timing, field observations should be undertaken prior to making any adjustments.

ROLLING BICYCLIST

Rolling bicycle crossing time determines the adequacy of any red clearance interval and any extension time, if provided. Although a small percentage of adult cyclists travel at speeds below 10 mph (14.7 ft/s) [16 km/h (4.5 m/s)], most cyclists momentarily can and do achieve higher speeds. Under typical conditions, the speed (V) can be assumed to be at least this great. If the approach is on an appreciable upgrade or downgrade, a modified value may be appropriate.

When estimating whether adequate time is available for a rolling bicycle to safely cross the intersection at the end of a green indication, it is also necessary to consider the braking distance and the width of the intersection. Towards the end of a green indication, beyond a certain point on the approach to the intersection, the bicyclist can neither stop comfortably prior to the intersection nor safely clear the intersection if clearance time is inadequate. A bicyclist requires some distance to brake and stop comfortably. This distance depends on the bicyclist's speed, perception reaction time and deceleration rates. Hence the equation for rolling bicycle crossing time considering braking distance is:

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US Customary	Metric
$BCT_{rolling} = \frac{BD + W + L}{V}$ $BD = PRT * V + \frac{V^2}{2a}$	$BCT_{rolling} = \frac{BD + W + L}{V}$ $BD = PRT * V + \frac{V^2}{2a}$
where: $BCT_{rolling}$ = bicycle crossing time (s) W = intersection width (ft) L = typical bicycle length = 6 ft (see chapter 3 for other design users) V = bicycle speed crossing an intersection (ft/s) BD = breaking distance (ft) PRT = perception reaction time = 1 s a = deceleration rate for wet pavement = 5 ft/s ²	where: $BCT_{rolling}$ = bicycle crossing time (s) W = intersection width (m) L = typical bicycle length = 1.8 m (see chapter 3 for other design users) V = bicycle speed crossing an intersection (m/s) BD = breaking distance (m) PRT = perception reaction time = 1 s a = deceleration rate for wet pavement = 1.5 m/s ²

Equation 4-4. Rolling Bicycle Crossing Time Considering Braking Distance

A signal should provide sufficient time for a rolling cyclist who enters at the end of the green interval to clear the intersection before traffic on a crossing approach receives a green indication. The time available for cyclists to cross the intersection is composed of the yellow change interval, all-red interval and any extension time if provided. As previously stated, the yellow interval is based on the approach speeds of automobiles and therefore should not be adjusted in order to accommodate bicycles. However, it may be feasible to increase the all-red interval. The time should be increased, if necessary, up to the longest interval used in local practice. The following equation is used to determine the all-red interval and extension time for the required rolling bicycle crossing time:

US Customary	Metric
$BCT_{rolling} \leq T_{extension} + Y + R_{clear}$	$BCT_{rolling} \leq T_{extension} + Y + R_{clear}$
where: $BCT_{rolling}$ = bicycle crossing time (s) $T_{extension}$ = extension time (s) Y = yellow change interval (s) R_{clear} = all-red (s)	where: $BCT_{rolling}$ = bicycle crossing time (s) $T_{extension}$ = extension time (s) Y = yellow change interval (s) R_{clear} = all-red (s)

Equation 4-5. All-Red and Extension Time Using Rolling Bicycle Crossing Time

If time for bicycle crossing is inadequate with maximum red clearance time, use of adaptive signal timing for bicycles may be helpful. This technique extends green time when a bicycle approaching late on green

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1 is detected. Traffic engineers typically use extension time and call features within traffic signal
2 controllers; however the extension setting can also be applied within a specific detector. An extension
3 setting for a phase within a traffic signal controller will extend the green time for vehicles that actuate
4 any detector that feeds the respective phase. However, an extension setting applied within a specific
5 detector will extend the green time only for actuations on that detector. Therefore, when using an
6 exclusive bicycle detector, it is recommended to use the extend feature in the bicycle detector settings
7 instead of the extension settings in the traffic signal controller.

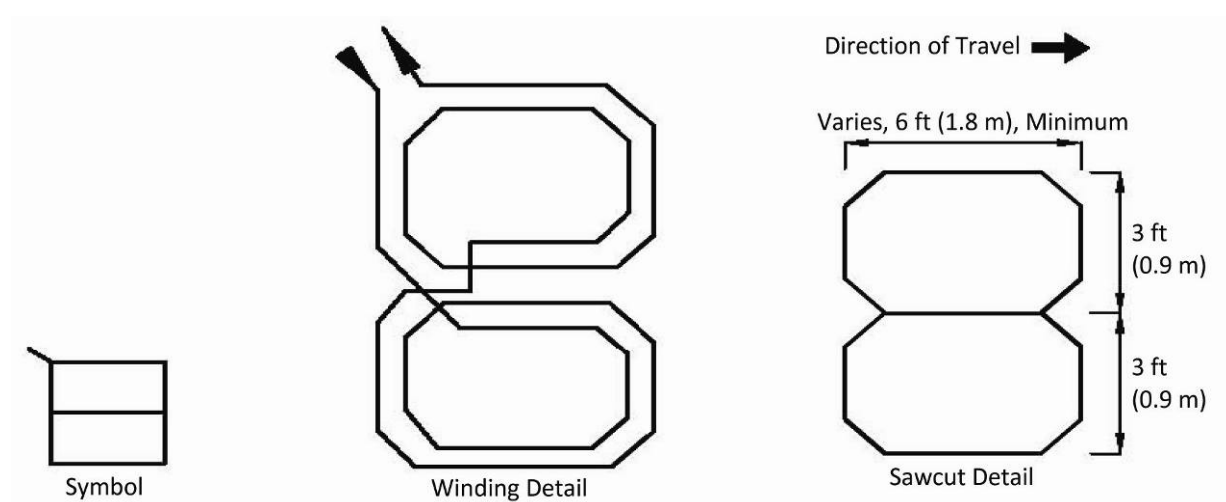
8 Loop detectors cannot distinguish between bicycles and motor vehicles. Therefore, a bike lane is
9 typically needed on the approach in order to provide a location where bicycles (and not automobiles)
10 are detected. In the absence of bike lanes, it may still be feasible to use video detection to distinguish
11 approaching cyclists. The braking distance mentioned earlier can also be used to help determine the
12 location of the bicycle detector. This is to ensure that adequate distance is provided for a bicyclist to
13 stop prior to the intersection if they don't reach the detector just before the end of the green interval.
14 Detection for bicycles at signals is discussed in the following section.

15 **4.12.4. DETECTION FOR BICYCLES AT TRAFFIC SIGNALS**

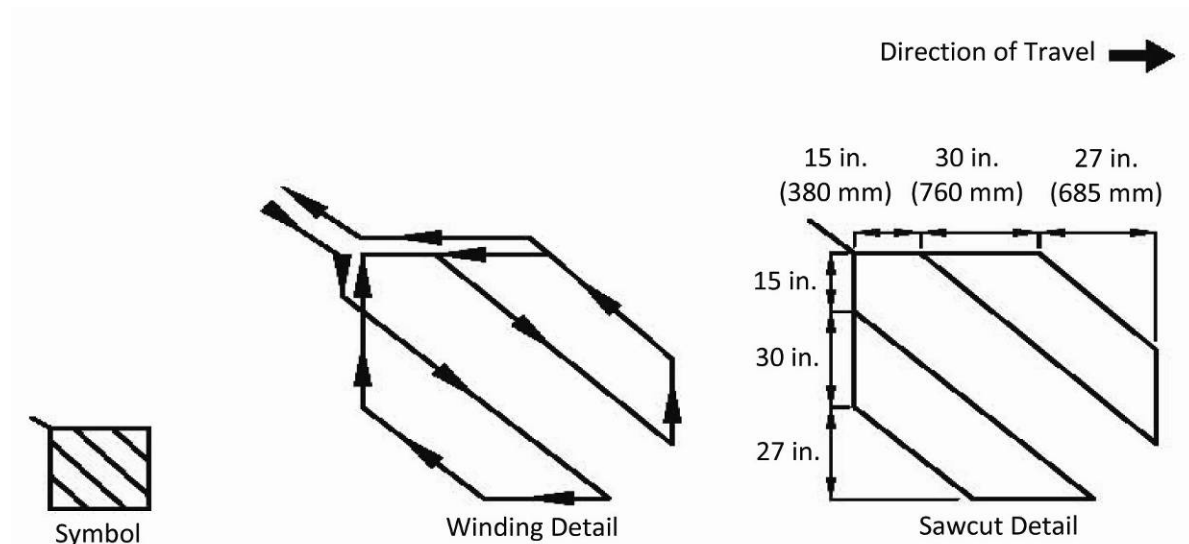
16 Actuated traffic signals should detect bicycles. If a traffic signal does not detect a bicycle, a bicyclist will
17 be unable to call a green light. If a motor vehicle does not arrive to actuate the signal, the cyclist who
18 chooses to proceed through the intersection can do so only by treating the red light as a STOP sign. The
19 most common type of detector is the inductive loop. Loops are wires installed in a specific configuration
20 beneath the pavement surface that can detect the presence of a conductive metal object.

21 **INDUCTIVE LOOP CONFIGURATIONS**

22 Significant research has been conducted to determine the best loop configurations to detect bicycles.
23 Loop layouts have been developed and tested both in bicycle lanes and shared lanes. The quadruple
24 loop detector illustrated in Exhibit 4.30 can detect a metal-frame or metal-rim bicycle at any location
25 above the loop.

DRAFT FOR AASHTO COMMITTEE REVIEW AND COMMENT**Exhibit 4.30. Quadruple Loop Detector**

A quadruple loop detector with a diagonal configuration as illustrated in Exhibit 4.31 can be used when bicyclists share the lane with motor vehicles.

**Exhibit 4.31. Diagonal Quadruple Loop**

The most important aspects of detection are the sensitivity setting of the detector amplifier and the location on the loop where the cycle crosses the loop. The use of sensitivity settings depends on local factors like the depth of the inductive loop, size of the adjacent lanes and the percentage of truck traffic in the adjacent lanes.

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At locations with bike lanes, it is possible to minimize delay to bicyclists and provide green extension time by installing one loop about 100 ft (30 m) from the stop bar, with a second loop located at the stop bar (6). The location of the upstream detector should be far enough from the intersection to allow for the bicycle stopping distance. Another key consideration in the location of the upstream detector is to avoid being triggered by right turn vehicles. The detector located upstream of the stop bar can have a standard loop configuration. When a bicycle is detected at the upstream loop, appropriate extension time is provided to hold the green to allow the bicycle to reach the loop at the stop bar. When the detection is made at the stop bar, extension time is provided to allow the bicycle to move far enough into the intersection to safely clear before the end of the yellow interval. If the detection occurs when the light is red, the minimum timing feature programmed in the signal controller provides the required minimum green time to cross the intersection.

At locations without bike lanes, the bicycle detector pavement marking should be installed over the spot that a bicycle must stand in order to activate the signal (see Exhibit 4.32). This pavement marking can be supplemented by a R10-22 sign (see Exhibit 4.33) to reinforce the message to the bicyclist.

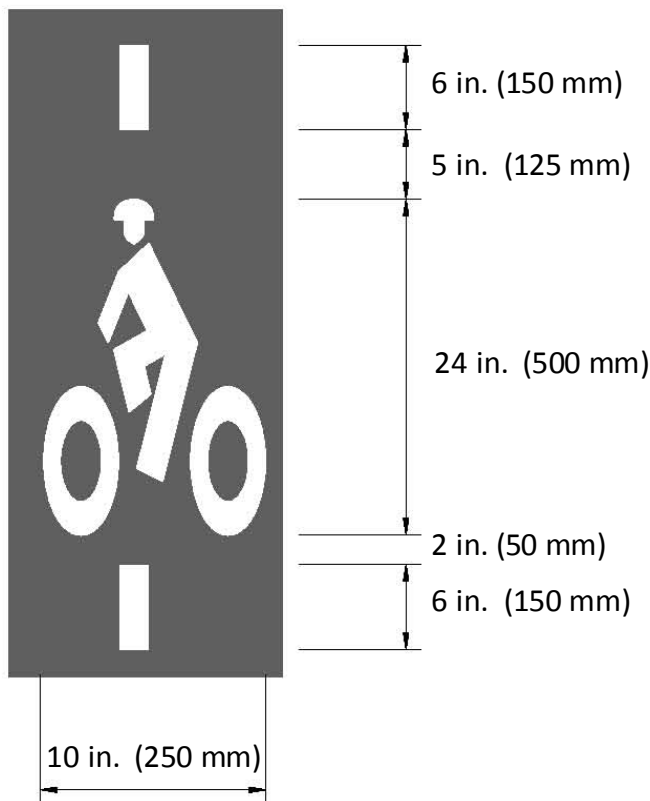


Exhibit 4.32. Typical Bicycle Detector Pavement Marking

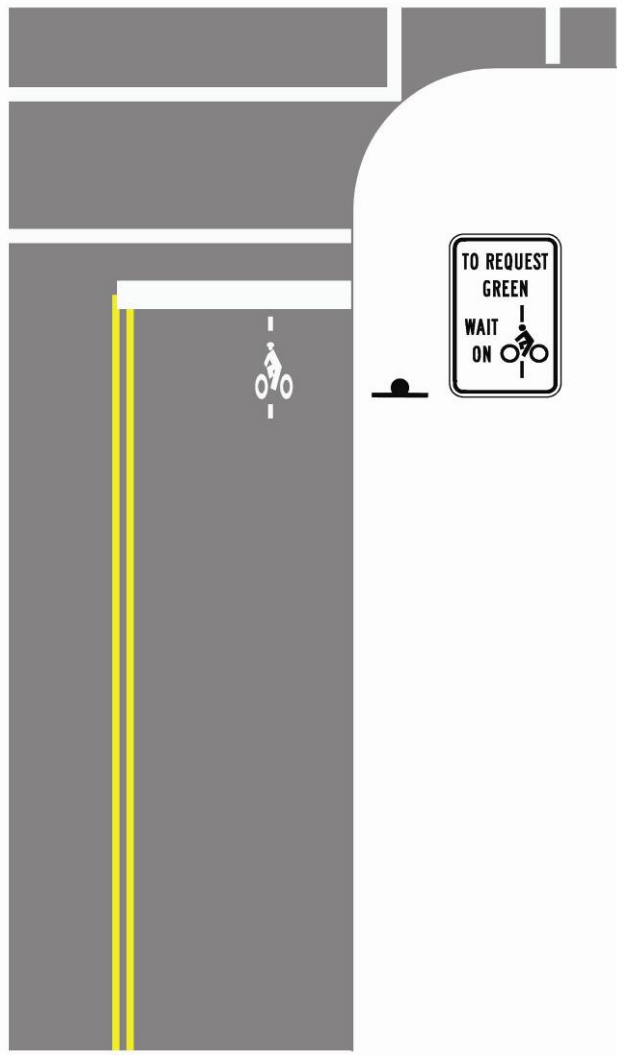
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Exhibit 4.33. Bicycle Detector Pavement Marking and Sign

OTHER SIGNAL DETECTION TECHNOLOGIES

In addition to loops, other detection technologies like video, microwave, and radar are currently being used by traffic agencies. Video detection uses a processor to analyze the video image from a video camera installed either on a signal mast arm or on a pole at the intersection. This processor analyzes the image in zones drawn by the operator. When a vehicle enters the zone, the change in the image is detected and a call is placed to the traffic signal controller. Video detection can be used to detect both moving and stationary objects.

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Agencies have had more success with video than microwave or radar technologies to detect bicycles. Even though some video detectors have some problems detecting vehicles, including bicycles, during poor lighting and weather conditions, many agencies continue to use video detection for ease of installation and maintenance, and flexibility in configuration.

4.12.5. BRIDGES, VIADUCTS AND TUNNELS

Bridges, viaducts and tunnels should accommodate bicycles. As a general exception, these structures are not required to accommodate bicycles on roadways where bicycle access is prohibited. However there are numerous examples of limited access highway bridges that cross major barriers (such as wide waterways) that incorporate a separated pathway for bicycle and pedestrian use.

The type of bicycle accommodation should be determined in consideration of the road function, length of the bridge or tunnel (i.e., potential need for disabled vehicle storage), and the design of the approach roadway. The absence of a bicycle accommodation on the approach roadway should not prevent the accommodation of bicyclists on the bridge or tunnel. Shoulder improvements associated with bridge projects (approach shoulders) should include bicycle accommodations, such as paved shoulders or bike lanes.

The most common types of bicycle facilities that are provided on bridges and in tunnels are bike lanes in urban and suburban areas, and shoulders in rural locations. In most cases (except for those cited below), the bicycle facility will be separated from the pedestrian facility (sidewalk).

In cases where a bridge on a controlled access freeway impacts a non-controlled access roadway (e.g. an overpass/underpass that impacts an existing surface roadway), the project should include the necessary access for bicycles on the non-limited access roadway, including such elements as bike lanes, paved shoulders, wide sidewalks, and bicycle crossings at associated ramps.

In locations where bicyclists will operate in close proximity to bridge railings or barriers, the railing or barrier should be a minimum of 42 inches (1.05 m) high. On bridges where bicycle speeds are likely to be high (such as on a downgrade), and where a bicyclist could impact a barrier at a 25 degree angle or greater (such as on a curve), a higher 48-inch (1.2 m) railing should be considered. If the shoulder is sufficiently wide so that a bicyclist does not operate in close proximity to the rail, lower rail heights are acceptable.

LONG BRIDGES

Long bridges often have higher motor vehicle speeds than their approach roadways. On bridges with a continuous span over 1/2 mile (0.3 km) in length and speeds that exceed 45 mph, consideration should be given to providing a shared use path separated from traffic with a concrete barrier, preferably on

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both sides of the bridge. The provision of a pathway on one side tends to result in wrong-way travel on the departures when cyclists continue on the same side of the road for some distance. If a pathway is only provided on one side, crossing provisions (grade separated, if necessary) are needed on each end of the bridge to allow bicyclists traveling against the flow of traffic to cross over to the other side of the roadway and proceed in a legal manner. See Chapter 5 (Section 5.2.10) for information on the appropriate widths of bridges and underpasses.

RETROFITS TO EXISTING BRIDGES AND TUNNELS

At existing bridges and viaducts, there are often sudden changes in roadway geometry that can significantly reduce travel lane widths and negatively impact bicyclists' safety and comfort for the length of the bridge span.

The preferred solution is to continue to enable bicyclist operation (riding with traffic) on the bridge or viaduct with shoulders or bike lanes by narrowing travel lanes where practical. Where the deck of a bridge is too narrow to accommodate shoulder widths useful for bicyclists, it may be feasible to widen a sidewalk to a shared-path width, e.g., by reducing travel lane widths or installing a cantilever structure. In both cases the weight increase must be compatible with the structural sufficiency of the bridge. A ramp between the roadway and the sidewalk is needed at either end of the bridge.

Retrofit options for tunnels include widening an existing sidewalk, or eliminating a narrow sidewalk. The latter may not be practical where the sidewalk functions as a barrier curb to deter large vehicles from traveling too close to the side, or where it is intended for emergency access or egress. In narrow tunnels where bicyclists must share travel lanes with motor vehicles, one option is to provide a warning beacon at the tunnel entrance that can be activated by bicyclists. The beacon should be designed to flash for the length of time that it will take for a typical bicyclist to travel through the tunnel, to signal to a motorist that a bicyclist is present. Adequate lighting is particularly important in these locations so that motorists can see and react to bicyclists using the tunnel.

4.12.6. BICYCLES AND TRAFFIC CALMING

Traffic calming measures are intended to lessen undesirable traffic impacts by restraining traffic speeds. Bicyclists operate at speeds close to what traffic calming aims for; therefore, effective traffic calming will enhance bicycling on local streets. Bicyclists could be considered the "design vehicle" for traffic calming programs and projects; if they work well for bicyclists, they should achieve other stated goals.

Reducing traffic speeds can be accomplished through physical constraints on the roadway, by adding friction on the side of the road, or by creating a sense of enclosure on the street corridor. Motorists typically drive at a speed they perceive as safe; this is usually related to the road design, especially

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available lane and roadway width. The following sections discuss individual traffic calming techniques in light of their potential advantages or disadvantages for bicycling.

NARROW (SLOW SPEED) STREETS

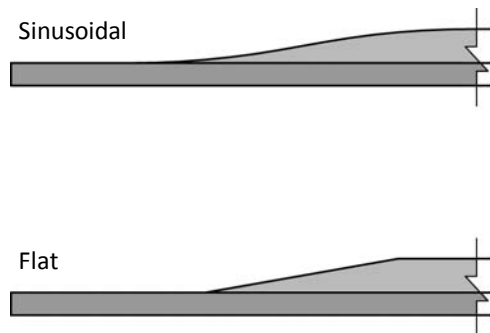
Narrow cross-sections can effectively reduce speeds, as most drivers adjust their speed to the available lane width. Narrow streets also reduce construction and long-term maintenance costs. Effective widths for two-way local streets are 26-28 feet (7.9 - 8.5 m) with parking on both sides, and 20 feet (6.0 m) with parking on one side. These dimensions create “queuing streets,” where oncoming drivers have to wait for the other to pull over into an available space at a driveway or empty parking spot. These dimensions leave enough room for emergency vehicle access, as well as the occasional moving van or large delivery truck.

- Effect on bicycling: **positive**, if operating speeds are reduced to 20-25 mph. Bicyclists simply ride in the lane. This is a strategy that works best on local and residential streets. On busier roads, narrow lanes are less comfortable for bicyclists.

VERTICAL DEFLECTIONS

Vertical deflections include speed humps, speed tables, and speed cushions, as well as raised intersections and raised crosswalks. Well-designed vertical deflections allow vehicles to proceed over the device at the intended speed with minimal discomfort, but will jolt the suspensions and occupants of vehicles driven at higher speeds. Speed humps should be designed with a sinusoidal profile, which is easier for bicyclists to traverse at normal cycling speeds (see Exhibit 4.34). The front edge or lip of the device should be as smooth as practical and meet the road with minimal vertical displacement. Except in speed cushion applications, at-grade gaps should not be provided in vertical deflections for cyclists to pass through, as motorists would take advantage of them, reducing the effectiveness of the feature. To allow drainage in gutters, tapers may be needed to street grade on the edges. Speed cushions, speed tables, raised intersections, and raised crosswalks usually use a flat ramp on each end, and a level area in the middle long enough to accommodate most wheelbases.

- Effect on bicycling: **positive**, as they reduce motor vehicle speeds, assuming that a sinusoidal profile is used.

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*Not to scale

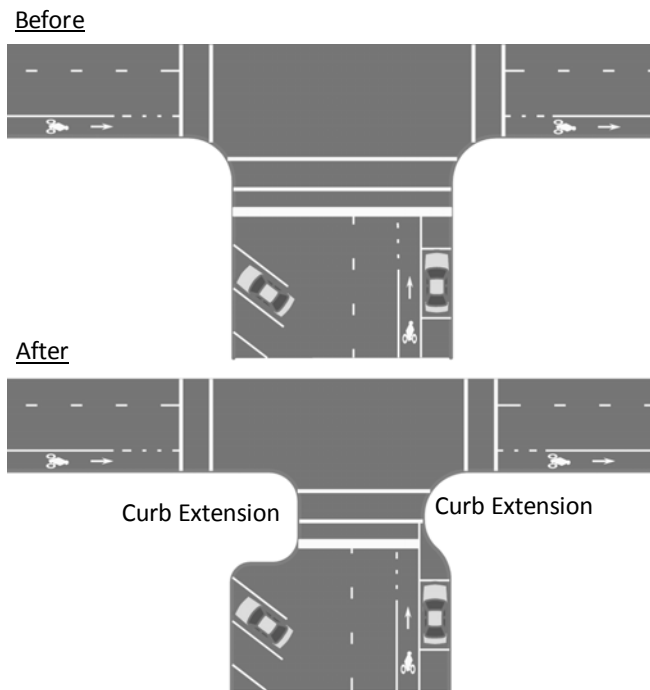
Exhibit 4.34. Examples of Bicycle-friendly Approach Profiles for Speed Humps and Speed Tables

Speed bumps are vertical deflections with heights comparable to speed humps but much shorter traversal lengths (in the range of 1 to 3 ft, typically, in parking area applications). Their use on public roads is unexpected and can result in a serious crash when bicyclists approach them at speed, and fail to notice them in time.

CURB EXTENSIONS (ALSO KNOWN AS CHOKERS, NECKDOWNS OR BULBOUTS)

Chokers constrict the street width to the traveled way minus the width of the nominal on-street parking lane [usually 7 feet (2.1 m)]. They are intended to reduce the pedestrian crossing distance, slow right-turning vehicles, and provide more space for landscaping and other features. Chokers should be highly visible and should not extend beyond the width of the parking lane into the travel path of a bicyclist. The visibility of curb extensions can be increased with bright paint on the curbs, and vertical elements such as landscaping, benches, trashcans, fire hydrants, etc. On busy thoroughfares, where lane lines are striped, a line should be painted between the bike lane and the parking lane to guide bicyclists past the curb extensions (see Exhibit 4.35).

- Effect on bicycling: **positive**, as long as the choker/curb extension is highly visible to bicyclists.

DRAFT FOR AASHTO COMMITTEE REVIEW AND COMMENT***Exhibit 4.35. Curb Extensions*****CHICANES**

By alternating placement of curb extensions (possibly including on-street parking bays or low-growing or narrow landscape features) from one side of the road to the other to establish a serpentine alignment, a chicane reduces the speed of a driver following the curves.

- Effect on bicycling: **generally neutral**. Care should be taken that bicyclists are not surprised by oncoming drivers, or squeezed by overtaking drivers where the width of the traveled way and sight lines have been reduced.

TRAFFIC CIRCLES

Traffic circles are a neighborhood traffic calming device for intersections. They are typically 12 to 16 feet (3.7 to 4.9 m) in diameter, and often include low landscaping and mountable curbs so that large vehicles can bypass the circle. They are used to reduce speeds by deflecting traffic at intersections (similar to a chicane) and reducing long vistas so that drivers tend to slow down.

- Effect on bicycling: **positive**. Traffic circles allow bicyclists to maintain momentum through intersections and are preferable to stop signs, which are often ignored by bicyclists using neighborhood streets.

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CREATING A SENSE OF ENCLOSURE

Establishing buildings at the back of the sidewalk, adding decorative pedestrian-scale lampposts, and planting tall trees at the street edge all help make the roadway appear narrower than it is.

- Effect on bicycling: **positive**, as traffic speeds may be reduced with no constraints on bicyclists.

4.12.7. BICYCLES AND TRAFFIC MANAGEMENT

Traffic management includes the use of traditional traffic control devices to manage volumes and routes of traffic. Traffic management is an area-wide treatment, rather than a solution for a specific street. Traffic management and traffic calming are often complementary, and a plan to retrofit an area often includes a variety of tools from each.

The following measures restrict traffic access to local streets. This may require some out-of-direction travel for certain trips; however, if combined with a plan to develop a bicycle boulevard, these strategies can improve bicycle access overall.

MULTI-WAY STOPS

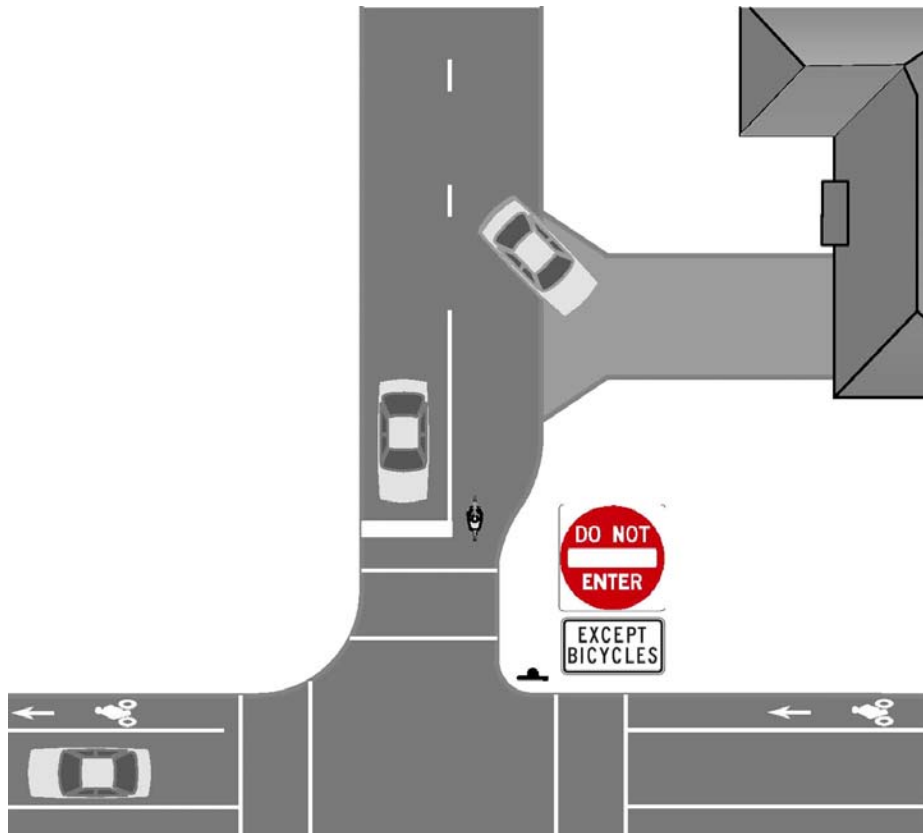
Stop signs are not a recommended traffic management technique. Four-way stops slow cars down excessively, encourage drivers to accelerate to higher speeds to make up for lost time, increase noise and air pollution, and may increase crashes. All-way stop signs are often ignored where there is no perceived danger, breeding disrespect for their legitimate use.

- Effect on bicycling: **negative**, as bicyclists want to maintain their momentum; they are often reluctant to come to a complete stop due to the added energy required to regain momentum.

ONE-WAY CHOKERS

At certain intersections with thoroughfares, motor vehicles are restricted from entering a local two-way street, but are allowed out; drivers must enter from another side street. Bicycles can be exempted from this restriction. This can be made possible with either a plaque (EXCEPT BICYCLES) mounted under a DO NOT ENTER SIGN (see Exhibit 4.36), or by providing a cut-through slot in a physical diverter. Two-way operation resumes immediately past the choker. This is a common strategy used on bicycle boulevards (see Section 4.10), to reduce the amount of motor vehicle traffic along the route.

- Effect on bicycling: **positive**, as long as exemptions are allowed for bicyclists.

DRAFT FOR AASHTO COMMITTEE REVIEW AND COMMENT**Exhibit 4.36. Choker with Bicycle Access****DIVERTERS AND CUL-DE-SACS**

These configurations separate otherwise adjoining street sections, preventing direct travel between them. Caution should be used when physically restricting access, as this may contradict other transportation goals, such as an open grid system. Cul-de-sacs should provide pathways for bicycle and pedestrian access that connect to adjacent streets and/or other cul-de-sacs to form a continuous route.

- Effect on bicycling: **positive** if access to neighboring streets is provided. The effect on bicycling is negative if through-access is not provided for bicyclists, as this limits bicyclists' ability to use low-volume local streets, and forces out-of-direction travel on busier thoroughfares.

Note on one-way chokers and diverters: the benefits to bicyclists are realized only if the cut-throughs are well designed and well maintained. The design should allow bicyclists to proceed with minimal change of direction or slowing; they should be in line with their path of travel (on the right side of the roadway, with no sudden turns required) and wide enough to allow passage for two bicyclists, if two-way traffic is accommodated in the cut-through. A cut-through at a one-way choker only needs to

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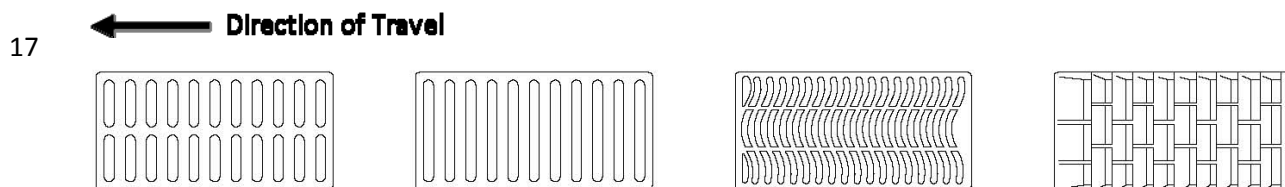
- 1 accommodate one-way bicycle traffic. Maintenance is equally important; cut-throughs tend to
 2 accumulate debris, which should be swept regularly to ensure useful passage by bicyclists.

4.12.8. DRAINAGE GRATES AND UTILITY COVERS

4 Drainage grates with openings running parallel to the curb can cause narrow bicycle wheels to drop into
 5 the gaps and cause a severe crash. Care must be taken to ensure drainage grates are bicycle-safe, with
 6 openings small enough to prevent a bicycle wheel from falling into the slots of the grate. Bicycle-
 7 compatible grates (Exhibit 4.37) should be used.

8 Another way to avoid drainage-grate problems is to eliminate them entirely with the use of inlets in the
 9 curb face. This may require more inlets per mile to handle bypass flow. Another bicycle-friendly option is
 10 to ensure the inlet grate is entirely contained in the gutter of the street, rather than extending it out into
 11 the traveled way.

12 Where bicycle-incompatible grates remain, metal straps can be welded across slots perpendicular to the
 13 direction of travel at a maximum longitudinal spacing of 4 inches (100 mm), although care must be taken
 14 to ensure that the grate does not become a debris collection site. These should be checked periodically
 15 to ensure that the straps remain in place. In general, this is only a temporary solution and the location
 16 should ultimately be retrofitted with bicycle-compatible drainage grates.



19 **Exhibit 4.37. Bicycle-Compatible Drainage Grates**

20 Another problem arises when the roadway surface sinks, crumbles, or becomes otherwise unrideable
 21 around the catch basin area. Surface grates should be flush with the road surface. Inlets should be raised
 22 after a pavement overlay to within 1/4 inch (6 mm) of the new surface. If this is not possible or practical,
 23 the pavement must taper into drainage inlets so it does not have an abrupt edge at the inlet.

24 Utility covers present similar problems and should be installed flush with the adjacent roadway surface.

DRAFT FOR AASHTO COMMITTEE REVIEW AND COMMENT**4.12.9. BICYCLES ON FREEWAYS AND AT INTERCHANGES**

Bicycling on freeways is prohibited in many states. In some states, however, bicycle operation is permitted on freeway shoulders where authorized by maintaining agencies. This is typically done where alternative routes are unavailable or unsuitable, and a freeway segment is deemed compatible with bicycle travel. Where freeways are open to bicycle travel, bicyclist usage is usually infrequent. Crash studies have revealed relatively few crashes involving bicyclists on freeways. (7) Where feasible, alternatives can be developed by improving existing routes or providing a shared use path within or adjacent to the freeway right-of-way.

The following factors should be considered in determining the relative suitability of a freeway segment and an alternative route:

- The wind blast effect of high speed vehicle traffic, particularly large trucks, should be considered. Clear shoulder width (exclusive of rumble strips) should be sufficient to provide adequate separation between bicyclists and high speed traffic. Bicycle level of service can be helpful in determining the appropriate shoulder width.
- The frequency and design of entrance/exit ramps should be considered. For example, two-lane ramps are difficult for cyclists to maneuver across. Flyover and left-side ramps can create very difficult conditions for bicyclists, depending upon their configuration. Bicyclists should not have to merge across the through-lanes of a highway to reach an exit.
- Heavy volumes of traffic on entrance/exit ramps can make it difficult for bicyclists to cross ramps at certain times of day.

At an exit beyond which cyclists are not permitted to continue on a limited-access highway, a sign should be posted to inform cyclists of the exit requirement.

Like motorists, bicyclists often have to pass through freeway interchanges to access roads and destinations on the other side of a freeway. In urban and suburban areas, bicyclists of all skill levels travel on arterial and collector streets at freeway interchanges. These interchanges can be significant obstacles to bicycling if they are poorly designed.

In rural areas, traffic volumes are usually low, and recreational and touring bicyclists are usually experienced enough to make their way through an interchange. Shoulder widths through interchanges should be wide enough for bicycle use.

BASIC DESIGN PRINCIPLES AT FREEWAY INTERCHANGES

It is important to consider both convenience and safety when accommodating bicycle travel near interchanges. The issue of safety becomes moot if facilities are not used because of perceived inconvenience. The path bicyclists need to follow should be obvious and logical, minimizing out-of-

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1 direction travel and grade changes. The interface between the ramps and the local cross streets should
2 minimize conflicts and ensure that both motorists and bicyclists are aware of merging and crossing
3 locations. Bike lanes or paved shoulders should be provided in both directions.

4 The critical areas for bicyclist safety and convenience are at the freeway ramp terminals, where freeway
5 traffic interacts with local traffic and the speed differential between bicyclists and motor vehicles is
6 often great. Designs that encourage high speed and/or free-flowing traffic movements are the most
7 difficult for bicyclists to negotiate safely and comfortably, and are generally not appropriate in urban
8 and suburban areas. Designs that are functional for bicycle passage typically require slowing or stopping
9 motor vehicle traffic.

10 Bicyclists are best accommodated at interchanges by designing junctions as right-angle intersections
11 (Exhibit 4.38) or single lane roundabouts. Such designs restrain speeds, minimize conflict areas, and
12 promote visibility. In this way, conflicts between bicyclists and motorists are dealt with in a manner
13 familiar from most urban intersections:

- 14 • Motorists exiting the freeway and making a left turn onto the arterial street are controlled by a
15 stop sign or signal.
- 16 • Motorists exiting the freeway and making a right turn onto the land access road are controlled
17 by a stop sign, signal, or yield sign, rather than allowing a free-flowing movement.

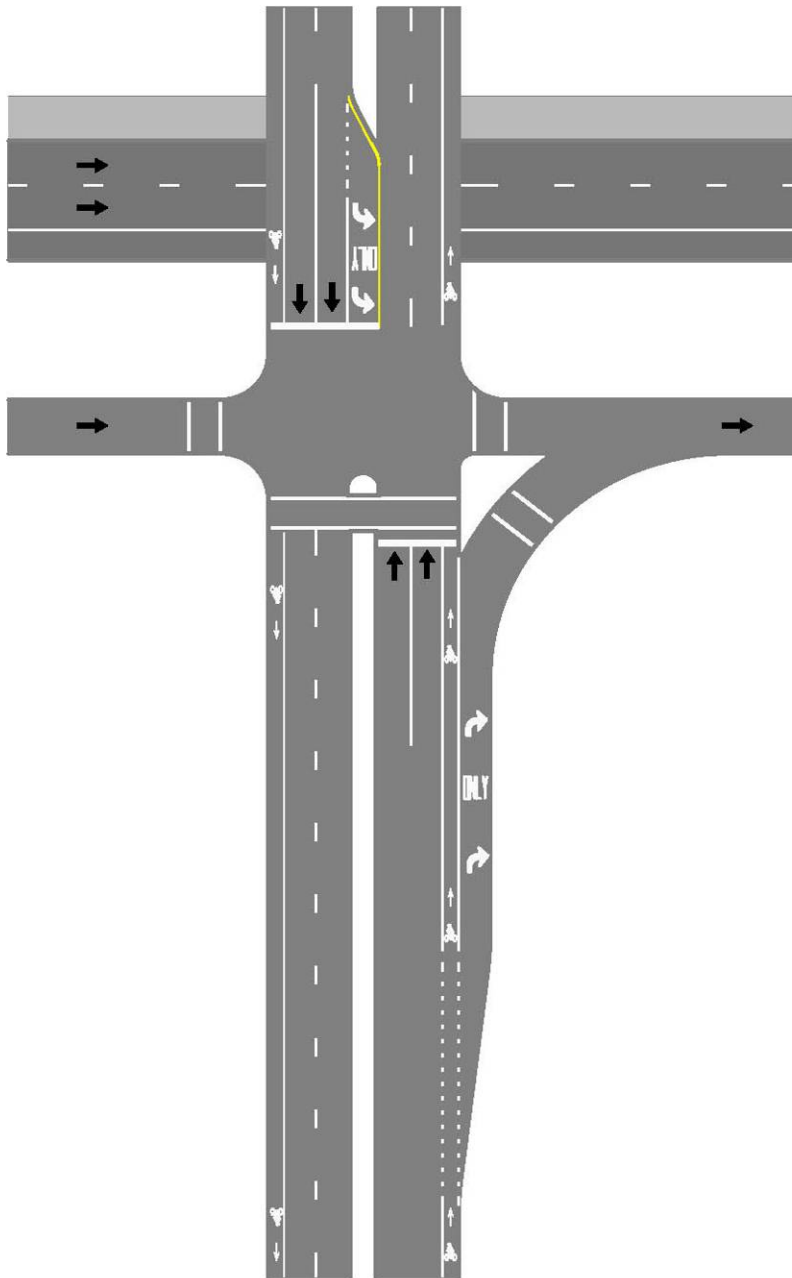
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Exhibit 4.38. Example of Bike Lane and Freeway Interchange

- Motorists turning left from the land access road onto a freeway entrance ramp are controlled by a traffic signal or yield to oncoming traffic, including bicyclists.

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- A right-turn lane should be added with a taper for motorists turning right onto the freeway entrance ramp. Where a bicycle lane is present on the approach, a bicycle lane continuation slot should be provided along the left side of the right-turn lane. Since motorists must cross the path of cyclists to enter the right-turn lane, they are required to yield. The slot treatment can also be helpful where an approach has a paved shoulder, providing for the correct positioning of the bicyclist at interchanges.

SINGLE-POINT URBAN INTERCHANGE (SPUI)

The Single-Point Urban Interchange (Exhibit 4.39) is gaining favor for urban locations because of the reduced need for right-of-way, its ability to handle high volumes of left-turning traffic, and the potential for improved cross street throughput. SPUIs can be made accessible to bicyclists by following these principles:

- Each vehicular movement should be clearly defined and controlled.
- Exit and entry ramps should be designed at close-to-right angles.
- The right-turning movement off the local arterial onto the freeway should be accommodated by using a standard right-turn lane with a bike lane to the left, encouraging motorists to yield to cyclists when merging into the right-turn lane.
- Bicyclists should be able to proceed through the intersection in a straight line. Dotted lane lines may be needed to guide bicyclists through wide intersections (see Exhibit 4.34).
- Careful consideration should be given to the traffic signal timing. The fact that all ramp terminals come to a single, signalized intersection creates a very large intersection, which can make it difficult to provide adequate signal clearance time for bicyclists. To solve this problem, the signal phasing order should be as follows:
 1. Through vehicles on the arterial.
 2. Left-turn movements from the arterial to the freeway.
 3. Left-turn movements from the freeway to the arterial.

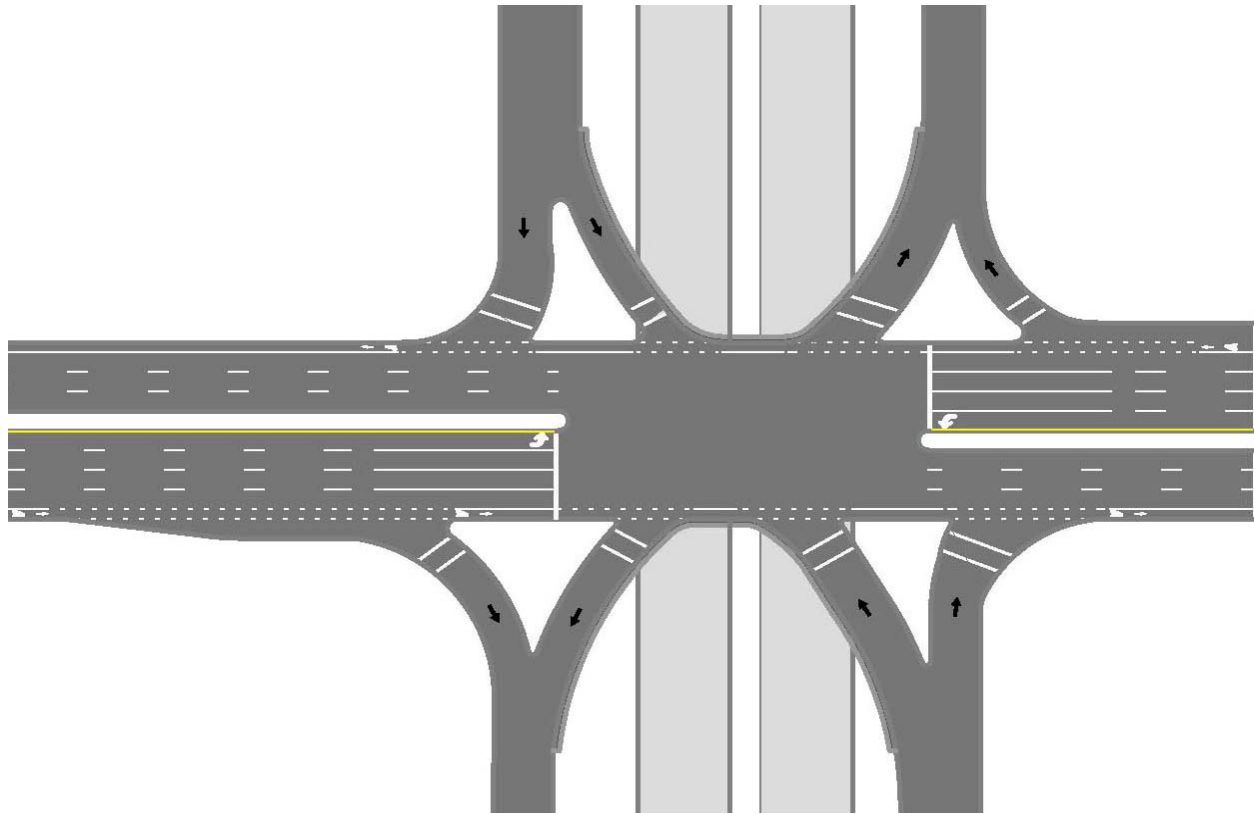
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Exhibit 4.39. Single-Point Urban Interchange (SPUI)

If the second phase is skipped (e.g., because no vehicle enters one of the left-turn lanes on the land access roadway), a through bicyclist might still be passing through the intersection when a green indication is displayed for the left-turn movements from the freeway exit ramps. To allow bicyclists time to clear the conflict area when this happens, use of a longer all-red interval may be necessary (see the section on traffic signals earlier in this chapter).

The SPUI can be designed to work reasonably well for bicyclists if it is the intersection of a local thoroughfare and a freeway; bicyclists need to be accommodated only on the cross street, not the freeway. If a SPUI is used for the grade-separated intersection of two surface streets, both of which accommodate cyclists, then the SPUI design is not effective, as bicyclists on one of the streets will be in a freeway-like environment, with free-flowing exiting and merging ramps.

HIGH-SPEED MERGE AND FREE-FLOW TURN LANES

As described above, configurations on local arterials with high-speed merges and/or free-flow turn lanes are difficult for bicyclists to negotiate safely and should be discouraged. However, there are many existing interchanges where high-speed merges and free-flow exit lanes are already in use, and there are some situations where these high-speed movements are used to avoid unacceptable levels of delay

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within the interchange. In addition, bike lanes are sometimes used on urban parkways, which often have freeway-style merging lanes and turn ramps rather than simple intersections. The difficulties for cyclists created by traffic entering or exiting a roadway at high speeds can be minimized using the designs below.

At some interchanges, it may be appropriate to allow bicyclists the option of using sidewalks, particularly if this will provide access to a signalized crosswalk or other crossing situation that may be more comfortable for some bicyclists. A disadvantage of this approach is that bicyclists riding on sidewalks conflict with pedestrians and may experience other operational difficulties (see discussion in Section 5.2.2). If this option is provided, there should be sidewalks on both sides, and they should be wide enough for shared use by bicyclists and pedestrians.

BICYCLE LANE TREATMENT AT MERGING RAMP LANES

It is difficult for bicyclists to traverse the undefined area created by right-lane merge movements, because the acute angle of approach reduces visibility, and the speed differential between cyclists and motorists is high because motor vehicles are accelerating to merge into traffic. There are two approaches to the treatment of bicycle lanes at such locations:

1. The first option is to simply allow bicyclists to choose their own merge, weave, or crossing maneuvers, as depicted in Exhibit 4.40. Where the merge area is fairly short (i.e. bicyclists are exposed for less distance), it may be appropriate to continue bike lane or shoulder markings as dotted lines through the merge area, if the ramp configuration is such that merging traffic is at fairly low speeds.
2. Where the merge distance is long and there are exceptionally high volumes of ramp traffic, it may be appropriate to provide a design that guides bicyclists in a manner that provides a short distance across the ramp at close to a right angle, and a crossing in an area where sight lines are good and drivers' attention is not entirely focused on merging with traffic (Exhibit 4.41). However, this configuration reverses the yielding relationships that would otherwise apply (if a bicyclist continued on a direct path), and can involve delay to bicyclists. Crosswalks should not be used at these locations, because vehicles merging should not be expected to stop here.

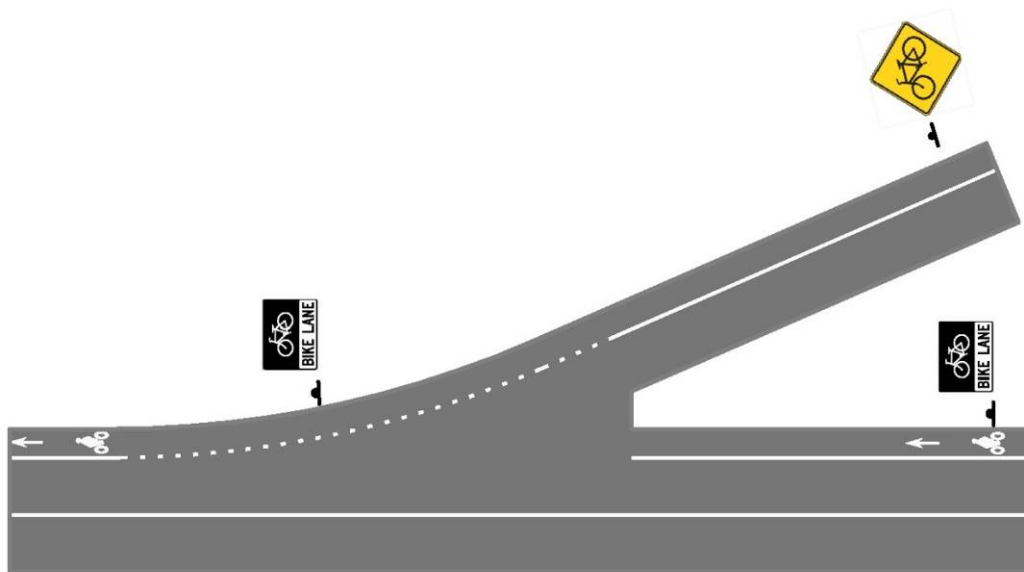


Exhibit 4.40. Option 1 – Bike Lane and Freeway On-ramp

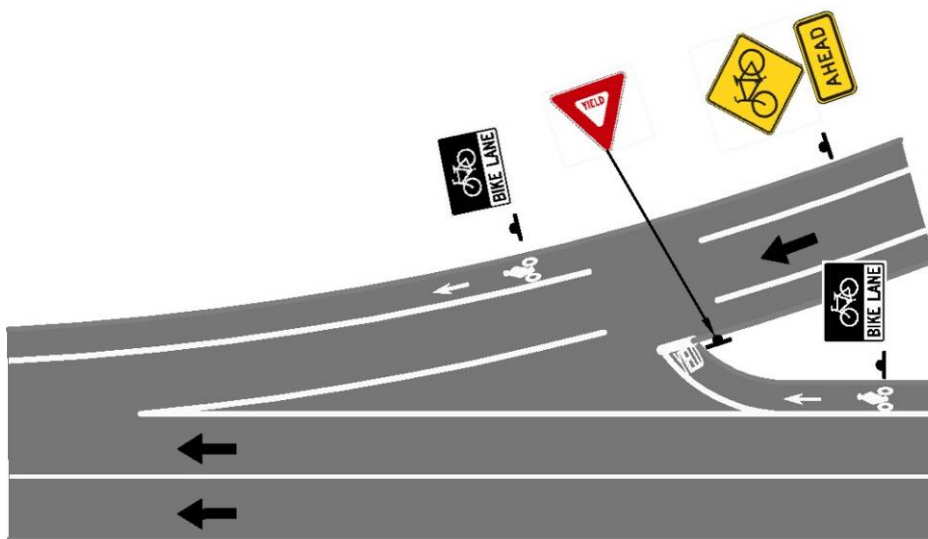


Exhibit 4.41. Option 2 – Bike Lane and On-ramp

DRAFT FOR AASHTO COMMITTEE REVIEW AND COMMENT**BICYCLE LANE TREATMENT AT DIVERGING RAMP LANES**

Diverging ramp lanes present difficulties for bicyclists because motorists expect to exit the roadway with little reduction in speed, and bicyclists may misjudge the intent of overtaking drivers who fail to use their turn signals. The best way to accommodate bicyclists at an exit ramp is to develop a right-turn lane prior to the point where the ramp diverges from the roadway, and place the bike lane to the left of the right-turn lane, similar to a right-turn lane configuration at a right-angle intersection (see Exhibit 4.42). Alternatively, where a ramp diverges from the roadway at a fairly steep angle, a bicycle lane can be dotted across the diverge area and the R4-4 BEGIN RIGHT TURN LANE YIELD TO BIKES sign placed at the beginning of the diverge area. In cases where motor vehicle speeds are high and sidewalks are present, bicyclists should be given the option to exit onto the sidewalk and to proceed through the interchange along the pedestrian route. However the on-road bike lane should still be provided for bicyclists who prefer to remain on the road.

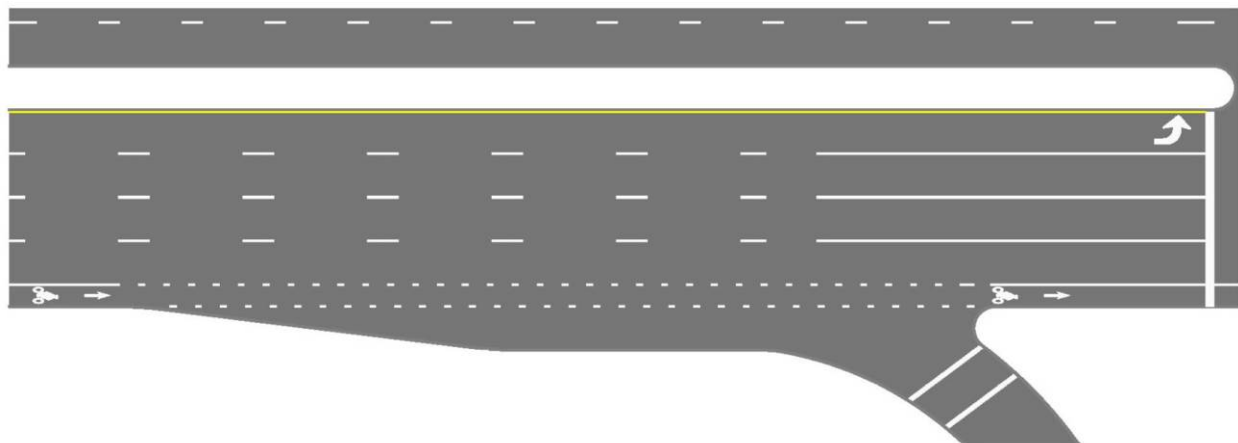


Exhibit 4.42. Example of Bike Lane and Exit Ramp

GRADE-SEPARATED CROSSINGS AT RAMPS

At especially complex interchanges where conflicts between bicycles and high-speed and free-flow motor vehicle movements are unavoidable, grade separation may be considered. Grade-separated facilities add out-of-direction travel, and will not be used if the added distance is too great. This can create a potentially hazardous situation if bicyclists ignore the facility and try to negotiate the interchange at grade with no accommodations to facilitate this movement.

Ideally, grade separation is achieved by providing separated paths on both sides of the arterial street that cross over or under the freeway ramps and the freeway itself, so approaching bicyclists from either direction do not have to cross the arterial to continue through the interchange. If a separated path for grade separation is provided on only one side of the interchange, some bicyclists will need to cross the

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arterial street in order to use the grade separation, and then they must cross back to continue on the correct side after going through the interchange.

Regardless of whether two paths or one path is used, clear directions must be given to guide bicyclists' movements at interchanges, particularly those that differ from standard bicycle operation. To ensure proper use by bicyclists, structures must be convenient and have good visibility – especially undercrossings. Personal security is an important consideration as well, as the grade separation may result in long sections of pathway that cannot be easily accessed in an emergency. Adequate lighting is particularly important at these locations.

Shared use paths at interchanges should be designed to avoid significant grade changes. Opportunities to provide direct links to destinations should be sought if they reduce travel distance compared to the roadway alignment.

Grade-separated crossings will also be used by pedestrians, therefore they must meet accessibility standards; see Chapter 5: Shared Use Paths for more information.

4.12.10. BICYCLES AT ROUNDABOUTS

Roundabouts are an increasingly popular design solution for intersections. Single-lane roundabouts can provide significant safety benefits for bicyclists when they are designed with their needs in mind. At roundabouts, some bicyclists will choose to travel on the roadway, while others will choose to travel on the sidewalk. Roundabouts can be designed to simplify this choice for cyclists.

GENERAL ROUNDABOUT DESIGN ISSUES

Since typical on-road bicycle travel speeds are between 10 and 20 mph (15 and 30 km/hr), roundabouts that are designed to constrain motor vehicle speeds to similar values will improve safety and usability for bicyclists. Urban roundabouts should have a maximum entry speed of 20 mph to 30 mph (30 km/hr to 50 km/hr); single-lane roundabouts are typically at the lower end of this range. As such, it is critical to ensure that the geometric features of a roundabout (e.g. entry and exit radius, entry and exit width, splitter islands, circulatory roadway width, and inscribed circle diameter) combine to constrain motor vehicle speeds. (8)

Single-lane roundabouts are much simpler for bicyclists than multilane roundabouts, since they do not require bicyclists to change lanes, and motorists are less likely to cut off bicyclists when they exit the roundabout. Therefore, when designing and implementing roundabouts, authorities should avoid implementing multilane roundabouts before their capacity is needed. If “design year” traffic volumes indicate the need for a multilane roundabout, but this need isn’t likely for several years, the roundabout can be built as a single-lane roundabout, and designed to be easily reconstructed with additional lanes

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1 in the future when and if traffic volumes increase. In addition, where a roundabout is proposed at an
2 intersection of a major multilane street and a minor street, consideration should be given to building a
3 roundabout with two-lane approaches on the major street and one-lane approaches on minor streets.
4 When compared to roundabouts with two lanes at all four legs, this design can significantly reduce
5 complexity for all users, including bicyclists.

DESIGNING FOR BICYCLE TRAVEL WITHIN THE ROUNDABOUT

7 In general, bicyclists who have the skills to ride in urban traffic can manage single-lane roundabouts with
8 little difficulty. Where appropriate design speeds are used, bicyclists can comfortably merge into the
9 lane of traffic. Even at multilane roundabouts, many bicyclists will be able to travel through roundabouts
10 in the same manner as other vehicles.

11 Bike lanes should be terminated in advance of roundabouts. The full-width bike lane should normally
12 end at least 100 feet (30 m) before the edge of the circulatory roadway (see Exhibit 4.43). Terminating
13 the bike lane cues bicyclists to merge into the lane of traffic. An appropriate taper should be provided
14 to narrow the sum of the travel lane and bike lane widths down to an appropriate entry width for the
15 roundabout. The taper should end prior to the crosswalk at the roundabout, to achieve the shortest
16 feasible pedestrian crossing distance. A taper rate of 7:1 is recommended to accommodate a design
17 speed of 20 mph (25 km/hr). To taper a 5 to 6 foot (1.5 to 1.8 m) wide bicycle lane, a 40 foot (12 m)
18 taper is recommended. The bicycle lane line should be dotted for 50 to 200 feet (15 to 60 m) in advance
19 of the taper. A longer dotted line encourages cyclists to avail themselves of timely gaps to merge into
20 traffic, rather than delay until a point where, if no gap is available at the moment, the only safe
21 alternative is to pause and wait for one. The bike lane line should be terminated at the start of the taper
22 or where normal bike lane width is no longer available.

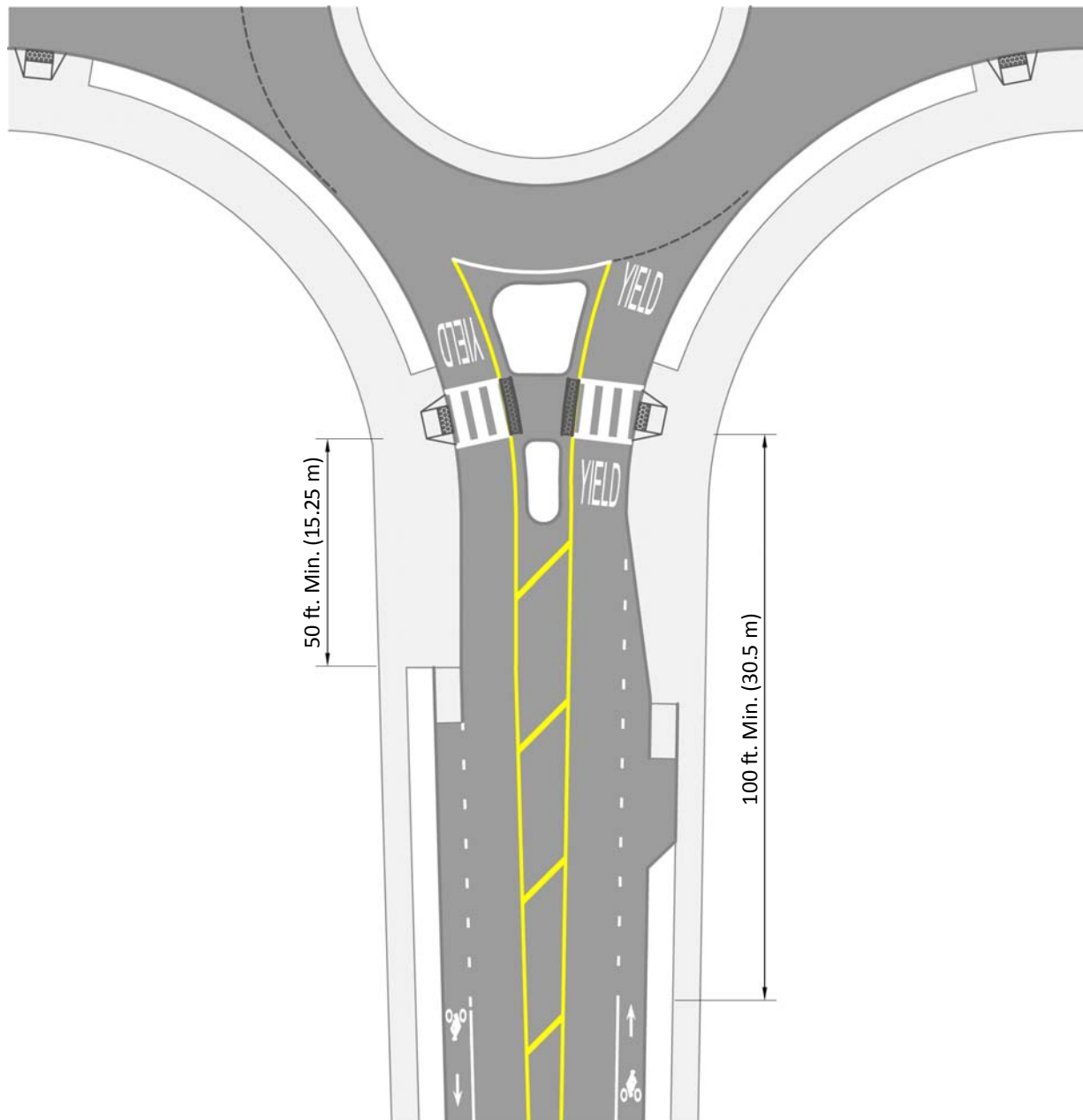
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Exhibit 4.43. Typical Layout of Roundabout with Bike Lanes (note: to be replaced with final graphic in next edition of FHWA Roundabout Guide)

Bike lanes should not be located within the circulatory roadway of roundabouts. This design would suggest that bicyclists should ride at the outer edge of the circulatory roadway, which creates turning conflicts at exits and entrances.

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At roundabout exits, an appropriate taper should begin after the crosswalk, with a dotted line for the bike lane through the taper. The solid bike lane line should resume as soon as the normal bicycle lane width is available.

DESIGNING FOR BICYCLISTS TO TRAVERSE ROUNDABOUTS ON THE SIDEWALK

Some on-road bicyclists may not feel comfortable navigating roundabouts on the roadway. Bicycle ramps can be provided to allow access to the sidewalk or a shared-use path at the roundabout. Bicycle ramps at roundabouts have the potential to be confused as pedestrian ramps, particularly for pedestrians who have visual impairments. Therefore, bicycle ramps should only be used where the roundabout complexity or design speed may result in less comfort for some bicyclists. As described above, multilane roundabouts are more challenging for bicyclists, therefore bicycle ramps can be useful in these locations. Bicycle ramps may also be appropriate at single-lane roundabouts, if traffic speeds or other conditions (e.g. a right-turn bypass lane) make circulating like other vehicles more challenging for bicyclists. Otherwise, ramps are not normally needed at urban, single-lane roundabouts.

Where bicycle ramps are provided at a roundabout, consideration should be given to providing a widened sidewalk at the roundabout. In areas with relatively low pedestrian usage and where bicycle usage of the sidewalks is expected to be low, the normal sidewalk width may be sufficient. In some jurisdictions, state or local laws may prohibit cyclists from riding on sidewalks. In these areas, bicycle ramps may not be appropriate.

The design details of bicycle ramps are critical to ensure usability and provide choice to bicyclists, and to reduce the potential for confusion of pedestrians, particularly those who are blind or who have low vision. Bicycle ramps should be placed at the end of the full width bicycle lane, just before the beginning of the taper for the bike lane. Bicyclists approaching the taper and bike ramp will thus be provided the choice of merging left into the travel lane, or moving to the right onto the sidewalk. Where no bike lane is present on the approach to a roundabout, a bicycle ramp, if used, should be placed at least 50 feet (15 m) prior to the crosswalk at the roundabout. Bicycle ramps should be placed at a 35° to 45° angle to the roadway to enable cyclists to use the ramp even if pulling a trailer, but to discourage them from entering the sidewalk at high speed. Ideally, the sidewalk approaching the roundabout is separated from the roadway with a planter strip, allowing the ramp to be placed outside of the normal sidewalk area. In this case, the bike ramp can be fairly steep, as it is not intended for pedestrian use (up to 20% slope). If placed within the sidewalk area itself, the ramp slope must be built in a manner so that it is not a tripping hazard. A bicycle ramp should not be placed directly in line with the bicycle lane or otherwise placed in a manner that appears to encourage or require its use.

Since bike ramps can be confusing for pedestrians with visual impairments, detectable warnings should be included on the ramp. Where the ramp is placed in a planter strip, the detectable warnings should be placed at the top of the ramp, as the ramp itself is part of the hazardous vehicular area. If the ramp is in the sidewalk itself, the detectable warning should be placed at the bottom of the ramp. Other aspects of

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- 1 the bike ramp design and placement can help keep pedestrians from misconstruing the bike ramp as a
- 2 pedestrian crossing location. These aspects include the angle of the ramp, the possible steeper slope of
- 3 the ramp, and location of the ramp relatively far from the roundabout and marked crosswalk location.
- 4 Bicycle ramps at roundabout exits should be built with similar geometry and placement as the ramps at
- 5 roundabout entries. Bike ramps should be placed at least 50 feet (15 m) beyond the crosswalk at the
- 6 roundabout exit.

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WORKS CITED

1. **Federal Highway Administration.** *Manual on Uniform Traffic Control Devices.* Washington, D.C. : Federal Highway Administration, U.S. Department of Transportation, 2009.
2. **Torbic, D.J. et al.** *Guidance for the Design and Application of Shoulder and Centerline Rumble Strips.* Washington, DC : National Cooperative Highway Research Program, 2009. Report 641.
3. **AASHTO.** *A Policy on Geometric Design of Highways and Streets.* Washington, D.C. : American Association of State Highway and Transportation Officials, 2004.
4. *Relationship of Lane Width to Safety for Urban and Suburban Arterials.* **Potts, Harwood and Richards.** s.l. : TRB 2007 Annual Meeting, 2007.
5. **Iowa Department of Transportation.** *Guidelines for the Conversion of Urban Four-Lane Undivided Roadways to Three-Lane Two-Way Left-Turn Lane Facilities.* : Center for Transportation Research and Education - Iowa State University, 2001.
6. **Kein, L.L, Mills, M.K and Gibson, D.R.P.** *Traffic Detector Handbook.* Washington, DC : Federal Highway Administration, 2006.
7. **Moeur, Richard C. and Bina, Michelle N.** *Bicycle-Motor Vehicle Collisions on Controlled Access Highways in Arizona.* s.l. : Arizona Department of Transportation, 2002.
8. **Federal Highway Administration.** *Roundabouts: An Informational Guide.* 2000.
9. **Institute of Transportation Engineers.** *Urban Street Geometric Design Handbook.* 2008.
10. **Federal Highway Administration.** *Technical Advisory T 5040.35: Roadway Shoulder Rumble Strips.* 2001.

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CHAPTER 5: DESIGN OF SHARED USE PATHS**5.1. INTRODUCTION**

Shared use paths are bikeways that are physically separated from motorized vehicular traffic by an open space or barrier and either within the highway right-of-way or within an independent right-of-way. Shared use paths are sometimes referred to as “trails.” However, in many states the term “trail” means an unimproved recreational facility. Care should be taken when using these terms interchangeably. Where shared use paths are called trails, they should be designed based on the guidance in this manual.

Path users are generally non-motorized and may include but are not limited to: typical bicyclists, recumbent bicyclists, bicyclists pulling trailers, tandem bicyclists, in-line skaters, roller skaters, skateboarders, kick scooter users, and pedestrians, including walkers, runners, people using wheelchairs (both non-motorized and motorized), people with baby strollers, people walking dogs, and others. Paths are most commonly designed for two-way travel, and the guidance herein assumes a two-way facility is planned unless otherwise stated.

Shared use paths can serve a variety of purposes. They can provide users with a shortcut through a residential neighborhood (e.g., a connection between two cul-de-sac streets). They can provide a commuting route between residential areas and job centers. Located in a park or a greenway, they can provide an enjoyable recreational opportunity. Shared use paths can be located along rivers, ocean fronts, canals, abandoned or active railroad and utility rights-of-way, roadway corridors, limited access freeways, within college campuses, or within parks and open space areas. Shared use paths can also provide bicycle access to areas that are otherwise served only by limited-access highways.

Shared use paths should be thought of as a system of off-road transportation routes for bicyclists and other users that extends and complements the roadway network. Shared use paths should not be used to preclude on-road bicycle facilities, but rather to supplement a network of on-road bike lanes, shared roadways, bike boulevards, and paved shoulders. Shared use path design is similar to roadway design, but on a smaller scale and with typically lower design speeds.

5.1.1. ACCESSIBILITY REQUIREMENTS FOR SHARED USE PATHS

Due to the fact that nearly all shared use paths are used by pedestrians, they fall under the accessibility requirements of the Americans with Disabilities Act (ADA). The technical provisions herein either meet or exceed those recommended in current accessibility guidelines. Paths in a public right-of-way that function as sidewalks should be designed in accordance with the draft *Public Rights-Of-Way Accessibility*

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Guidelines (PROWAG) (1), or subsequent guidance that may supersede PROWAG in the future. These guidelines also apply to street crossings for all types of shared use paths.

Shared use paths built in independent corridors should meet the proposed accessibility standards described in the *Architectural Barriers Act Accessibility Guidelines for Outdoor Developed Areas (AGODA)* (2), or any subsequent guidance that supersedes AGODA. Again, the technical provisions in this manual either meet or exceed those recommended in AGODA.

5.2. ELEMENTS OF DESIGN

Shared use path design criteria are based on the physical and operating characteristics of bicycles and other path users, which are substantially different than motor vehicles. Due to a large percentage of path users being adult bicyclists, they are the primary design user for shared use paths and are the basis for most of the design recommendations in this chapter. This chapter also provides information on critical design issues and values for other potential design users, which should be used in the event that large volumes of these other user types are anticipated.

Some paths are frequently used by children. The operating characteristics of children are highly variable, and their specific characteristics have not yet been fully defined through research studies. However, it is generally assumed that the speed of child cyclists is lower than adult cyclists. Due to the fact that much of the design criteria in this Guide is based on design speed, children will be accommodated to a large extent. When considering criteria unrelated to design speed, engineering judgment should be used when modifying these values for children.

5.2.1. WIDTH AND CLEARANCE

The usable width and the horizontal clearance required for a shared use path are primary design considerations. Exhibit 5.1 depicts the typical cross section of a shared use path. The appropriate paved width for a shared use path is dependent on the context, volume, and mix of users. The minimum paved width for a two-directional shared use path is 10 feet (3.0 m). Typically, widths range from 10 – 14 feet (3.0 – 4.3 m), with the wider values applicable to areas with high use and/or a wider variety of user groups.

In very rare circumstances, a reduced width of 8 feet (2.4 m) may be used where the following conditions prevail:

- Bicycle traffic is expected to be low, even on peak days or during peak hours.
- Pedestrian use of the facility is not expected to be more than occasional.
- Horizontal and vertical alignments provide safe and frequent passing opportunities.

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- The path will not be regularly subjected to maintenance vehicle loading conditions that would cause pavement edge damage.

In addition, a path width of 8 feet (2.4 m) may be used for a short distance due to a physical constraint such as an environmental feature, bridge abutment, utility structure, fence, etc. Warning signs that indicate the pathway narrows, per the *Manual on Uniform Traffic Control Devices* (MUTCD) (3) should be considered at these locations.

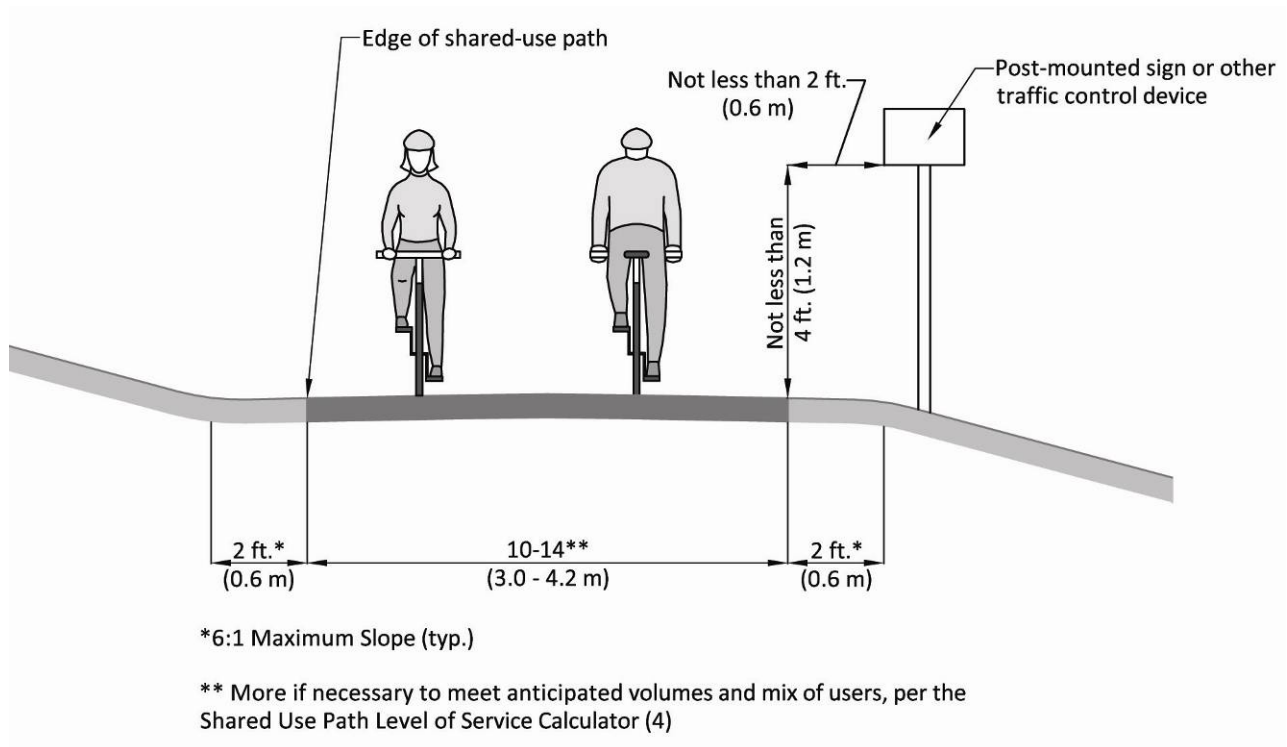


Exhibit 5.1. Typical Cross Section of Two-Way Shared Use Path on Independent Alignment

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A wider path is needed to provide an acceptable level of service on pathways that are frequently used by pedestrians and wheeled users. The Shared Use Path Level of Service Calculator is helpful in determining the appropriate width of a pathway given existing or anticipated user volumes and mixes. (4) Wider pathways, typically 11-14 feet (3.4-4.2 m) are recommended in locations that are anticipated to serve a high percentage of pedestrians (up to 30% of the total pathway volume) and high user volumes (more than 300 total users in the peak hour). Eleven-foot (3.4 m) wide pathways are necessary to enable a bicyclist to pass another path user going the same direction, at the same time a path user is approaching from the opposite direction (see Exhibit 5.2). (5) Wider paths are also advisable in the following situations:

- Where there is significant use by in-line skaters, adult tricycles, or other users that need more operating width (see Chapter 3);
- Where the path is used by larger maintenance vehicles;
- On steep grades to provide additional passing area; or
- Through curves to provide more operating space.

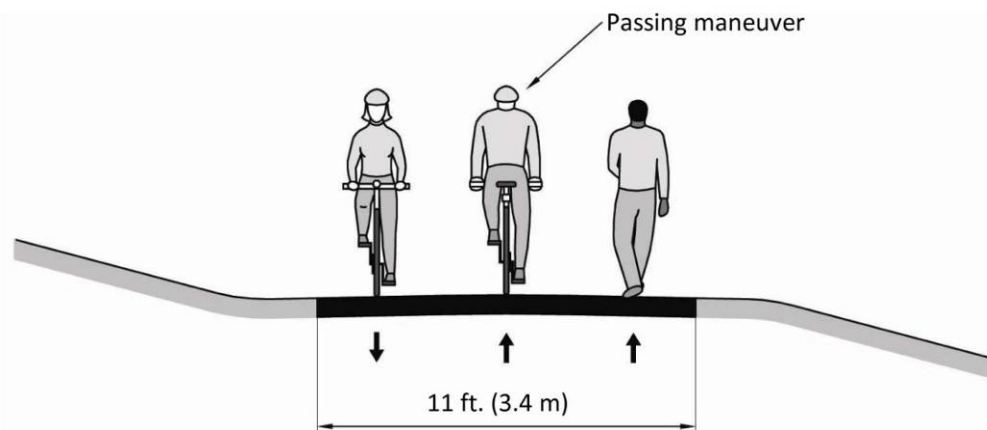


Exhibit 5.2. Minimum Width Needed to Facilitate Passing on a Shared Use Path

Under most conditions, it is not necessary to segregate pedestrians and bicyclists on a shared use path, even in areas with high user volumes – they can typically coexist. Path users customarily keep right except to pass. Signs may be used to remind bicyclists to pass on the left and to give an audible warning prior to passing other slower users. Part 9 of the MUTCD provides a variety of regulatory signs that can be used for this purpose.

On pathways with heavy peak hour and/or seasonal volumes, or other operational challenges such as sight distance constraints, the use of a centerline stripe on the path can help clarify the direction of travel and organize pathway traffic. A solid yellow centerline stripe may be used to separate two directions of travel where passing is not permitted, and a broken yellow line may be used where passing

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1 is permitted. The centerline can either be continuous along the entire length of the path, or may be
2 used only in locations where operational challenges exist. Per the MUTCD, all markings used on
3 bikeways should be retroreflectorized.

4 In areas with extremely heavy pathway volumes, segregation of pedestrians from wheeled users may be
5 appropriate; however care must be taken to ensure the method of segregation is simple and
6 straightforward. Pedestrians are typically provided with a bi-directional walking lane on one side of the
7 pathway, while bicyclists are provided with directional lanes of travel. This solution should only be used
8 when a minimum path width of 15 feet (4.6 m) is provided, with at least 10 feet (3 m) for two-way
9 wheeled traffic, and at least 5 feet (1.5 m) for pedestrians.

10 Where this type of segregation is used on a path with a view (e.g. adjacent to a lake or river), the
11 pedestrian lane should be placed on the side of the path with the view. Again, this solution should only
12 be used for pathways with heavy volumes, as pedestrians will often walk in the “bicycle only” portion of
13 a pathway unless it is heavily traveled by bicycles.

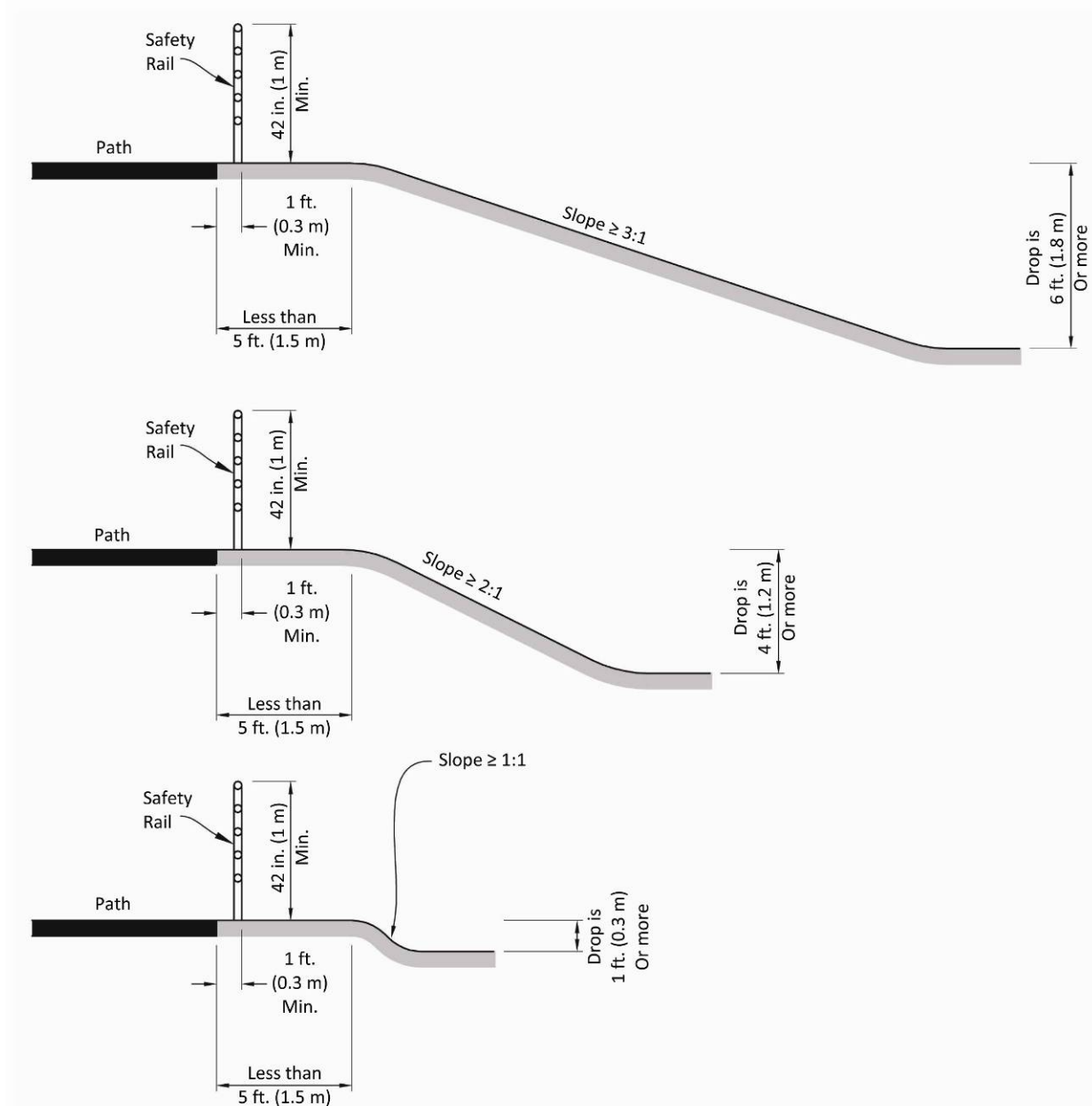
14 Another solution is to provide physically separated pathways for pedestrians and wheeled users. A
15 number of factors should be considered when determining whether to provide separate paths, such as
16 general site conditions (i.e., the width of separation and setting), origins and destinations of different
17 types of path users, and the anticipated level of compliance of users choosing the appropriate path. In
18 some instances the dual paths may have to come in close proximity or be joined for a distance due to
19 site constraints. As allowed by the MUTCD (3) and described in more detail in Section 5.4.2., mode-
20 specific signs may be used to guide users to their appropriate paths.

21 Ideally, a graded area (shoulder) at least 3 – 5 feet (0.9-1.5 m) wide with a maximum cross-slope of 6:1
22 should be maintained on each side of the pathway. At a minimum, a 2-foot (0.6 m) graded area with a
23 maximum 6:1 slope should be provided for clearance from lateral obstructions such as bushes, large
24 rocks, bridge piers, abutments, and poles. Where “smooth” features such as bicycle railings or fences are
25 introduced with appropriate flaring end treatments (as described below), a lesser clearance (not less
26 than 1 ft) is acceptable. If adequate clearance cannot be provided between the path and lateral
27 obstructions, then warning signs, object markers, or enhanced conspicuity and reflectorization of the
28 obstruction should be used.

29 Where a path is adjacent to parallel water hazards or downward slopes equal to or steeper than 3:1, a
30 wider separation should be considered. A 5-foot (1.5 m) separation from the edge of the path pavement
31 to the top of the slope is desirable. Depending on the height of the embankment and condition at the
32 bottom, a physical barrier, such as dense shrubbery, railing, or fencing may be needed. This is an area
33 where engineering judgment must be applied, as it is necessary to compare the risk for an errant
34 bicyclist that swerves off the path to the risk of the rail itself. Where a recovery area (i.e., distance
35 between the edge of the path pavement and the top of the slope) is less than 5 feet (1.5 m), physical
36 barriers or rails are recommended in the following situations (see Exhibit 5.3).

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- 1 Slopes 1:1 or steeper, with a drop of 1 foot (0.3 m) or greater
- 2 Slopes 2:1 or steeper, with a drop of 4 feet (1.2 m) or greater
- 3 Slopes 3:1 or steeper, with a drop of 6 feet (1.8 m) or greater
- 4 Slopes 3:1 or steeper, adjacent to a parallel water hazard or other obvious hazard

**Exhibit 5.3. Safety Rail between Path and Adjacent Slope**

The barrier or rail should begin prior to, and extend beyond the area of need. The lateral offset of the barrier should be at least 1 foot (0.3 m) from the edge of the path. The ends of the barrier should be

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1 flared away from the path edge. Barrier or rail ends that remain within the 2-foot (0.6 m) clear area
2 should be marked with object markers.

3 Railings that are used to protect users from slopes or to discourage path users from venturing onto a
4 roadway or neighboring property can typically have relatively large openings. A typical design includes
5 two to four horizontal elements with vertical elements spaced fairly widely, but frequently enough to
6 provide the necessary structural support. Where the path-side hazard is a high vertical drop or a body of
7 water, engineering judgment should be used to determine whether a railing suitable for bridges (as
8 described in Section 5.2.10.) should be used.

9 Other materials in addition to railings can be used to separate paths from adjacent areas, either due to
10 hazardous conditions or to discourage pathway users from venturing onto adjacent properties. Berms
11 and/or vegetation can serve this function.

12 It is not desirable to place the pathway in a narrow corridor between two fences for long distances, as
13 this creates an uncomfortable experience for the user and prevents path users from leaving the path in
14 the event of an emergency.

15 The desirable vertical clearance to obstructions is 10 feet (3.0 m). Fixed objects should not be permitted
16 to protrude within the vertical or horizontal clearance of a shared use path. 8 feet (2.4 m) is the
17 recommended minimum vertical clearance that can be used in constrained areas. In some situations,
18 vertical clearance greater than 10 feet (3.0 m) may be necessary to permit passage of maintenance and
19 emergency vehicles.

5.2.2. SHARED USE PATHS ADJACENT TO ROADWAYS (SIDEPATHS)

21 While it is generally preferable to select path alignments in independent rights-of-way, there are
22 situations where existing roads provide the only corridors available. Consideration is sometimes given to
23 placing paths adjacent to the roadway (also called sidepaths), where right-of-way and other physical
24 constraints dictate. However, as stated in Chapter 2, provision of a pathway adjacent to the road is
25 generally not a substitute for the provision of on-road accommodation such as paved shoulders or bike
26 lanes, but may be considered in some locations in addition to on-road bicycle facilities, or as an interim
27 accommodation until roadway conditions can be improved. A sidepath should satisfy the same design
28 criteria as shared use paths in independent corridors.

29 The discussion in this section refers to two-way sidepaths. Additional design considerations for
30 sidepaths are provided in Section 5.3.4. of this chapter.

31 Paths can function along highways for short sections, or for longer sections where there are few street
32 and/or driveway crossings, given appropriate separation between facilities and attention to user safety

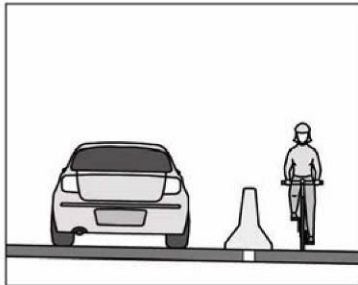
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at junctions. However before committing to this option for longer distances on urban and suburban streets with many driveways and street crossings, practitioners should be aware that two-way sidepaths can create operational and safety problems; primarily arising from the fact that bicycle traffic in one direction rides against the flow of adjacent roadway traffic, contrary to normal rules of the road and driver expectations. See Exhibit 5.4 for examples of safety issues associated with sidepaths. These issues include:

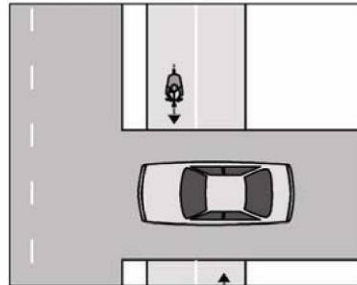
1. At intersections and driveways, motorists entering or crossing the roadway often will not notice bicyclists approaching from their right, as they do not expect wheeled traffic from this direction. Motorists turning from the roadway onto the cross street may likewise fail to notice bicyclists traveling the opposite direction from the norm.
2. Bicyclists traveling against the flow of traffic on sidepaths are apt to cross intersections and driveways at unexpected speeds (i.e., at speeds that are significantly faster than pedestrian speeds). This exacerbates crash risk, especially where sight distance is limited.
3. Motor vehicles waiting to enter the roadway from a driveway or side street may block the sidepath crossing, as drivers pull forward to get an unobstructed view of traffic (this is the case at many sidewalk crossings, as well).
4. Although the shared use path should be given the same priority through intersections as the parallel highway, some motorists mistakenly expect bicyclists to yield at all cross streets and driveways. Attempts to require bicyclists to yield or stop at each cross-street or driveway are inappropriate and are typically not effective.
5. Where the sidepath ends, bicyclists traveling in the direction opposed to roadway traffic may be encouraged to continue on the wrong side of the roadway. Similarly, bicyclists approaching a path may travel on the wrong side of the roadway to access the path. Wrong-way travel by bicyclists is a common factor in bicycle-automobile crashes.
6. Depending upon the bicyclist's specific origin and destination, a two-way sidepath on one side of the road may require additional road crossings (and therefore increased exposure), however the sidepath may also reduce the number of road crossings for some bicyclists.
7. Signs posted for roadway users are backwards for contra-flow riders, who are apt not to notice such information. The same applies to traffic signal faces that are not oriented to contra-flow riders.
8. Because of proximity of roadway traffic to opposing path traffic, barriers or railings are sometimes necessary to keep traffic from the roadway or path from inappropriately entering the other way. These barriers can represent an obstruction to bicyclists and motorists and can complicate path maintenance.
9. Bicyclists using a sidepath may conflict with pedestrians and other slower path users.
10. Sidepath width is sometimes constrained by fixed object hazards (such as utility poles, trash cans, mailboxes, etc).
11. Some bicyclists will use the roadway instead of the sidepath because of the operational issues described above. Bicyclists using the roadway may be harassed by motorists who believe

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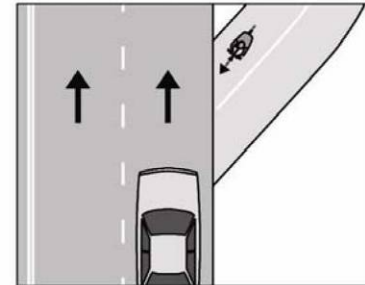
- 1 bicyclists should use the sidepath. In addition, there are some states that prohibit bicyclists from
 2 using the adjacent roadway when a sidepath is present.



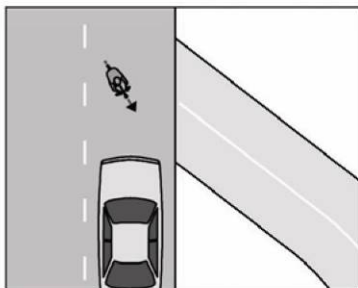
Barriers, while needed in tight spaces, can narrow both roadway and path and create hazards.



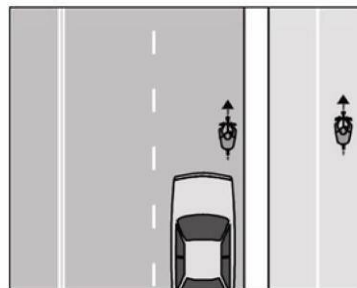
Stopped motor vehicles on side streets or driveways may block the path.



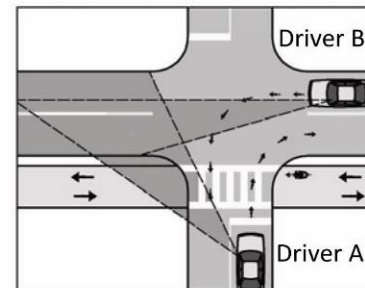
At path's end, bicyclists going against traffic may continue riding the wrong way.



To get to a path entrance, bicyclists may ride against traffic or make unanticipated crossings.



Some bicyclists may find the road cleaner, safer, and more convenient, frustrating some motorists.



Right turning Driver A is looking for traffic on the left; left turning driver B is looking for traffic ahead. In both cases, a contra-flow bicyclist is not in the drivers' main field of vision.

- 3
 4 **Exhibit 5.4. Sidepath Conflicts**

- 5 For these reasons, other types of bikeways may be better suited to accommodate bicycle traffic along
 6 some roadways. Shared-use paths in road medians are generally not recommended. These facilities
 7 result in multiple conflicting turning movements by motorists and bicyclists at intersections. Therefore,
 8 shared use paths in medians should be considered only where these turning conflicts can be avoided or
 9 mitigated through signalization or other techniques.

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GUIDELINES FOR SIDEPATHS

Although paths in independent rights-of-way are preferred, sidepaths may be considered where one or more of the following conditions exist:

- The adjacent roadway has relatively high-volume and high-speed motor vehicle traffic that might discourage many bicyclists from riding on the roadway, potentially increasing sidewalk riding, and there are no practical alternatives for either improving the roadway or accommodating bicyclists on nearby parallel streets.
- The sidepath is used for a short distance to provide continuity between sections of path in independent rights-of-way, or to connect local streets that are used as bicycle routes.
- The sidepath can be built with few roadway and driveway crossings.
- The sidepath can be terminated at each end onto streets that accommodate bicyclists, onto another path, or in a location that is otherwise bicycle compatible.

In some situations, it may be possible to place one-way sidepaths on both sides of the street or highway, directing wheeled users to travel in the same direction as adjacent motor vehicle traffic. Clear directional information is needed if this type of design is used, as well as appropriate intersection design to enable bicyclists to safely cross to the other side of the roadway. This can reduce *some* of the problems associated with two-way sidepaths at driveways and intersections; however, it should be done with the understanding that many bicyclists will ignore the directional indications if they involve additional crossings or otherwise inconvenient travel patterns.

Separation is desirable between a two-way sidepath and the adjacent roadway to demonstrate to both the bicyclist and the motorist that the path functions as an independent facility for bicyclists and other users. The minimum recommended distance between a path and the roadway curb or edge of pavement (where there is no curb) is 5 feet (1.5 m). Where the separation is less than 5 feet (1.5 m), a physical barrier or railing should be provided between the path and the roadway. Such barriers or railings serve both to prevent path users from making undesirable or unintended movements from the path to the roadway and to reinforce the concept that the path is an independent facility. Where used, the barrier or railing should be a minimum of 42 inches (1 m) high. A barrier or railing between a shared use path and adjacent highway should not impair sight distance at intersections, and should be designed not to pose a hazard to errant motorists. The barrier or railing need not be of size and strength to redirect errant motorists toward the roadway, unless other conditions require a crashworthy barrier.

Where a sidepath is placed along a high-speed highway, a separation greater than 5 feet (1.5 m) is desirable for safety and path user comfort. If greater separation cannot be provided, use of a crashworthy barrier should be considered. See Section 5.3.4. for guidance on the design of sidepath intersections.

DRAFT FOR AASHTO COMMITTEE REVIEW AND COMMENT**5.2.3. SHARED USE WITH MOPEDS, MOTORCYCLES, SNOWMOBILES, AND HORSES**

Although in some jurisdictions it may be permitted, it is undesirable to mix mopeds, motorcycles, or all-terrain vehicles with bicyclists and pedestrians on shared-use paths. In general, these types of motorized vehicles should not be allowed on shared use paths because of conflicts with slower moving bicyclists and pedestrians. Motorized vehicles also diminish the quiet, relaxing experience most users seek on paths. Motorized wheelchairs are an exception to this rule, and should be permitted to access shared use paths.

In cases where mopeds or other similar motorized users are permitted and are expected to use the pathway, it is necessary to reduce conflicts by providing additional width, signing, and striping. Signs that emphasize appropriate user etiquette may be a particular need for these paths.

Bicycling and equestrian use have successfully been integrated on many pathways in the U.S. However, care must be taken in designing these facilities to reduce potential conflicts between users. Bicyclists are often unaware of the need for slower speeds and additional clearance around horses. Horses can be startled easily and may act unpredictably if they perceive approaching bicyclists as a danger. Measures to mitigate bicyclist-equestrian conflicts include provision of separate bridle paths, maintenance of adequate sight lines so that bicyclists and equestrians are able to see each other well in advance, and signing that clarifies appropriate passing techniques and yielding responsibilities. Where used, a separate, unpaved bridle path can often serve a dual purpose, as many joggers also prefer unpaved surfaces (see Exhibit 5.5).

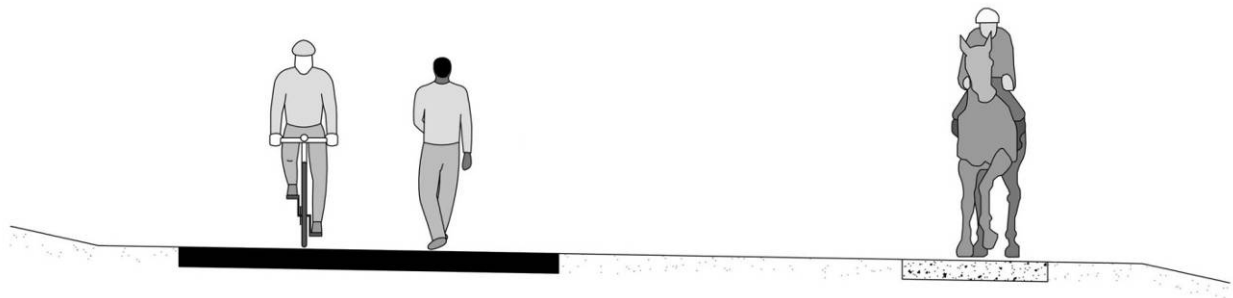


Exhibit 5.5. Shared Use Path with Separate Unpaved Equestrian/Jogger Path

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The speed a path user travels is dependent on several factors, including the physical condition of the user; the type and condition of the user's equipment; the purpose of the trip; the condition, location and grade of the path; the prevailing wind speed and direction; and the number and types of other users on the path. In most situations shared use paths should be designed for a speed that is at least as high as the preferred speed of the fastest common user.

There is no single design speed that is recommended for all paths. When selecting an appropriate design speed for a specific path, planners and designers should consider several factors including the context of the path, the types of users expected, the terrain the path runs through, prevailing winds, the path surface, and other path characteristics. The following examples help to illustrate these factors:

- *Types of users and context.* An urban path with a variety of users and frequent conflicts and constraints may be designed for lower speeds than a rural path with few conflicts that is primarily used by recreational cyclists (potentially including recumbent bicyclists, whose 85th percentile speed is 18 mph).
- *Terrain.* A path in fairly hilly terrain should be designed for a higher speed.
- *Path surface.* Bicyclists tend to ride slower on unpaved paths, so a lower design speed may be used.

In street and highway design, design speeds are generally selected in 5 mph or 10 km/h increments, which is appropriate based on the approximate 85th percentile speed range on various types of roadways of 20 mph (30 km/h) to 75 mph (120 km/h) or higher. On paths, the range of speeds is much smaller, ranging as low as 12 mph (19 km/h) to 30 mph (50 km/h). Therefore, design speeds for paths can be selected in 2 mph (3 km/h) increments. Design criteria for geometric features in this document are provided in 2 mph (3 km/h) increments for the slower end of the scale [design speeds between 12 mph (19 km/h) and 20 mph (32 km/h)]. For design speeds above 20 mph (32 km/h), 5 mph increments are used.

The following guidance and the aforementioned consideration of various factors should drive the selection of an appropriate design speed:

- For most paths in relatively flat areas (grades less than 2%), a design speed of 18 mph (30 km/h) is generally sufficient, except on inclines where higher speeds can occur. The design speed should not be lower than 14 mph (23 km/h), except in rare circumstances where the context and user types support a lower speed.
- In areas with hilly terrain and sustained steeper grades, the appropriate design speed should be selected based on the anticipated travel speeds of cyclists traveling downhill. In all but the most extreme cases, 30 mph (48 km/h) is the maximum design speed that should be used.

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Lower speeds can improve path safety when approaching crossings or potential hazards by allowing the path user to better perceive the crossing situation or hazard. It is important to give the bicyclist adequate warning (either through signs or by ensuring adequate sight lines) prior to areas of the pathway where lower design speeds are employed. See Section 5.4.2. for guidance on warning signs.

Geometric design and traffic control devices can be used to reduce path users' speed and to encourage faster bicyclists to use the roadway system where appropriate. Speeds can be reduced by geometric features such as horizontal curvature.

Effectiveness of speed control through design is limited if bicyclists can veer off a path to "straighten out" curves, and speed limit signs on paths may not be effective, as most bicyclists do not use speedometers. Traffic management through use of a centerline stripe can be as effective as geometric design in reducing speeds and addressing conflicts in some locations.

5.2.5. HORIZONTAL ALIGNMENT

The typical adult bicyclist is the design user for horizontal alignment. The minimum radius of horizontal curvature for bicyclists can be calculated using two different methods. One method uses "lean angle," and the other method uses superelevation and coefficient of friction. As detailed below, in general, the lean angle method should be used in design, although there are situations where the superelevation method is helpful.

CALCULATING MINIMUM RADIUS USING LEAN ANGLE

Unlike an automobile, a bicyclist must lean while cornering to prevent falling outward due to forces associated with turning movements. Most bicyclists usually do not lean drastically; 20 degrees is considered the typical maximum lean angle for most users. (6) Assuming an operator who sits straight in the seat, a simple equation can determine the minimum radius of curvature for any given lean angle and design speed:

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US Customary	Metric
$R = \frac{0.067V^2}{\tan \theta}$	$R = \frac{0.0079V^2}{\tan \theta}$
where: R = minimum radius of curvature (ft) V = design speed (mph) θ = lean angle from the vertical (degrees)	where: R = minimum radius of curvature (m) V = design speed (km/h) θ = lean angle from the vertical (degrees)

Equation 5-1. Minimum Radius of Curvature Based on Lean Angle

As described in Section 5.1.1., shared use paths must meet accessibility guidelines, which restrict the steepness of cross slopes. One percent slopes are recommended on shared use paths where feasible, because they are easier to navigate for people using wheelchairs. In most cases the lean angle formula should be used when determining the minimum radius of a horizontal curve, due to the need for relatively flat cross slopes and the fact that bicyclists lean when turning (regardless of their speed or the radius of their turn). The curve radius should be based upon various design speeds of 12 to 30 mph (19-48 km/h) and a desirable maximum lean angle of 20 degrees. Minimum radii of curvature for a paved path can be selected from Exhibit 5.6.

US Customary		Metric	
Design Speed (mph)	Minimum Radius (ft)	Design Speed (km/h)	Minimum Radius (m)
12	27	19	8
14	36	23	11
16	47	26	15
18	60	29	18
20	74	32	22
25	115	40	35
30	166	48	50

Exhibit 5.6. Desirable Minimum Radii for Horizontal Curves on Paved Shared Use Paths at 20 Degree Lean Angle**CALCULATING MINIMUM RADIUS USING SUPERELEVATION**

The second method of calculating minimum radius of curvature negotiable by a bicycle uses the design speed, the superelevation rate of the pathway surface, and the coefficient of friction between the bicycle tires and the surface, as shown in the following formula:

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US Customary	Metric
$R = \frac{V^2}{15\left(\frac{e}{100} + f\right)}$	$R = \frac{V^2}{127\left(\frac{e}{100} + f\right)}$
where:	where:
R = minimum radius of curvature (ft)	R = minimum radius of curvature (m)
V = design speed (mph)	V = design speed (km/h)
e = rate of bikeway superelevation (percent)	e = rate of bikeway superelevation (percent)
f = coefficient of friction	f = coefficient of friction

Equation 5-2. Minimum Radius of Curvature Based on Superelevation

The coefficient of friction depends upon speed, surface type and condition, tire type and condition, and whether the surface is wet or dry. Friction factors used for design should be selected based upon the point at which turning forces or perceived lack of surface traction causes the bicyclist to recognize a feeling of discomfort and instinctively act to avoid higher speed. Extrapolating from values used in highway design, design friction factors for paved shared use paths can be assumed to vary from 0.34 at 6 mph (10 km/h) to 0.21 at 30 mph (48 km/h). On unpaved surfaces friction factors should be reduced by 50 percent to allow a sufficient margin of safety.

Calculating minimum radius based on superelevation may be useful on unpaved paths, where bicyclists may be hesitant to lean as much while cornering due to the perceived lack of traction. In these situations, the superelevation formula should be used with appropriate friction factors for unpaved surfaces. Calculating minimum radius based on superelevation may also be useful on paved paths intended for bicycle use only, allowing higher design speeds to be accommodated on relatively sharp curves with cross slopes (superelevation) up to 8 percent.

When a radius is smaller than that needed for an 18 mph (29 km/h) design speed, standard turn or curve warning signs (W1 series) should be installed in accordance with the MUTCD. (3) Smaller radius curves are typically used when there are constrained site conditions, topographic challenges, or a desire to reduce path user speeds. The negative effects of sharper curves can also be partially offset by widening the pavement through the curves.

5.2.6. CROSS SLOPE

As previously described, shared use paths must be accessible to people with disabilities. Shared use paths located adjacent to roadways essentially function as sidewalks, and therefore should follow PROWAG (1), which requires that cross slopes not exceed 2 percent. Paths in independent rights-of-way

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should be designed according to AGODA (7), which requires that cross slopes not exceed 5 percent. As described in the previous section, 1 percent cross slopes are recommended on shared use paths, to better accommodate people with disabilities and to provide enough slope to convey surface drainage in most situations.

Because this guide recommends a relatively flat cross slope of 1 percent, and because horizontal curvature can be based on a 20-degree lean angle, superelevation for horizontal curvature is not required. Because superelevation is not needed for horizontal curvature, cross slopes can follow the direction of the existing terrain. This practice enables the designer to better accommodate surface drainage and lessen construction impacts.

If cross slopes steeper than 2 percent are necessary, they should be sloped to the inside of horizontal curves regardless of drainage conditions. Steeper cross slopes (up to 5%) may occasionally be desirable on unpaved shared use paths to reduce the likelihood of puddles caused by surface irregularities and to allow increased superelevation to achieve smaller radii of curvature, as previously described in the subsection on horizontal alignment. In the rare situation where a path is intended for bicycle use only (e.g. pedestrians are accommodated on a separate pathway) and does not need to meet accessibility guidelines, cross slopes between 5 and 8 percent can be used to allow for smaller minimum horizontal curve radii, as discussed above.

Cross slopes must be transitioned to connect to existing slopes, or to adjust to a reversal of predominant terrain slope or drainage, or to a horizontal curve in some situations. Cross slope transitions should be comfortable for the path user. A minimum transition length of 5 feet (1.5 m) for each 1 percent change in cross slope should be used.

5.2.7. GRADE

For pathways adjacent to roads (sidepaths), pathway grade should generally match the grade of the adjacent roadway. Grades on shared use paths in independent corridors should be kept to a minimum, especially on long inclines. Grades greater than 5 percent are undesirable because the ascents are difficult for many path users, and the descents cause some users to exceed the speeds at which they are competent or comfortable. In addition, because shared use paths are generally open to pedestrians, the allowable grades on paths are subject to the accessibility guidelines described in Section 5.1.1.

Grades on paths in independent rights-of-way should be limited as follows (2):

- 5% maximum for any distance
- 8.3% maximum for up to 200 feet (61 m)
- 10% maximum for up to 30 feet (9 m)
- 12.5% for up to 10 feet (3 m)

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1 Additionally, no more than 30 percent of the total path length should have a grade exceeding 8.3
2 percent. Where grades exceed 5 percent, a resting interval is required at the end of any segment of
3 maximum length as described above. A resting interval must be at least 5 feet (1.5 m) long, be as wide
4 as the path, and have a maximum slope not exceeding 5 percent in any direction. Smooth, gradual
5 transitions must be provided between the sloped segments and the resting intervals.

6 Options to mitigate excessive grades on shared use pathways include the following:

- 7 • Use higher design speeds for horizontal and vertical curvature, stopping sight distance, and
8 other geometric features.
- 9 • When using a longer grade, consider an additional 4-6 feet (1.2-1.8 m) of width to permit slower
10 bicyclists to dismount and walk uphill, and to provide more maneuvering space for fast downhill
11 bicyclists.
- 12 • Install the Hill warning sign for bicyclists (W7-5) and advisory speed plaque, if appropriate, per
13 the MUTCD (3).
- 14 • Provide signing that alerts path users to the maximum percent of grade as shown in the MUTCD
15 (3).
- 16 • Exceed minimum horizontal clearances, recovery area, and/or protective railings.
- 17 • If other designs are not practicable, use a series of short switchbacks to traverse the grade. If
18 this is done, an extra 4 to 6 feet (1.2 to 1.8 m) of path width is recommended to provide
19 maneuvering space.
- 20 • Provide resting intervals with flatter grades, to permit users to stop periodically and rest.

21 Grades steeper than 3 percent may not be practical for shared use paths with crushed stone or other
22 unpaved surfaces for both bicycle handling and drainage erosion reasons. Typically, grades less than 0.5
23 percent should be avoided, because they are not efficient in conveying surface drainage. Where paths
24 are built in very flat terrain, proposed path grades can be increased to provide a gradually rolling vertical
25 profile that helps convey surface drainage to outlet locations.

26 **5.2.8. STOPPING SIGHT DISTANCE**

27 To provide path users with opportunities to see and react to unexpected conditions, shared use paths
28 should be designed with adequate stopping sight distances. The distance required to bring a path user
29 to a fully controlled stop is a function of the user's perception and braking reaction times, the initial
30 speed, the coefficient of friction between the wheels and the pavement, the braking ability of the user's
31 equipment, and the grade. The coefficient of friction for the typical bicyclist is 0.32. Exhibit 5.7 indicates
32 the minimum stopping sight distance for various design speeds and grades based on a total perception
33 and brake reaction time of 2.5 seconds and a coefficient of friction of 0.16 (Exhibit 3.4), appropriate for
34 wet conditions. Minimum stopping sight distance can also be calculated using the following equation:

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US Customary	Metric
$S = \frac{V^2}{30(f \pm G)} + 3.67V$	$S = \frac{V^2}{254(f \pm G)} + \frac{V}{1.4}$
where: S = stopping sight distance (ft) V = velocity (mph) f = coefficient of friction (use 0.16 for a typical bike) G = grade (ft/ft) (rise/run)	where: S = stopping sight distance (m) V = velocity (km/h) f = coefficient of friction (use 0.16 for a typical bike) G = grade (m/m) (rise/run)

Equation 5-3. Minimum Stopping Sight Distance

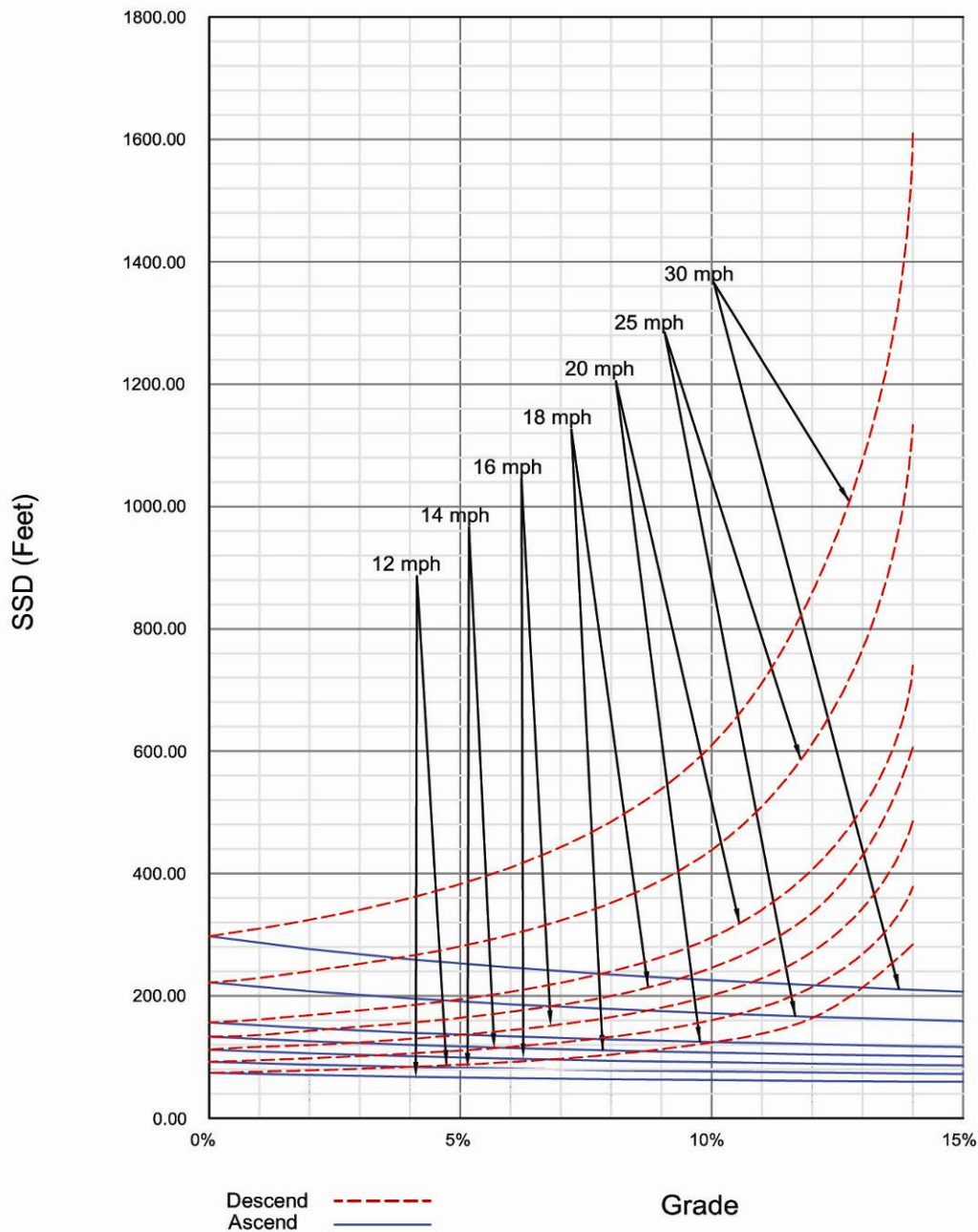
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Exhibit 5.7. US Customary Units. Minimum Stopping Sight Distance vs. Grades for Various Design Speeds

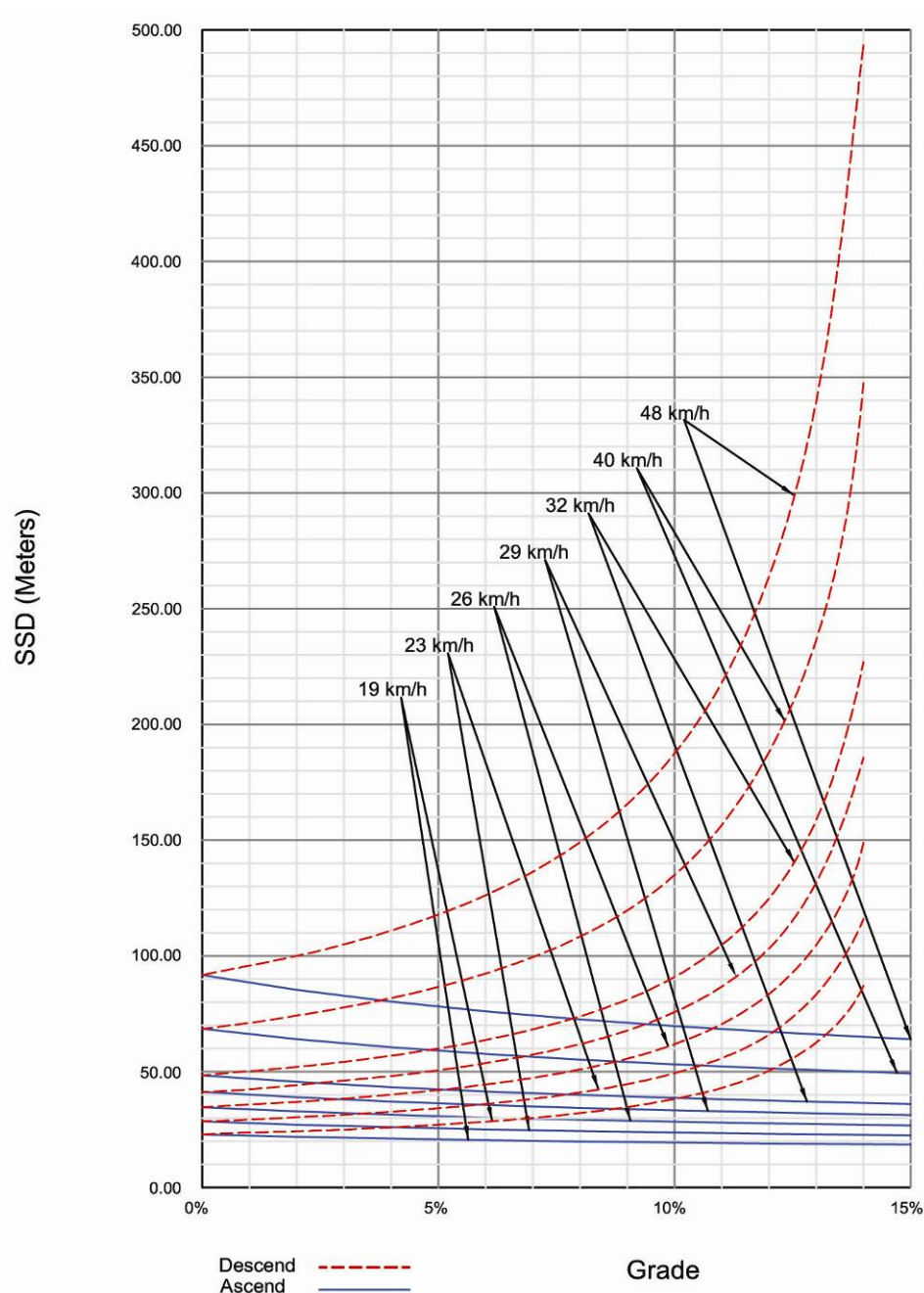
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Exhibit 5.7. Metric Units. Minimum Stopping Sight Distance vs. Grades for Various Design Speeds (Continued)

Research indicates that the coefficient of friction of various other path users range from 0.20 for inline skaters to 0.30 for recumbent bicyclists. If users with lower coefficients of friction such as inline skaters

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or recumbent bicyclists are expected to make up a relatively large percentage of path users, stopping sight distances should be increased. For two-way shared use paths, the sight distance in the descending direction, that is, where “G” is defined as negative, will control the design.

Exhibit 5.8 is used to select the minimum length of vertical curve necessary to provide minimum stopping sight distance at various speeds on crest vertical curves. The eye height of the typical adult bicyclist is assumed to be 4.5 feet (1.4 m), and the object height is assumed to be 0 inches (0 mm) to recognize that impediments to bicycle travel exist at pavement level. The minimum length of vertical curve can also be calculated using the following equation:

US Customary	Metric
$S > L \quad L = 2S - \frac{200(\sqrt{h_1} + \sqrt{h_2})^2}{A}$ $S < L \quad L = \frac{AS^2}{100(\sqrt{2h_1} + \sqrt{2h_2})^2}$	$S > L \quad L = 2S - \frac{200(\sqrt{h_1} + \sqrt{h_2})^2}{A}$ $S < L \quad L = \frac{AS^2}{100(\sqrt{2h_1} + \sqrt{2h_2})^2}$
where:	where:
L = minimum length of vertical curve (ft)	L = minimum length of vertical curve (m)
A = algebraic grade difference (percent)	A = algebraic grade difference (percent)
S = stopping sight distance (ft)	S = stopping sight distance (m)
h ₁ = eye height (4.5 ft for a typical bicyclist)	h ₁ = eye height (1.4 m for a typical bicyclist)
h ₂ = object height (0 ft)	h ₂ = object height (0 m)

Equation 5-4. Length of Crest Vertical Curve to Provide Sight Distance

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US Customary															
A (%)	S=Stopping Sight Distance (ft)														
	20	40	60	80	100	120	140	160	180	200	220	240	260	280	300
2												30	70	110	150
3								20	60	100	140	180	220	260	300
4						15	55	95	135	175	215	256	300	348	400
5					20	60	100	140	180	222	269	320	376	436	500
6				10	50	90	130	170	210	267	323	384	451	523	600
7				31	71	111	151	191	231	311	376	448	526	610	700
8			8	48	88	128	168	208	248	356	430	512	601	697	800
9			20	60	100	140	180	220	260	400	484	576	676	784	900
10			30	70	110	150	190	230	270	444	538	640	751	871	1000
11			38	78	118	158	198	238	278	489	592	704	826	958	1100
12		5	45	85	125	165	205	245	285	533	645	768	901	1045	1200
13		11	51	91	131	171	211	251	291	578	699	832	976	1132	1300
14		16	56	96	136	176	216	256	296	622	753	896	1052	1220	1400
15		20	60	100	140	180	220	260	300	667	807	960	1127	1307	1500
16		24	64	104	144	184	224	264	304	711	860	1024	1202	1394	1600
17		27	67	107	147	187	227	267	307	756	914	1088	1277	1481	1700
18		30	70	110	150	190	230	270	310	800	968	1152	1352	1568	1800
19		33	73	113	153	193	233	273	313	844	1022	1216	1427	1655	1900
20		35	75	115	155	195	235	275	315	889	1076	1280	1502	1742	2000
21		37	77	117	157	197	237	277	317	933	1129	1344	1577	1829	2100
22		39	79	119	159	199	239	279	319	978	1183	1408	1652	1916	2200
23		41	81	121	161	201	241	281	321	1022	1237	1472	1728	2004	2300
24	3	43	83	123	163	203	243	283	323	1067	1291	1536	1803	2091	2400
25	4	44	84	124	164	204	244	284	324	1111	1344	1600	1878	2178	2500

————— Represents S=L
 Minimum Length of Vertical Curve = 3 ft

Exhibit 5.8. Minimum Length of Crest Vertical Curve Based on Stopping Sight Distance

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A (%)	Metric S=Stopping Sight Distance (m)																		
	10	15	20	25	30	35	40	45	50	55	60	65	70	75	80	85	90	95	100
2														10	20	30	40	50	60
3									7	17	27	37	47	57	67	77	87	97	107
4						0	10	20	30	40	50	60	70	80	91	103	116	129	143
5				4	14	24	34	44	54	64	75	88	100	114	129	145	161	179	
6			3	13	23	33	43	54	65	77	91	105	121	137	155	174	193	214	
7			10	20	30	40	51	63	76	90	106	123	141	160	181	203	226	250	
8		5	15	25	35	46	58	71	86	103	121	140	161	183	206	231	258	286	
9		9	19	29	39	51	65	80	97	116	136	158	181	206	232	260	290	321	
10		2	12	22	32	44	57	72	89	108	129	151	175	201	229	258	289	322	357
11		5	15	25	35	48	63	80	98	119	141	166	193	221	251	284	318	355	393
12		7	17	27	39	53	69	87	107	130	154	181	210	241	274	310	347	387	429
13		8	18	29	42	57	74	94	116	140	167	196	228	261	297	335	376	419	464
14		10	20	31	45	61	80	101	125	151	180	211	245	281	320	361	405	451	500
15	1	11	21	33	48	66	86	108	134	162	193	226	263	301	343	387	434	483	536
16	3	13	23	36	51	70	91	116	143	173	206	241	280	321	366	413	463	516	571
17	4	14	24	38	55	74	97	123	152	184	219	257	298	342	389	439	492	548	607
18	4	14	26	40	58	79	103	130	161	194	231	272	315	362	411	464	521	580	643
19	5	15	27	42	61	83	109	137	170	205	244	287	333	382	434	490	550	612	679
20	6	16	29	45	64	88	114	145	179	216	257	302	350	402	457	516	579	645	714
21	7	17	30	47	68	92	120	152	188	227	270	317	368	422	480	542	608	677	750
22	7	18	31	49	71	96	126	159	196	238	283	332	385	442	503	568	636	709	786
23	8	18	33	51	74	101	131	166	205	248	296	347	403	462	526	593	665	741	821
24	8	19	34	54	77	105	137	174	214	259	309	362	420	482	549	619	694	774	857
25	9	20	36	56	80	109	143	181	223	270	321	377	438	502	571	645	723	806	893

————— Represents S=L

Minimum Length of Vertical Curve = 1 m

Exhibit 5.8. Minimum Length of Crest Vertical Curve Based on Stopping Sight Distance (Continued)

Other path users such as child bicyclists, hand bicyclists, recumbent bicyclists, and others have lower eye heights than a typical adult bicyclist. Eye heights are approximately 2.6 feet (0.85 m) for hand cyclists and 3.9 feet (1.2 m) for recumbent bicyclists. When compared to the eye heights of typical bicyclists, these lower eye heights limit sight distance over crest vertical curves. However, since most hand bicyclists and child bicyclists travel slower than typical adult bicyclists, their needs are met by using the values in Exhibit 5.8. Recumbent bicyclists generally travel faster than typical upright bicyclists, so if they are expected to make up a relatively large percentage of path users, crest vertical curve lengths should be increased accordingly (operating characteristics of recumbent bicyclists are found in Chapter 3).

Exhibit 5.9 and 5.10, and Equation 5-5 below indicate the minimum clearance that should be used for line-of-sight obstructions for horizontal curves. The lateral clearance (horizontal sight line offset or HSO)

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is obtained by using the table in Exhibit 5.10 with the stopping sight distance (Exhibit 5.7) and the proposed horizontal radius of curvature.

Path users typically travel side-by-side on shared use paths. On narrow paths, bicyclists have a tendency to ride near the middle of the path. For these reasons, and because of the higher potential for crashes on curves, lateral clearances on horizontal curves should be calculated based on the sum of the stopping sight distances for path users traveling in opposite directions around the curve. Where this is not possible or feasible, consideration should be given to widening the path through the curve, installing a yellow center line stripe, installing turn or curve warning signs (W1 series) in accordance with the MUTCD (3), or a combination of these alternatives. See Section 5.4.1. and 5.4.2. of this chapter for more information about center line pavement markings and signs.

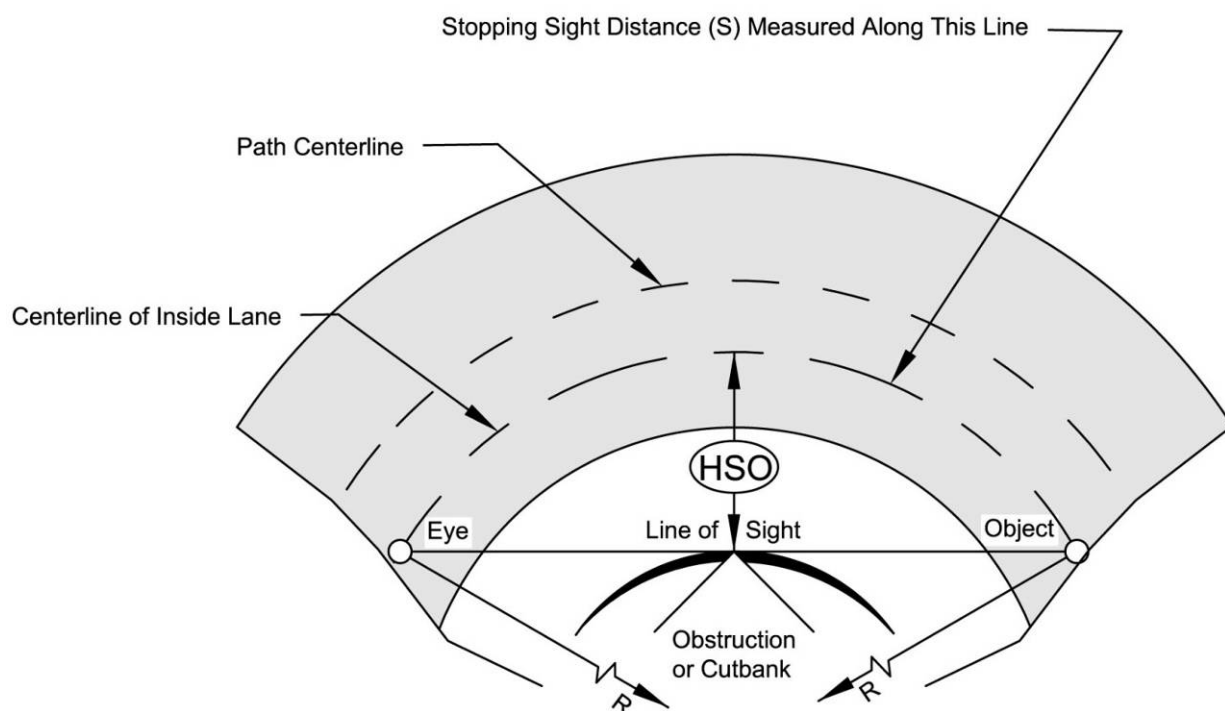


Exhibit 5.9. Diagram Illustrating Components for Determining Horizontal Sight Distance

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US Customary	Metric
$HSO = R \left[1 - \cos \left(\frac{28.65S}{R} \right) \right]$ $S = \frac{R}{28.65} \left[\cos^{-1} \left(\frac{R - HSO}{R} \right) \right]$	$HSO = R \left[1 - \cos \left(\frac{28.65S}{R} \right) \right]$ $S = \frac{R}{28.65} \left[\cos^{-1} \left(\frac{R - HSO}{R} \right) \right]$
<p>where:</p> <p>S = stopping sight distance (ft)</p> <p>R = radius of centerline of lane (ft)</p> <p>HSO = horizontal sightline offset, distance from centerline of lane to obstruction (ft)</p> <p>Note: -angle is expressed in degrees -line of sight is 2.3 ft above centerline of inside lane at point of obstruction</p>	<p>where:</p> <p>S = stopping sight distance (m)</p> <p>R = radius of centerline of lane (m)</p> <p>HSO = horizontal sightline offset, distance from centerline of lane to obstruction (m)</p> <p>Note: -angle is expressed in degrees -line of sight is 0.7 m above centerline of inside lane at point of obstruction</p>

Equation 5-5. Horizontal Sight Distance

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R (ft)	US Customary															
	S= Stopping Sight Distance (ft)															
25	2.0	7.6	15.9													
50	1.0	3.9	8.7	15.2	23.0	31.9	41.5									
75	0.7	2.7	5.9	10.4	16.1	22.8	30.4	38.8	47.8	57.4	67.2					
95	0.5	2.1	4.7	8.3	12.9	18.3	24.7	31.8	39.5	48.0	56.9	66.3	75.9	85.8		
125	0.4	1.6	3.6	6.3	9.9	14.1	19.1	24.7	31.0	37.9	45.4	53.3	61.7	70.6	79.7	
155	0.3	1.3	2.9	5.1	8.0	11.5	15.5	20.2	25.4	31.2	37.4	44.2	51.4	59.1	67.1	
175	0.3	1.1	2.6	4.6	7.1	10.2	13.8	18.0	22.6	27.8	33.5	39.6	46.1	53.1	60.5	
200	0.3	1.0	2.2	4.0	6.2	8.9	12.1	15.8	19.9	24.5	29.5	34.9	40.8	47.0	53.7	
225	0.2	0.9	2.0	3.5	5.5	8.0	10.8	14.1	17.8	21.9	26.4	31.3	36.5	42.2	48.2	
250	0.2	0.8	1.8	3.2	5.0	7.2	9.7	12.7	16.0	19.7	23.8	28.3	33.1	38.2	43.7	
275	0.2	0.7	1.6	2.9	4.5	6.5	8.9	11.6	14.6	18.0	21.7	25.8	30.2	34.9	39.9	
300	0.2	0.7	1.5	2.7	4.2	6.0	8.1	10.6	13.4	16.5	19.9	23.7	27.7	32.1	36.7	
350	0.1	0.6	1.3	2.3	3.6	5.1	7.0	9.1	11.5	14.2	17.1	20.4	23.9	27.6	31.7	
390	0.1	0.5	1.2	2.1	3.2	4.6	6.3	8.2	10.3	12.8	15.4	18.3	21.5	24.9	28.5	
500	0.1	0.4	0.9	1.6	2.5	3.6	4.9	6.4	8.1	10.0	12.1	14.3	16.8	19.5	22.3	
565		0.4	0.8	1.4	2.2	3.2	4.3	5.7	7.2	8.8	10.7	12.7	14.9	17.3	19.8	
600		0.3	0.8	1.3	2.1	3.0	4.1	5.3	6.7	8.3	10.1	12.0	14.0	16.3	18.7	
700		0.3	0.6	1.1	1.8	2.6	3.5	4.6	5.8	7.1	8.6	10.3	12.0	14.0	16.0	
800		0.3	0.6	1.0	1.6	2.2	3.1	4.0	5.1	6.2	7.6	9.0	10.5	12.2	14.0	
900		0.2	0.5	0.9	1.4	2.0	2.7	3.6	4.5	5.6	6.7	8.0	9.4	10.9	12.5	
1000		0.2	0.5	0.8	1.3	1.8	2.4	3.2	4.0	5.0	6.0	7.2	8.4	9.8	11.2	

R (m)	Metric																		
	S= Stopping Sight Distance (m)																		
10	1.2	2.7	4.6	6.8	9.3														
15	0.8	1.8	3.2	4.9	6.9	9.1	11.0	14.0											
20	0.6	1.4	2.4	3.8	5.4	7.2	9.2	11.0	14.0	16.0	19.0								
25	0.5	1.1	2.0	3.1	4.4	5.9	7.6	9.5	11.0	14.0	16.0	18.0	21.0	23.0					
50	0.3	0.6	1.0	1.6	2.2	3.0	3.9	5.0	6.1	7.4	8.7	10.0	12.0	13.0	15.0	17.0	19.0	21.0	23.0
75	0.2	0.4	0.7	1.0	1.5	2.0	2.7	3.4	4.1	5.0	5.9	6.9	8.0	9.2	10.0	12.0	13.0	15.0	16.0
100	0.1	0.3	0.5	0.8	1.1	1.5	2.0	2.5	3.1	3.8	4.5	5.2	6.1	7.0	7.9	8.9	10.0	11.0	12.0
125	0.1	0.2	0.4	0.6	0.9	1.2	1.6	2.0	2.5	3.0	3.6	4.2	4.9	5.6	6.3	7.2	8.0	8.9	9.9
150		0.2	0.3	0.5	0.7	1.0	1.3	1.7	2.1	2.5	3.0	3.5	4.1	4.7	5.3	6.0	6.7	7.5	8.3
175		0.2	0.3	0.4	0.6	0.9	1.1	1.4	1.8	2.2	2.6	3.0	3.5	4.0	4.6	5.1	5.8	6.4	7.1
200		0.1	0.3	0.4	0.6	0.8	1.0	1.3	1.6	1.9	2.2	2.6	3.1	3.5	4.0	4.5	5.0	5.6	6.2
225		0.1	0.2	0.3	0.5	0.7	0.9	1.1	1.4	1.7	2.0	2.3	2.7	3.1	3.5	4.0	4.5	5.0	5.5
250		0.1	0.2	0.3	0.5	0.6	0.8	1.0	1.2	1.5	1.8	2.1	2.4	2.8	3.2	3.6	4.0	4.5	5.0
275		0.1	0.2	0.3	0.4	0.6	0.7	0.9	1.1	1.4	1.6	1.9	2.2	2.6	2.9	3.3	3.7	4.1	4.5
300			0.2	0.3	0.4	0.5	0.7	0.8	1.0	1.3	1.5	1.8	2.0	2.3	2.7	3.0	3.4	3.8	4.2

Exhibit 5.10. Minimum Lateral Clearance (Horizontal Sightline Offset or HSO) for Horizontal Curves

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Hard, all-weather pavement surfaces are generally preferred over those of crushed aggregate, sand, clay, or stabilized earth, since unpaved surfaces provide a lower level of service and require more maintenance. On unpaved surfaces, bicyclists and other wheeled users must use a greater effort to travel at a given speed when compared to a paved surface. Some users, such as rollerbladers, are unable to use unpaved paths. In areas that experience frequent or even occasional flooding or drainage problems, or in areas of moderate or steep terrain, unpaved surfaces will often erode and are not recommended. Additionally, unpaved paths are difficult to plow for use during the winter.

Unpaved surfaces may be appropriate on rural paths, where the intended use of the path is primarily recreational, or as a temporary measure to open a path before funding is available for paving. Unpaved pathways should be constructed of materials that are firm and stable. Possible surfaces for unpaved paths include crushed stone, stabilized earth, and limestone screenings, depending upon local availability.

Asphalt or Portland cement concrete provide good quality, all-weather pavement structures. Advantages of Portland cement concrete include longer service life, reduced susceptibility to cracking and deformation from roots and weeds, and a more consistent riding surface after years of use and exposure to the elements. A disadvantage of Portland cement concrete is that the seams can degrade the experience of using the path for some wheeled users. In addition, pavement markings (such as centerlines) are not as visible on concrete; they are more visible on asphalt, particularly at night.

Advantages of asphalt include a smooth rolled surface when new, and lower construction costs than with concrete. Asphalt surfaces are softer and are therefore preferred by joggers over concrete. However, asphalt pavement is less durable (typical life expectancy is 15-20 years) and requires more interim maintenance.

Because of wide variations in soils, loads, materials, and construction practices, and varying costs of pavement materials, it is not practical to recommend typical structural sections that will be applicable nationwide. However, the total pavement depth should typically be a minimum of 6 inches, inclusive of the surface course (asphalt or Portland cement concrete) and the base course (typically an aggregate rock base). Any pavement section should be placed over a compacted subgrade.

Designing and selecting pavement sections for shared use paths is similar to designing and selecting highway pavement sections. A soils investigation should be conducted to determine the load-carrying capabilities of the native soil, or former railroad bed (if ballast has been removed), and the need for any special treatments. A soils investigation should also be conducted to determine whether subsurface drainage may be applicable. In colder climates, the effects of freeze-thaw cycles should be anticipated. Geotextiles and other similar materials should be considered where subsurface conditions warrant, such as in locations with swelling clay subgrade.

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Experience in roadway pavement design, together with sound engineering judgment, can assist in the selection and design of a proper path pavement structure and may identify energy-conserving practices, such as the use of sulfur-extended asphalt, asphalt emulsions, porous pavement, and recycled asphalt.

While loads on shared use paths will be substantially less than roadways, paths should be designed to sustain wheel loads of occasional emergency, patrol, maintenance and other motor vehicles that are expected to use or cross the path. When motor vehicles are driven on shared use paths, their wheels often will be at, or very near, the edges of the path. This can cause edge damage that, in turn, will reduce the effective operating width of the path. The path should therefore be constructed of sufficient width to accommodate the vehicles, and adequate edge support should be provided. Edge support can be provided by means of stabilized shoulders, flush or raised concrete curbing, or additional pavement width or thickness. The use of flush concrete curbing has other long-term maintenance benefits, such as reducing the potential for encroachment of vegetation onto the path surface. If raised curbs are used, additional path width should be provided, as users will shy away from the curb, resulting in a narrower effective path width.

It is important to construct and maintain a smooth riding surface on shared use paths. Pavements should be machine laid; soil sterilizers should be used where necessary to prevent vegetation from erupting through the pavement. On Portland cement concrete pavements, the transverse joints, necessary to control cracking, should be saw cut, rather than tooled, to provide a smoother ride. On the other hand, skid resistance qualities should not be sacrificed for the sake of smoothness. Broom finish or burlap drag concrete surfaces are preferred.

Utility covers and bicycle-safe drainage grates should be flush with the surface of the pavement on all sides. Railroad crossings should be smooth and be designed at an angle between 60 and 90 degrees to the direction of travel in order to minimize the danger of falls. Refer to Chapter 4 for design treatments that can be used to improve railroad crossings.

Where a shared use path crosses an unpaved road or driveway, the road or driveway should be paved a minimum of 20 feet (6 m) on each side of the crossing to reduce the amount of gravel scattered onto or along the path by motor vehicles. The pavement structure at the crossing should be adequate to sustain the expected loading at that location.

5.2.10. BRIDGES AND UNDERPASSES

A bridge or underpass may be necessary to provide continuity to a shared use path. The "receiving" clear width on the end of a bridge (from inside of rail or barrier to inside of opposite rail or barrier) should allow 2 feet (0.6 m) of clearance on each side of the pathway, as recommended in Section 5.2.1., but under constrained conditions may taper to the pathway width.

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1 Carrying the clear areas across the structures has two advantages. First, the clear width provides a
2 minimum horizontal shy distance from the railing or barrier, and second, it provides needed
3 maneuvering space to avoid conflicts with pedestrians or bicyclists who have stopped on the bridge
4 (e.g., to admire the view).

5 Access by emergency, patrol, and maintenance vehicles should be considered in establishing design
6 clearances of structures on shared use paths. Similarly, vertical clearance may be dictated by occasional
7 authorized motor vehicles using the path. Where practical, a minimum vertical clearance of 10 feet (3.0
8 m) is desirable for adequate vertical shy distance.

9 Where grade separation is desired between a path and a roadway or railroad, designers sometimes have
10 the choice between constructing a bridge over the roadway or railroad, and constructing a tunnel or
11 underpass under the roadway or railroad. The adjacent topography typically is the greatest factor in
12 determining which option is best, however all else being equal, bridges are preferred to underpasses
13 because they have security advantages and are less likely to have drainage problems.

14 When a bridge or underpass is built over a public right-of-way (such as a road), a connection is often
15 needed between the path and roadway, as this represents a potential access point for pedestrians and
16 bicyclists. This often requires significant ramping or other means to ensure an accessible connection
17 between the two.

18 Protective railings, fences, or barriers on either side of a shared use path on a stand-alone structure
19 should be a minimum of 42 inches (1 m) high. There are some locations where a 48-inch (1.2 m) high
20 railing should be considered in order to prevent bicyclists from falling over the railing during a crash.
21 This includes bridges or bridge approaches where high-speed, steep-angle (25 degrees or greater)
22 impacts between a bicyclist and the railing may occur, such as at a curve at the foot of a long,
23 descending grade where the curve radius is less than that appropriate for the design speed or
24 anticipated speed.

25 Openings between horizontal or vertical members on railings should be small enough that a 6-inch (150
26 mm) sphere cannot pass through them in the lower 27 inches (0.7 m). For the portion of railing that is
27 higher than 27 inches (0.7 m), openings may be spaced such that an 8-inch (200 mm) sphere cannot pass
28 through them. This is done to prevent children from falling through the openings. Where a bicyclist's
29 handlebar may come into contact with a railing or barrier, a smooth, wide rub-rail may be installed at a
30 height of about 36 inches (0.9 m) to 44 inches (1.1 m), to reduce the likelihood that a bicyclist's
31 handlebar will be caught by the railing (see Exhibit 5.11).

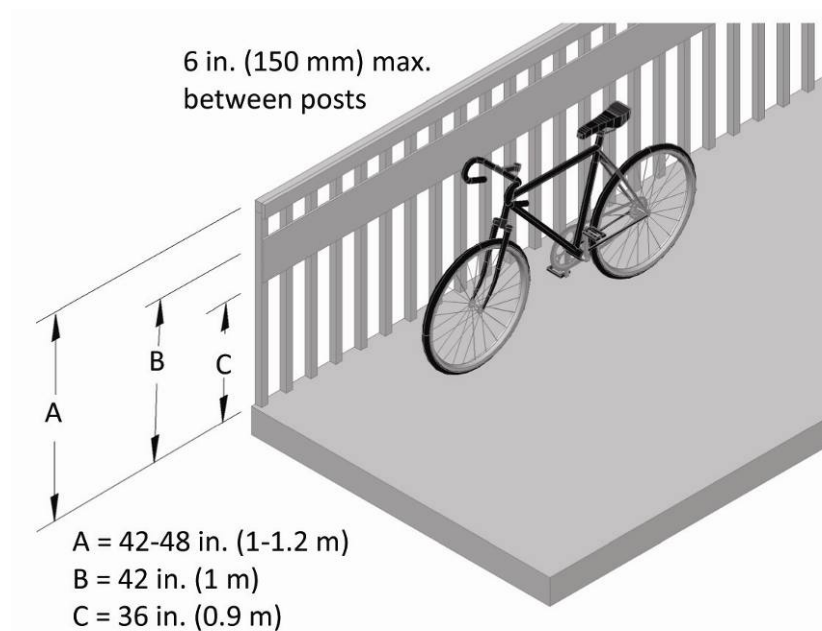
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Exhibit 5.11. Example Bridge Railing

Bridges should be designed for pedestrian live loadings. Where maintenance and emergency vehicles may be expected to cross the bridge, the design should accommodate them. On all bridge decks, special care should be taken to ensure that bicycle-safe expansion joints are used, and that decking materials are not slippery when wet.

There are often opportunities to retrofit path structures to existing highway or railroad bridges. Using an existing bridge can result in significant cost savings and provide path continuity over large rivers and other obstacles. These retrofits can be accomplished in several ways, including cantilevering the path onto an existing bridge, or by placing the path within the substructure of the existing bridge as shown in Exhibit 5.12.

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Exhibit 5.12. Example of Bridge Structures (photo by Toole Design Group)

In many situations, there is a desire to retrofit a path under a bridge along a river or waterway to provide a grade-separated crossing of a major road or railroad. Special treatments may be necessary in these circumstances. These paths are often located within a floodplain, so path pavement and subgrade treatments may need to be enhanced. In extreme cases, paths can be built below the normal water level, requiring that the water be retained and a pumping system be provided for the path.

5.2.11. DRAINAGE

The minimum recommended pavement cross slope of 1 percent usually provides adequate drainage. Sloping in one direction instead of crowning is preferred and usually simplifies drainage and surface construction. An even surface is essential to prevent water ponding and ice formation. On unpaved shared use paths, particular attention should be paid to drainage to avoid erosion.

Depending on site conditions, typically paths with cross slope in the direction of the existing terrain will provide sheet flow of surface runoff and avoid the need for channelizing flow in ditches, cross culverts, and closed pipe systems. However, where a shared use path is constructed on the side of a slope that has considerable runoff, or other conditions that result in relatively high runoff, a ditch of suitable dimensions should be placed on the uphill side to intercept the slope's drainage. Such ditches should be designed so that no undue obstacle or hazard is presented to errant bicyclists. Where necessary, catch basins with drains should be provided to carry the intercepted water under the path. Bicycle-safe drainage grates and manhole covers should be located outside the clearance area of the pathway.

Paths that are located in low-lying areas may require attention to other drainage issues in the vicinity that have not been previously addressed to ensure that the path drains properly, and that retention areas located away from the pathway are provided.

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To prevent erosion in the area adjacent to the shared use path, consideration should be given to preserving a hardy, natural ground cover. In addition, pathway design should meet applicable storm water management regulations. In an effort to improve water quality and manage the quantity of runoff, low-impact development techniques such as bio-retention swales should be considered. Other erosion and sediment control measures should be employed as necessary, including seeding, mulching, and sodding of adjacent slopes, swales, and other erodible areas.

5.2.12. LIGHTING

Fixed-source lighting can improve visibility along paths and at intersections at night or under other dark conditions. Lighting can also greatly improve riders' ability to detect surface hazards under such conditions, even when their bicycles are properly equipped with headlamps. Provision of lighting should be considered where nighttime usage is anticipated, such as on paths that provide convenient connections to transit stops and stations, schools, universities, shopping, and employment areas.

Where nighttime use is permitted, pathway lighting is necessary at path-roadway intersections. If nighttime use is prohibited, lighting at crosswalks may still be necessary if the pathway connects to existing sidewalks, because the crossing is in the public right-of-way and may be used at night even if the pathway is not. Lighting should also be considered in locations where personal security is an issue.

Pedestrian-scale lighting is preferred to tall, highway-style lamps. Pedestrian-scale lighting is characterized by shorter light poles (standards about 15 ft [4.6 m] high), lower levels of illumination (except at crossings), closer spacing of standards (to avoid dark zones between luminaires), and high pressure sodium vapor or metal halide lamps. Metal halide lamps produce better color rendition ("white light") than sodium vapor lamps and can facilitate user recognition in areas with high volumes of night use. Depending on the location, average maintained horizontal illumination levels of 0.5 to 2 foot-candles (5 to 22 lux) should be considered. For personal safety, higher levels may be needed in some locations.

Placement of light poles should provide the recommended horizontal and vertical clearances from the pathway. Light fixtures should be chosen to reduce the loss of light and may need to comply with local "dark sky" guidelines and regulations. The use of solar-powered lighting can be considered; however care should be taken to ensure it provides adequate light. Solar-powered lighting is often inadequate in locations with significant tree canopy, or in northern climates where it sometimes fails to provide enough illumination during winter months.

If a pathway is used infrequently at night, lighting can be provided at certain hours only, based on an engineering study of pathway usage, for example up to 11 pm, and starting at 6 am. These conditions should be made known to path users with a sign at path entrances. Where lighting is not provided, or

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only provided during certain hours, reflective edge lines should be provided as described in Section 5.4.1. of this chapter.

Lighting should be provided in pathway tunnels and underpasses. At night, lighting in tunnels is important to provide security. Daytime lighting of tunnels and underpasses is often necessary, and should be designed in a manner similar to the design of lighting in roadway tunnels. This includes brighter lighting during the day than at night, due to the fact that users' eyes cannot make fast adjustments to changing light conditions. On long tunnels it is appropriate to use varying light intensities through the tunnel, with higher levels of illumination near the entrances and lower levels in the middle. Refer to the *Roadway Lighting Design Guide* (8) for more information about designing appropriate lighting in tunnels and underpasses.

5.3. SHARED USE PATH-ROADWAY INTERSECTION DESIGN

The design of intersections between shared use paths and roadways has a significant impact on users' comfort and safety. Intersection design should not only address cross-traffic movements, but should also address turning movements of riders entering and exiting the path. Due to potential conflicts at these junctions, careful design should be used for predictable and orderly operation between shared use path traffic and other traffic.

Regardless of whether a pathway crosses a roadway at an existing intersection between two roadways, or at a new "midblock" location, the principles that apply to general pedestrian safety at crossings (controlled and uncontrolled) are transferable to pathway intersection design. There are a wide range of design features that improve pedestrian and bicyclist safety at such intersections. This guide provides a general overview of crossing measures; other sources, such as the *AASHTO Guide for the Planning, Design and Operation of Pedestrian Facilities* (9), should be consulted for more detail.

Shared use path crossings come in many configurations with many variables: the number of roadway lanes to be crossed, divided or undivided roadways, number of approach legs, the speeds and volumes of traffic, and traffic controls that range from uncontrolled to yield, stop, or signal controlled. Each intersection is unique and requires engineering judgment to determine an appropriate intersection treatment.

Due to the mixed nature of shared use path traffic, the practitioner must keep in mind the speed variability of each travel mode and its resulting effect on design values when considering design treatments for path-roadway intersections. The fastest vehicle should be considered for approach speeds (typically the bicyclist and motor vehicle) as these modes are the most likely to surprise cross traffic at the intersection. By contrast, for departures from a stopped condition, the characteristics of slower path users (typically pedestrians) must be taken into account due to their greater exposure to cross traffic.

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Intersections between pathways and roadways should be designed to be accessible to all users, as stated in Section 5.1.1. of this chapter.

5.3.1. SHARED USE PATH CROSSING TYPES

Shared use path crossings can be broadly categorized as midblock, sidepath, or grade-separated crossings. A crossing is considered midblock if it is located outside of the functional area of any adjacent intersection. In some respects, a midblock shared use path crossing can be considered as a four-leg intersection.

A sidepath crossing occurs within the functional area of an intersection of two or more roadways (see Exhibit 5.13). Sidepath crossings are typically parallel to at least one roadway. Sidepath intersections have unique operational challenges that are similar to those of parallel frontage roadways. Section 5.2.2 covers these operational issues in detail, and provides guidelines for locations where sidepaths may not be appropriate.

In some locations, roadway or path traffic conditions may warrant consideration of a grade-separated crossing consisting of either a bridge over the roadway or an underpass beneath the roadway. An analysis should be made to assess the demand for and viability of a grade-separated crossing. See Section 5.2.10. and the discussion of grade-separated crossings in the *AASHTO Guide for the Development of Pedestrian Facilities*. (9)

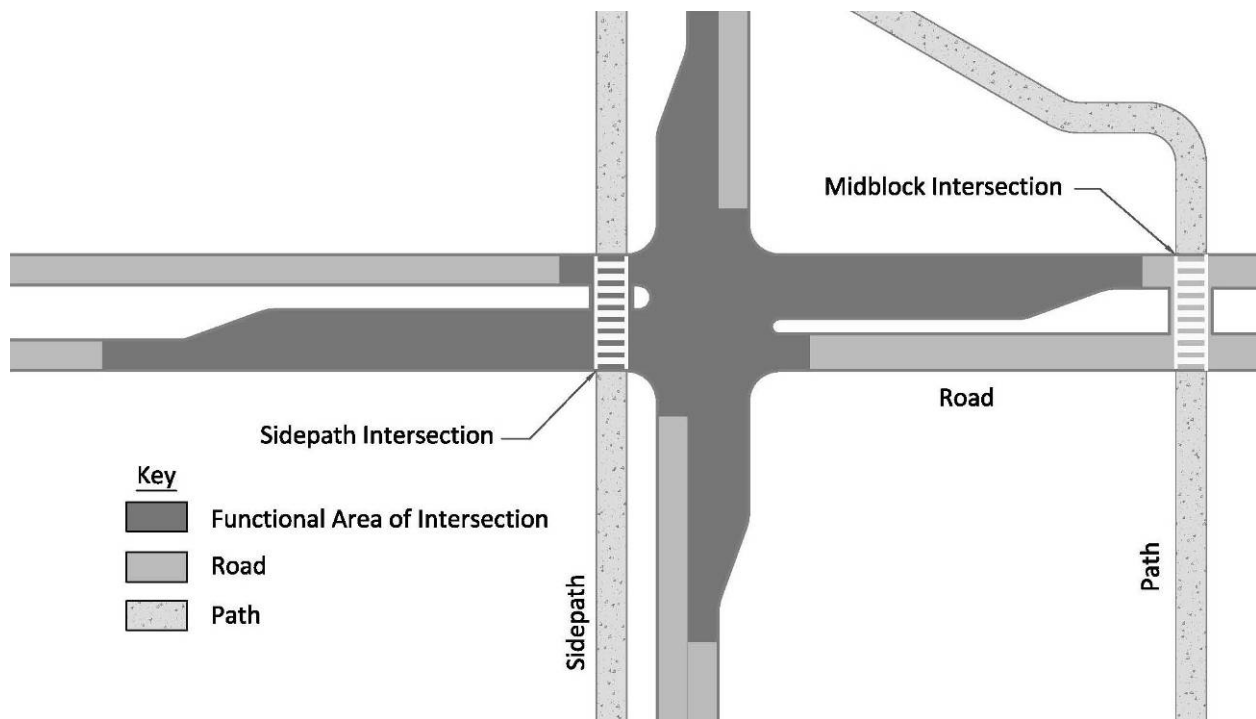
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Exhibit 5.13. Midblock and Sidepath Crossings Relative to Intersection Functional Area

5.3.2. DESIGN OF MIDBLOCK CROSSINGS

The task of designing a midblock crossing between a pathway and a roadway involves a number of variables, including anticipated mix and volume of path users, the speed and volume of motor vehicle traffic on the roadway being crossed, the configuration of the road, the amount of sight distance that can be achieved at the crossing location, and other factors. Geometric design features and traffic controls must be used in combination to achieve safe and efficient accommodations for all users.

GEOMETRIC DESIGN ISSUES AT CROSSINGS

The design approach for the intersection of a shared use path with a roadway is similar to the design approach used for the intersection of two roadways in the following ways:

- The intersection should be conspicuous to both road users and path users.
- Sight lines should be maintained to meet the requirements of the traffic control provided.
- Intersections and approaches should be on relatively flat grades.
- Intersections should be as close to a right angle as possible, given the existing conditions.

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- The least traffic control that is effective should be selected.

It is preferable for midblock path crossings to intersect the roadway at an angle as close to perpendicular as practical, so as to minimize the exposure of crossing path users and maximize sight lines. A crossing skewed at 30 degrees is twice as long as a perpendicular crossing, doubling the exposure of path users to approaching motor vehicles, and increasing delays for motorists who must wait for path users to cross. Retrofitting skewed path crossings can reduce the roadway exposure for path users. Exhibit 5.14 depicts a path realignment to achieve a 90-degree crossing. A minimum 60-degree crossing angle may be acceptable to minimize right-of-way requirements. (2)

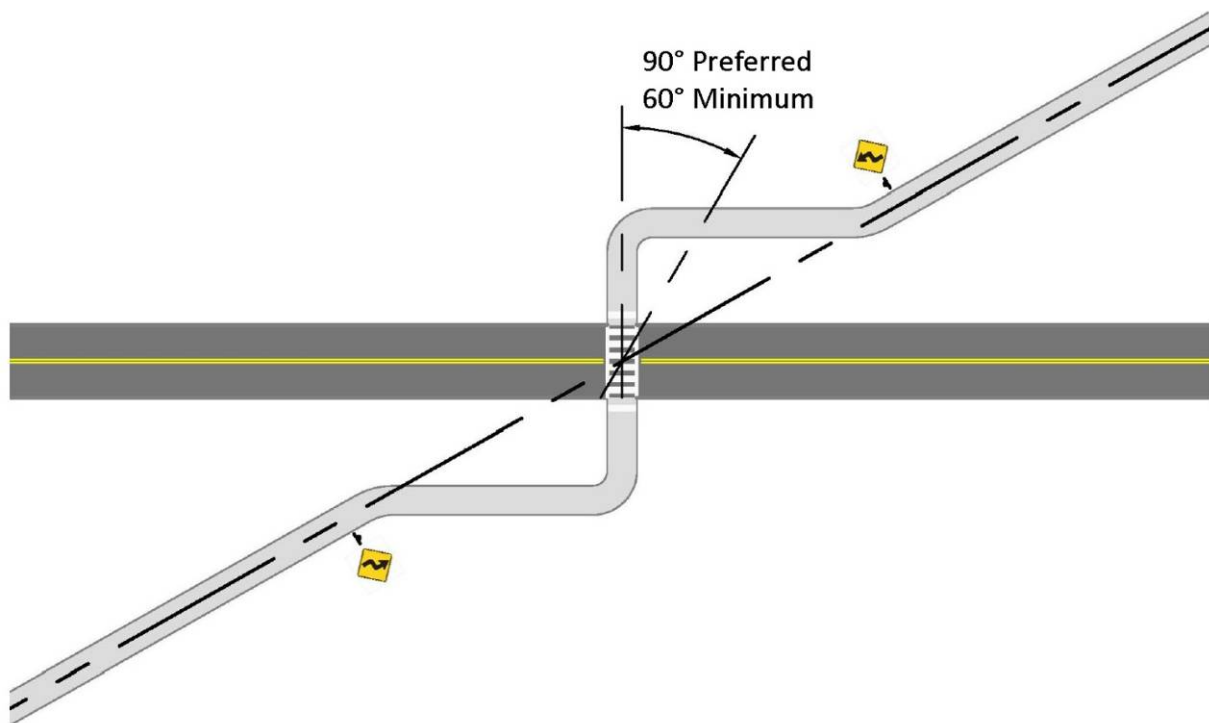


Exhibit 5.14. Crossing Angle for Midblock Path

SPECIAL ISSUES WITH ASSIGNMENT OF RIGHT OF WAY

Shared use paths are unique in terms of the assignment of the right of way, due to the legal responsibility of drivers to yield to (or stop for) pedestrians in crosswalks. Most state codes also stipulate that a pedestrian (and by presumed extension in many states, a cyclist entering a path crosswalk) may not suddenly leave any curb (or place of safety) and walk or run into the path of a vehicle that is so close that it is impossible for the driver to yield. The result is a mutual yielding responsibility among motor vehicle drivers and path users, depending upon the timing of their arrival at an intersection. The speed differential between bicyclists and pedestrians on the pathway must also be taken into account. Bicyclists approach the intersection at a far greater speed than pedestrians, and they

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1 desire to maintain their speed as much as possible. The result is a need to remind bicyclists of their
2 responsibility to yield or stop, while not confusing the issue of who has the legal right of way at midblock
3 crossings.

4 Given these complexities, the most prudent approach when determining the appropriate safety and
5 control measures at midblock pathway intersections is to first determine what measures are needed for
6 pedestrian safety and access (as described below), as it may be determined through this process that a
7 pedestrian signal or beacon is needed. If a signal or a beacon is not needed, the next step is to
8 determine clear sight triangles on the major and minor approaches, so as to evaluate applicability of
9 yield control on the minor approach. Engineering judgment should be applied.

10 DETERMINING APPROPRIATE CROSSING MEASURES

11 Pedestrians amount to a substantial share of users on most paths and experience the greatest amount
12 of exposure at intersections. Uncontrolled pathway crossings designed for pedestrian safety will result
13 in better safety for all users.

14 High-visibility marked crosswalks are recommended at all uncontrolled path-roadway intersections. On
15 roadways with low traffic volumes and speeds where sight distances are adequate, the marked
16 crosswalk should be sufficient to address pedestrian safety. However, additional crossing measures
17 (such as reducing traffic speeds, shortening crossing distance, enhancing driver awareness of the
18 crossing, and/or providing active warning of crosswalk user presence) are needed at uncontrolled
19 locations where the speed limit exceeds 40 miles per hour and either:

- 20 • The roadway has four or more lanes of travel without a raised crossing island and an ADT of
21 12,000 vehicles per day or greater; or
- 22 • The roadway has four or more lanes of travel with a raised crossing island (either existing or
23 planned) and an ADT of 15,000 vehicles per day or greater. (10)

24 DETERMINING PRIORITY ASSIGNMENT

25 In conventional roadway intersection design, right of way is assigned to the higher volume and/or higher
26 speed approach. In the case of a path-roadway intersection, user volumes on the path should be
27 considered. While in many cases roadways will have greater volumes, user volumes on popular paths
28 sometimes exceed traffic volumes on minor crossed streets. In such situations, total user delay may be
29 minimized if roadway traffic yields to path traffic, and given cyclists' reluctance to lose momentum, such
30 an operating pattern often develops spontaneously. In such situations, YIELD or STOP control is more
31 appropriately applied on the roadway approaches (given an analysis of speeds, sight distances, etc. as
32 described below).

33 Changes in user volumes over time should also be considered. New shared use paths are often built in
34 segments, resulting in low initial volumes. In that case, assignment of priority to roadway traffic is

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usually appropriate. However, path volumes may increase over time, raising the need to re-examine priority assignment. Traffic flows at path-roadway intersections should be reviewed occasionally to assure that the priority assignment remains appropriate.

ROUTINE USE OF STOP SIGNS

Application of intersection controls (YIELD signs, STOP signs, or traffic signals) should follow the principle of providing the least control that is effective. Installing unwarranted or unrealistically restrictive controls on path approaches in an attempt to “protect” path users can lead to disregard of controls and intersection operating patterns that are routinely different than indicated by the controls. This can increase an unfamiliar user's or driver's risk of collision, and potentially lead to a loss of respect for the control at warranted locations.

A common misconception is that the routine installation of stop control for the pathway is an effective treatment for preventing crashes at path-roadway intersections. Poor bicyclist compliance with STOP signs at path-roadway intersections is well documented. Bicyclists tend to operate as though there are YIELD signs at these locations: they slow down as they approach the intersection, look for oncoming traffic, and proceed with the crossing if it is safe to do so. Yield control (either for vehicular traffic on the roadway or for users on the pathway) can therefore be an effective solution at some midblock crossings, as it encourages caution without being overly restrictive.

EVALUATING SIGHT DISTANCE TO SELECT TYPE OF CONTROL

Intersection sight distance (sight triangles) is a fundamental component in selecting the appropriate control at a midblock path-roadway intersection. As described above, the least restrictive control that is effective should be used. As noted in horizontal sight distance equation (Equation 5-5), the line of sight is considered to be 2.3 feet (0.7 m) above the roadway or path surface.

Roadway approach sight distance and departure sight triangles should be calculated in accordance with procedures detailed in the AASHTO *Policy on Geometric Design of Highways and Streets* (11), as motor vehicles will control the design criteria.

Generally, pathway approach sight distance should be calculated utilizing the fastest typical path user, which in most cases is the adult two-wheeled bicyclist. Under certain conditions it may be necessary or desirable to use a different design user (therefore different approach speed) if they are more prevalent and represent a faster value. Ideally, approach sight triangles provide an unobstructed view of the entire intersection and a sufficient amount of the intersecting facility to anticipate and avoid a potential collision with crossing traffic, regardless of the traffic control. Approaches to uncontrolled and yield-controlled intersections should provide the recommended approach sight triangle, or else a more restrictive control should be considered.

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Approach sight triangles needed for yield control depend on the design speeds of both the path and the roadway. If yield control is to be used for either the roadway approach or the path approach, available sight distance should be adequate for a traveler on either approach to slow, stop, and avoid a traveler on the other approach. The roadway leg of the sight triangle is based on bicyclists' ability to reach and cross the roadway if they don't see a potentially conflicting vehicle approaching on the roadway, and have just passed the point where they can execute a stop without entering the intersection (see Exhibit 5.15 and Equation 5-6). See Equation 5-3 and Exhibit 5.7 for bicyclist stopping sight distance. Similar to the roadway approach, the path leg of the sight triangle is based on motorists' ability to reach and cross the junction if they don't see a potentially conflicting path user approaching, and have passed the point where they can execute a stop without entering the intersection. The length along the path leg of each approach is given by Equation 5-7¹. If this yield sight triangle cannot be provided, a more restrictive control is required.

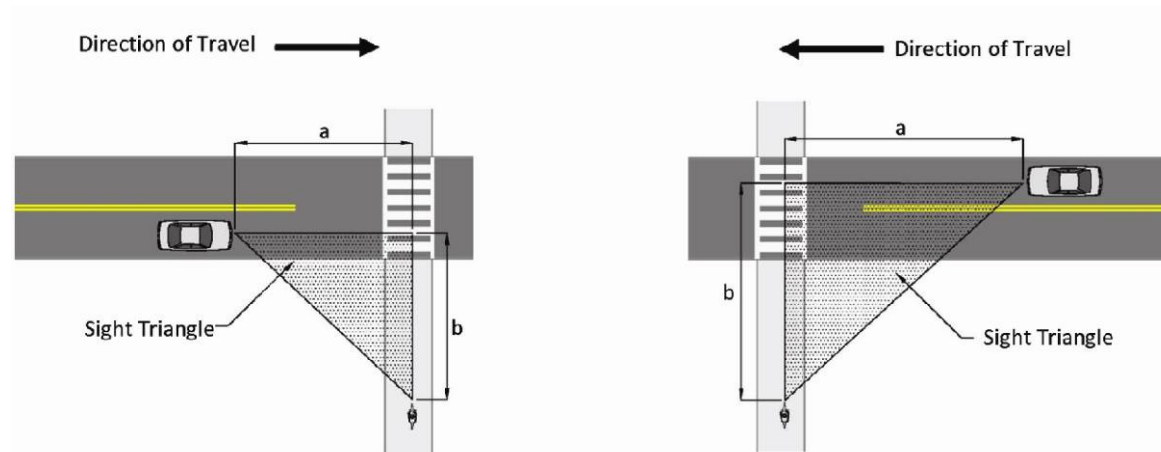


Exhibit 5.15. Yield Sight Triangles

¹ Equation 5-7 accounts for reduced motor vehicle speeds per standard practice in the AASHTO Policy on Geometric Design of Highways and Streets. (10)

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US Customary	Metric
$t_a = \frac{S}{1.47V_{path}}$ $t_g = t_a + \frac{w + L_a}{1.47V_{path}}$ $a = 1.47V_{road}t_g$	$t_a = \frac{S}{0.278V_{path}}$ $t_g = t_a + \frac{w + L_a}{0.278V_{path}}$ $a = 0.278V_{road}t_g$
<p>where:</p> <p>t_g = travel time to reach and clear the road (s)</p> <p>a = length of leg of sight triangle along the roadway approach (ft)</p> <p>t_a = travel time to reach the road from the decision point for a path user that doesn't stop (s)</p> <p>w = width of the intersection to be crossed (ft)</p> <p>L_a = typical bicycle length = 6 ft (see chapter 3 for other design users)</p> <p>V_{path} = design speed of the path (mph)</p> <p>V_{road} = design speed of the road (mph)</p> <p>S = stopping sight distance for the path user traveling at design speed (ft)</p>	<p>where:</p> <p>t_g = travel time to reach and clear the road (s)</p> <p>a = length of leg of sight triangle along the roadway approach (m)</p> <p>t_a = travel time to reach the road from the decision point for a path user that doesn't stop (s)</p> <p>w = width of the intersection to be crossed (m)</p> <p>L_a = typical bicycle length = 1.8 m (see chapter 3 for other design users)</p> <p>V_{path} = design speed of the path (km/h)</p> <p>V_{road} = design speed of the road (km/h)</p> <p>S = stopping sight distance for the path user traveling at design speed (m)</p>

Equation 5-6. Length of Roadway Leg of Sight Triangle

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US Customary	Metric
$t_a = \frac{1.47 V_e - 1.47 V_b}{a_i}$ $t_g = t_a + \frac{w + L_a}{0.88 V_{road}}$ $b = 1.47 V_{path} t_g$	$t_a = \frac{0.278 V_e - 0.278 V_b}{a_i}$ $t_g = t_a + \frac{w + L_a}{0.167 V_{road}}$ $b = 0.278 V_{path} t_g$
<p>where:</p> <p>t_g = travel time to reach and clear the path (s)</p> <p>b = length of leg of sight triangle along the path approach (ft)</p> <p>t_a = travel time to reach the path from the decision point for a motorist that doesn't stop (s). For road approach grades that exceed 3 percent, value should be adjusted in accordance with the AASHTO Green Book (10)</p> <p>V_e = speed at which the motorist would enter the intersection after decelerating (mph) (assumed 0.60 x road design speed)</p> <p>V_b = speed at which braking by the motorist begins (mph) (same as road design speed)</p> <p>a_i = motorist deceleration rate (ft/s²) in intersection approach when braking to a stop is not initiated (assume -5.0 ft/s²)</p> <p>w = width of the intersection to be crossed (ft)</p> <p>L_a = length of the design vehicle (ft)</p> <p>V_{path} = design speed of the path (mph)</p> <p>V_{road} = design speed of the road (mph)</p>	<p>where:</p> <p>t_g = travel time to reach and clear the path (s)</p> <p>b = length of leg of sight triangle along the path approach (m)</p> <p>t_a = travel time to reach the path from the decision point for a motorist that doesn't stop (s). For road approach grades that exceed 3 percent, value should be adjusted in accordance with the AASHTO Green Book (10)</p> <p>V_e = speed at which the motorist would enter the intersection after decelerating (km/h) (assumed 0.60 x road design speed)</p> <p>V_b = speed at which braking by the motorist begins (km/h) (same as road design speed)</p> <p>a_i = motorist deceleration rate (m/s²) in intersection approach when braking to a stop is not initiated (assume -1.5 ft/s²)</p> <p>w = width of the intersection to be crossed (m)</p> <p>L_a = length of the design vehicle (m)</p> <p>V_{path} = design speed of the path (km/h)</p> <p>V_{road} = design speed of the road (km/h)</p>

Equation 5-7. Length of Path Leg of Sight Triangle

Determining sufficient stop- and signal-controlled approach sight distance is simpler than yield-controlled. Regardless of which approach has stop-control or whether the intersection is signal-controlled, the roadway and path approaches to an intersection should always provide enough stopping sight distance to obey the control, and execute a stop before entering the intersection.

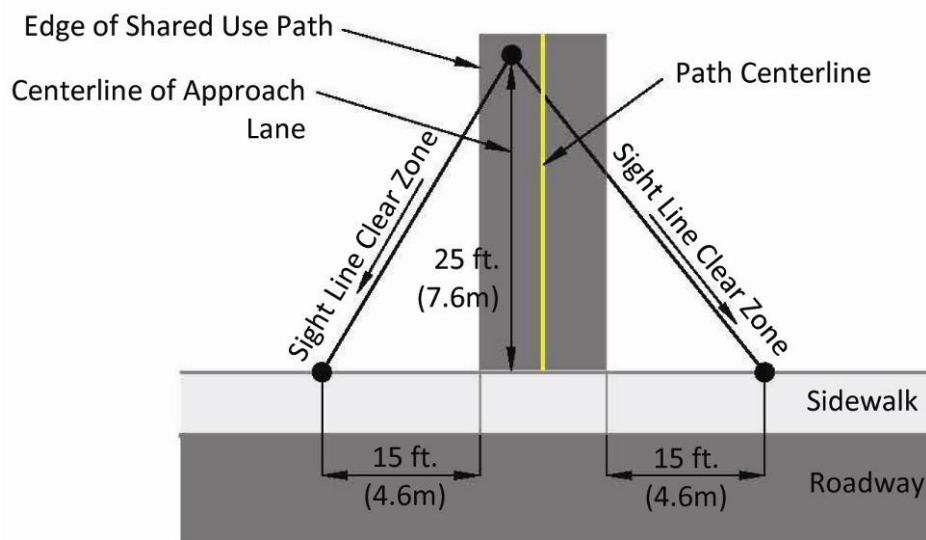
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Departure sight distance for the path should be based on the slowest user who will have the most exposure to crossing traffic. This is typically the pedestrian. However, because path crossings also function as legal crosswalks, a key sight distance consideration is stopping sight distance for the roadway approach to provide adequate distance for the motor vehicle to stop if the path user is either already in the crosswalk, or is just beginning to enter it. Ideally, departure sight distance provides stopped pathway users with enough sight distance of the intersecting roadway to judge adequate gaps in oncoming traffic to cross the road safely. This type of departure sight distance is desirable for yield- and stop-controlled path approaches. Under certain conditions it may be necessary or desirable to use a different design user (and therefore different departure speed) if they are more prevalent and represent a slower value.

Regardless of intersection sight triangle lengths, roadway and path approaches to an intersection should always provide enough stopping sight distance to avoid hazards or potential conflicts within the intersection.

Multi-way (i.e., all-way) stops at path-roadway intersections are not recommended.

At an intersection of a shared use path with a walkway, a clear sight triangle extending at least 15 ft (4.6 m) along the walkway should be provided (see Exhibit 5.16). The clear sight line will enable pedestrians approaching the pathway to see and react to oncoming path traffic to avoid potential conflicts at the path-walkway intersection. If a shared use path intersects another shared use path, sight triangles should be provided similar to a yield condition at a path-roadway intersection. However, both legs of the sight triangle should be based on the stopping sight distance of the paths. Use Equation 5-6 for both legs of the sight triangle.

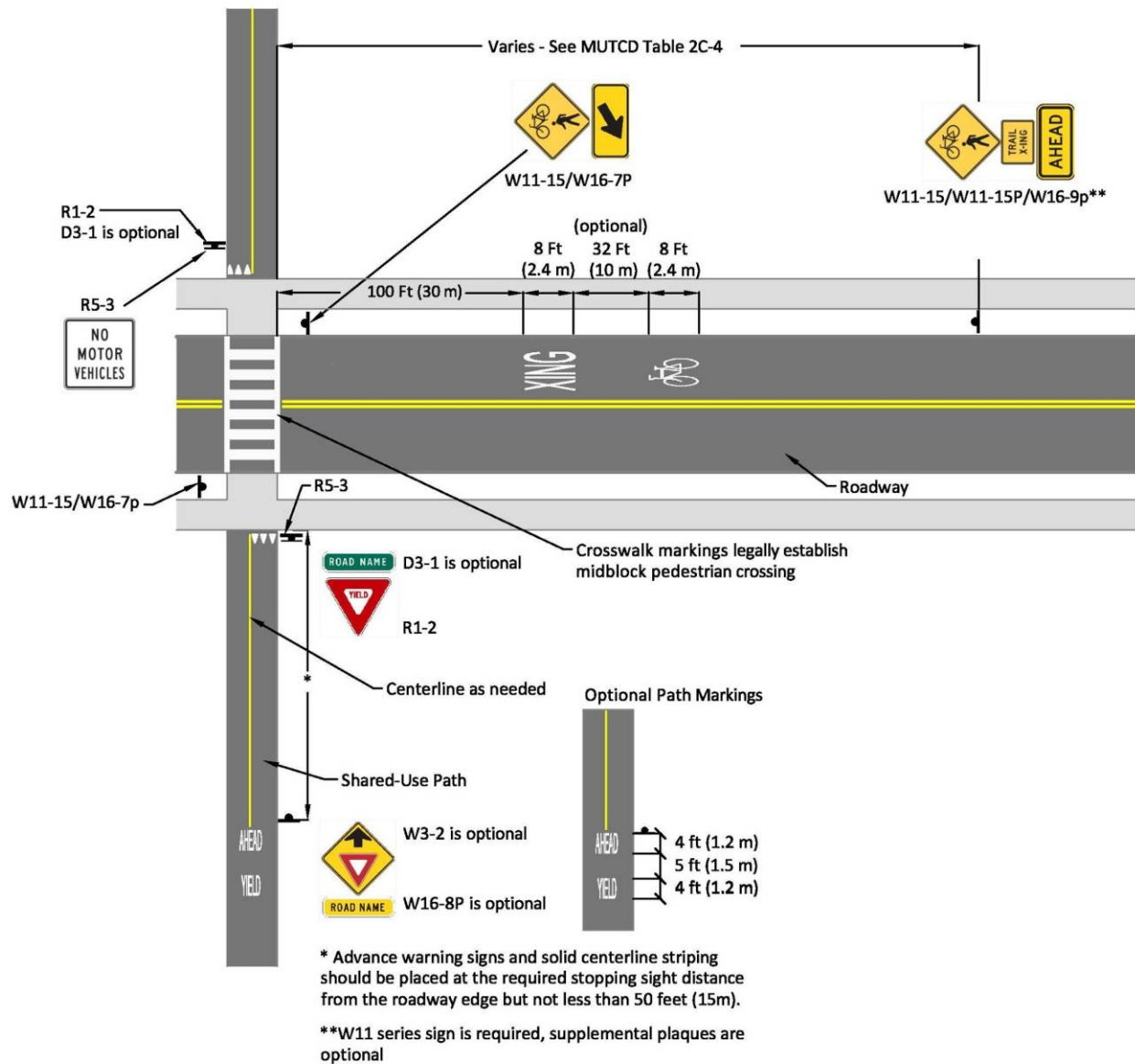
DRAFT FOR AASHTO COMMITTEE REVIEW AND COMMENT**Exhibit 5.16. Minimum Path-Walkway Sight Triangle****MIDBLOCK SIGNALIZED INTERSECTIONS**

If traffic and roadway characteristics make crossing difficult for the path user, the need for a signal or active warning device (such as a beacon) should be considered based on traffic volumes, speed, number of lanes, and availability of a refuge. Guidance on the need for additional crossing measures is provided in FHWA's *Safety Effects of Marked versus Unmarked Crosswalks at Uncontrolled Locations: Final Report and Recommended Guidelines* (10). The use of path user volume to determine the need for a signalized crossing may not be appropriate. In some situations the path user may not have access to another appropriate crossing location.

More information on signals at path-roadway intersections is provided in Section 5.4.3.

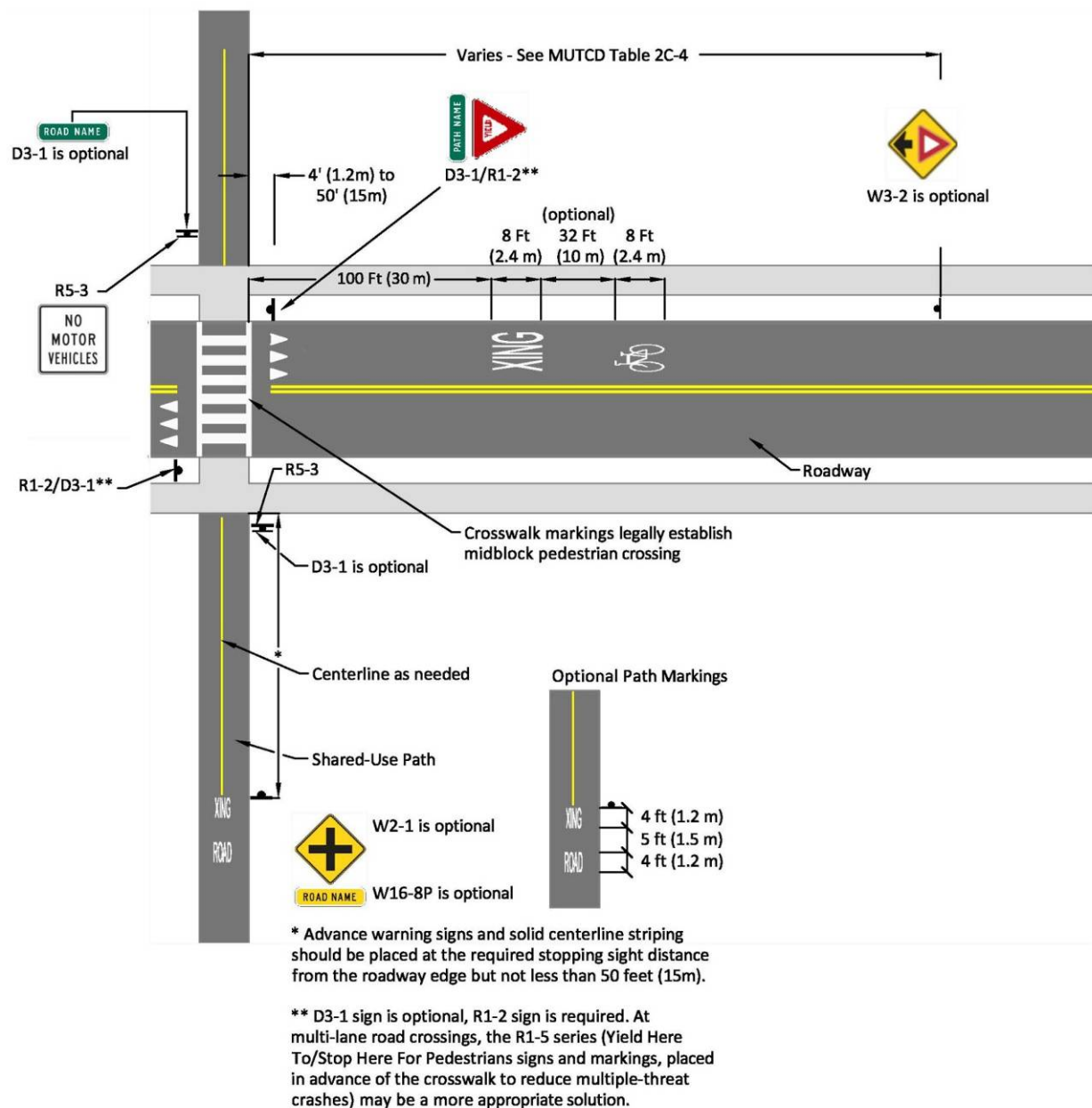
5.3.3. EXAMPLES OF MIDBLOCK INTERSECTION CONTROLS

Exhibits 5.17, 5.18, 5.19 and 5.20 illustrate various examples of midblock control treatments. They show typical pavement marking and sign crossing treatments. These diagrams are illustrative and are not intended to show all signs and markings that may be necessary or advisable, or all types of design treatments that are possible at these locations. Each graphic assumes the appropriate minimum sight distances are provided for the roadway and the path.

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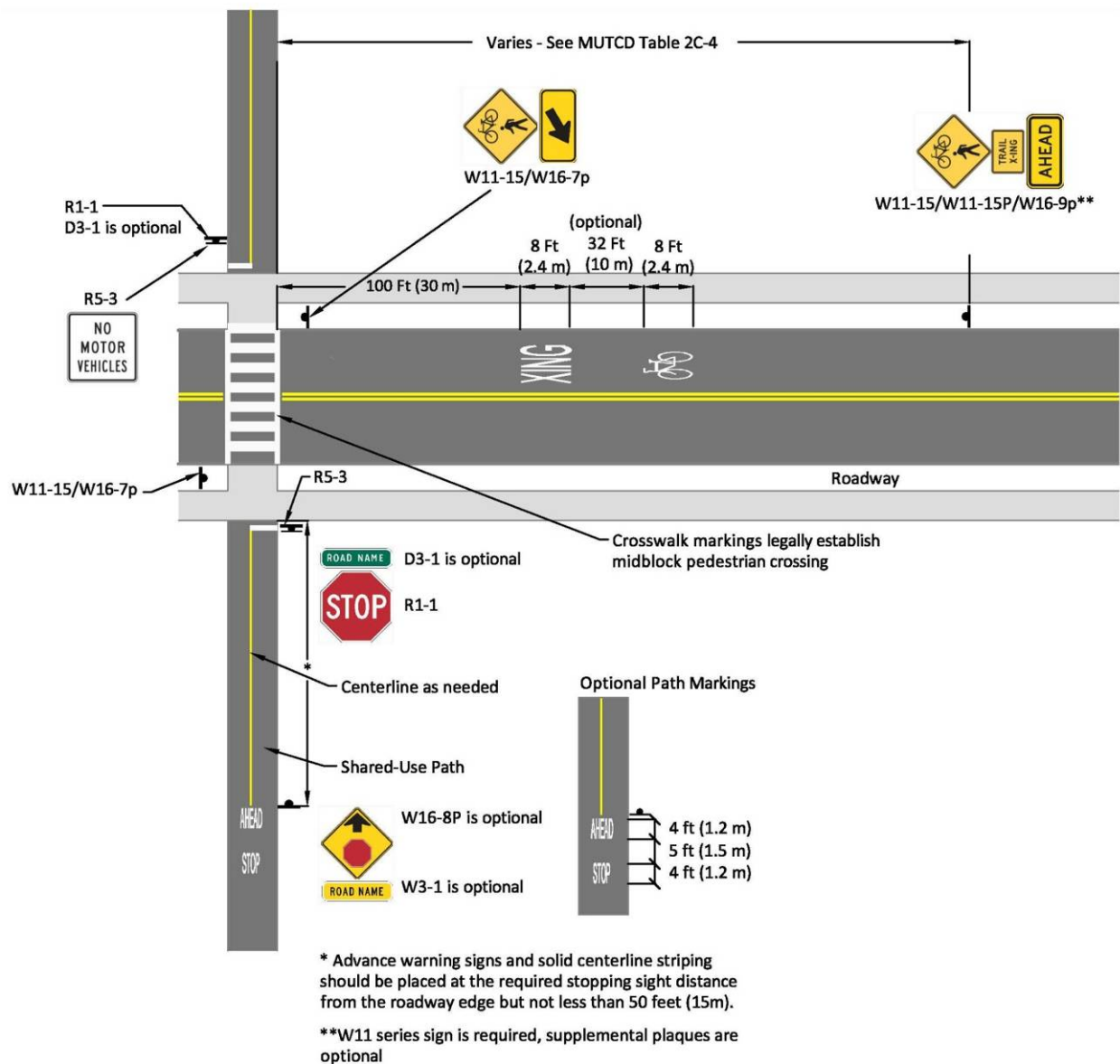
2 **Exhibit 5.17. Example of Midblock Path-Roadway Intersection – Path is YIELD Controlled**

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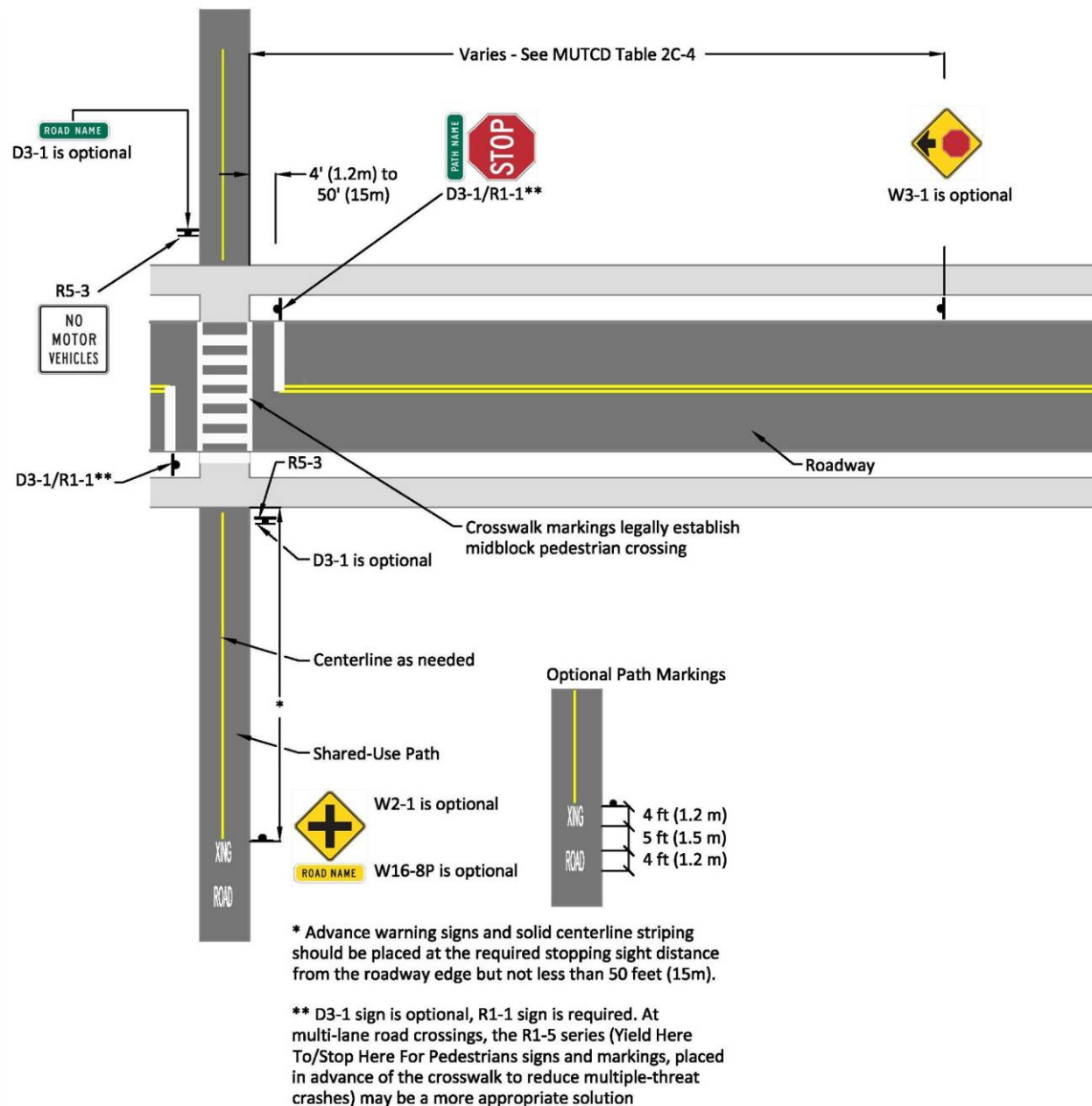
2

Exhibit 5.18. Example Midblock Path-Roadway Intersection – Roadway is YIELD Controlled

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2 **Exhibit 5.19. Example of Midblock Path-Roadway Intersection – Path is STOP Controlled**

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2 **Exhibit 5.20. Example Midblock Path-Roadway Intersection – Roadway is STOP Controlled**

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5.3.4. SIDEPATH INTERSECTION DESIGN CONSIDERATIONS

As described in Section 5.2.2., there are a variety of operational challenges when pathways are provided adjacent to roadways (sidepaths). Depending upon motor vehicle and pathway user speeds, the width and character of the adjacent roadway, the amount of separation between the pathway and the roadway, and the characteristics of conflict points, sidepath travel may involve lesser--or greater--risk of motor vehicle collision for cyclists than roadway travel.

The first and most important step is to objectively assess whether the location is a candidate for a two-way sidepath. Guidance on this issue is given in Section 5.2.2.

Assuming that the location has been determined to be a candidate for a two-way sidepath, pathway width and separation from roadway at intersections and driveways should be determined with respect to roadway speeds and number of lanes. Motorists on multi-lane roadways with higher speeds are more distracted by driving conditions, and are less likely to notice the presence of bicyclists on the sidepath during turning movements. On roads with speed limits of 50 mph or greater, increasing the separation from roadway is recommended to improve path user safety. At lower speeds, greater separation does not improve safety; therefore the sidepath should be located in close proximity to the parallel roadway at intersections, so motorists turning off the roadway can better detect sidepath riders. (12)

In all cases, the key to optimizing user safety at driveways and intersections is to reduce speeds of both path users and motorists at conflict points, increase the predictability of trail and road user behavior, and limit the amount of exposure at these conflict points as much as possible. Design measures to accomplish this are discussed below:

- Reduce the frequency of driveways, their widths, and (where practical) the volume of motor vehicle crossings through access management. For example, combine driveways of adjacent properties, reduce driveway width to the minimum needed to accommodate ingress and egress volumes, limit access by installing channelization devices that only allow right-in/right-out movements, and provide median refuge islands for wide driveways.
- Design intersections to reduce driver speeds and heighten awareness of path users. Strategies include tighter corner radii, avoidance of high speed free-flowing movements (such as ramp-style turns), providing median refuge islands, ensuring adequate sight distances between intersecting users, and other measures to reduce motor vehicle speeds at intersections.
- Design driveways to reduce driver speeds and heighten awareness of path users. Strategies can include tighter corner radii, maintaining adequate sight distances, and keeping the path surface continuous across the driveway entrance, so that it is clear that motorists are crossing an area

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where the path user has the right of way, among other measures. A TURNING VEHICLES YIELD TO PEDESTRIANS (R10-15) sign may be used to increase driver awareness at these and other appropriate locations.

- Consider design measures on approaches to intersections and driveways that encourage lower speeds for pathway approaches. There are a variety of measures that jurisdictions have used to encourage lower speeds, however it is important that these measures not present a hazard to pathway users, or cause the pathway to become inaccessible. This is another reason why in many cases it is important to accommodate bicycles on the roadway as well as the sidepath, so that bicyclists who prefer to travel at faster speeds may do so on the roadway.
- Employ measures on the parallel roadway (appropriate to the roadway function) to reduce speeds. These may include, among others, installation of raised medians, reduction of the number of travel lanes, and provision of on-street parking (configured so as to avoid restriction of sight lines at driveways).
- Design intersection crossings to facilitate bicycle access to and from the road or driveway that is being crossed, as this location represents an entry and exit point to the pathway.
- Keep approaches to intersections and major driveways clear of obstructions due to parked vehicles, shrubs, and signs on public or private property. Consider adding stop bars or yield markings for vehicles pulling up to the sidepath intersection.
- Design sidepath termini so as to facilitate proper vehicular operation of cyclists entering from or continuing on the roadway.

At signalized intersections, the pathway should be integrated into the controls of the intersection following the same principles as a pedestrian crossing. Care should be taken to avoid turning movements that will conflict with the “green” signal for the pathway. Solutions include prohibiting right turns on red, eliminating a phase which allows left turns on a green ball where it conflicts with the pedestrian signal, providing a leading pedestrian interval, and providing an exclusive pedestrian phase where there are high volumes of path users. Pedestrian countdown signal heads and accessible push buttons should be provided along with high visibility crosswalks, crossing islands at wide intersections, and sufficient space for queuing cyclists if high volumes of pathway users are expected.

As described above, in locations where the trail parallels a high speed roadway and crosses a minor road, it is advisable to move the crossing away from the intersection to a midblock location. By moving the crossing away from the intersection, motorists are able to exit the high speed roadway first, and then turn their attention to the pathway crossing.

Path users should never be given conflicting traffic control messages (e.g., with use of a STOP sign at a signalized intersection), leaving it unclear as to which device should be obeyed.

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5.3.5. OTHER INTERSECTION TREATMENTS**CURB RAMPS AND APRONS**

The opening of a shared use path at the roadway should be at least the same width as the shared use path itself. If a curb ramp is provided, the ramp should be the full width of the path, not including any side flares if utilized. The approach should provide a smooth and accessible transition between the path and the roadway. The ramp should be designed in accordance with the draft PROWAG. (1) Detectable warnings should be placed across the full width of the ramp. A 5-foot (1.5 m) radius or flare may be considered to facilitate turns for bicyclists.

Unpaved shared use paths should be provided with paved aprons extending a minimum of 20 feet (6 m) from paved road surfaces.

PATH WIDENING AT INTERSECTIONS

For locations where queuing at an intersection results in crowding at the roadway edge, consideration can be given to widening the path approach. This can increase the crossing capacity and help reduce conflicts at path entrances.

SHARED USE PATH CHICANES

Chicanes (i.e., horizontal curvature) can be designed to reduce path users' approach speeds at intersections where users must stop or yield, or where sight distance is limited. Care should be taken to end chicanes far enough in advance of the intersection to allow the user to focus on the curves in the pathway first, then the approaching intersection (rather than both at the same time). A solid centerline stripe is recommended at chicanes to reduce the instances of bicyclists "cutting the corners" of the curves. Chicanes should not be designed for speeds less than 8 mph (13 km/h).

RESTRICTING MOTOR VEHICLE TRAFFIC

Unauthorized access by motor vehicles is a problem on some pathways. In general, this is a greater issue on pathways that extend through independent corridors that are not visible from adjacent roads and properties. Per the MUTCD (3), the R5-3 NO MOTOR VEHICLES sign can be used to reinforce the rules.

The routine use of bollards and other similar barriers to restrict motor vehicle traffic is discouraged, unless there is a known history of use by unauthorized motor vehicles. Barriers such as bollards, fences, or other similar devices create permanent fixed object hazards to path users. Bollards on pathways are often struck by cyclists and other path users and can cause serious injury. Approaching riders may shield

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even a conspicuous bollard from a following rider's view until a point where he lacks sufficient time to react.

Furthermore, physical barriers are often ineffective at the job they were intended for – keeping out motorized traffic. People who are determined to use the path illegally will often find a way around the physical barrier, damaging path structures and adjacent vegetation. Barrier features can also slow access for emergency responders.

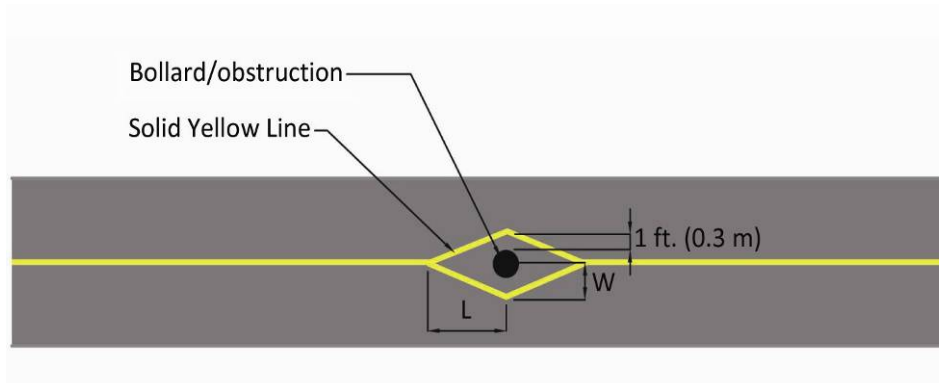
A preferred method of restricting entry of motor vehicles is to split the entry way into two sections separated by low landscaping. Each section should be half the nominal path width; for example a 10-foot (3 m) path should be split into two 5-foot (1.5 m) sections. Emergency vehicles can still enter if necessary by straddling the landscaping. Alternatively, it may be more appropriate to designate emergency vehicle access via protected access drives that can be secured. The approach to the split should be delineated with solid line pavement markings to guide the path user around the split.

Where the need for bollards or other vertical barriers in the pathway can be justified despite the hazard posed to cyclists, measures should be taken to ensure they are as safe as possible (13):

- Bollards should be marked with a retroreflectorized material on both sides or appropriate object markers, per Section 9B.26 of the MUTCD.
- Bollards should permit passage, without dismounting, for adult tricycles, bicycles towing trailers, and tandem bicycles. Bollards should not restrict access for people with disabilities. All users legally permitted to use the facility should be accommodated; failure to do so increases the likelihood that the bollards will be hazardous.
- Bollard placement should provide adequate sight distance to allow users to adjust their speed to avoid hitting them.
- Bollards should be a minimum height of 40 inches (1.0 m) and minimum diameter of 4 inches (100 mm). Some jurisdictions have used taller bollards that can be seen above users in order to reinforce their visibility.
- Striping an envelope around the approach to the post is recommended as shown in Exhibit 5.21 to guide path users around the object.
- One strategy is to use flexible delineators, which may reduce unauthorized vehicle access without causing the injuries that are common with rigid bollards.
- Bollards should only be installed in locations where vehicles cannot easily bypass the bollard. Use of one bollard in the center of the path is preferred. When more than one post is used, an odd number of posts at 6-foot (1.8 m) spacing is desirable. Two posts are not recommended, as they direct opposing path users towards the middle, creating conflict and the possibility of a head-on collision. Wider spacing can allow entry to motor vehicles, while narrower spacing might prevent entry by adult tricycles, wheelchair users, and bicycles with trailers.

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- Bollards should be set back from the roadway edge a minimum of 30 feet (10 m). Bollards set back from the intersection allow path users to complete their navigation of the potential hazard before approaching the roadway.
- Hardware installed in the ground to hold a bollard or post should be flush with the surface to avoid creating an additional safety hazard.
- Lockable, removable (or reclining) bollards allow entrance by authorized vehicles.

**Exhibit 5.21. Bollard Approach Markings****CROSSING ISLANDS**

Raised medians are associated with significantly lower pedestrian crash rates at multi-lane crossings. Although crossing islands (or medians) can be helpful on most road types, they are of particular benefit at path-roadway intersections in which one or more of the following apply: (1) high volumes of roadway traffic and/or speeds create difficult crossing conditions for path users; (2) roadway width is excessive given the available crossing time; or (3) the roadway is three or more lanes in width. In addition to improving bicycle safety, crossing islands benefit children, the elderly, people with disabilities, and others who travel slowly.

Crossing islands should be large enough to accommodate platoons of users, including groups of pedestrians and/or bicyclists, tandem bicycles (which are considerably longer than standard bicycles), wheelchairs, people with baby strollers and equestrians (if this is a permitted path use). The area may be designed with the storage aligned perpendicularly across the island or via a diagonal or offset storage bay (see example in Exhibit 5.22). The diagonal storage area has the added benefit of directing attention towards oncoming traffic, and should therefore be angled towards the direction from which traffic is approaching. Crossing islands should be designed in accordance with the draft Public Rights-of-Way Accessibility Guidelines (PROWAG). (1)

The minimum width of the storage area (shown as dimension “Y” in Exhibit 5.22) should be 6 feet (1.8 m), however 10 feet (3 m) is preferred in order to accommodate a bicycle with a trailer.

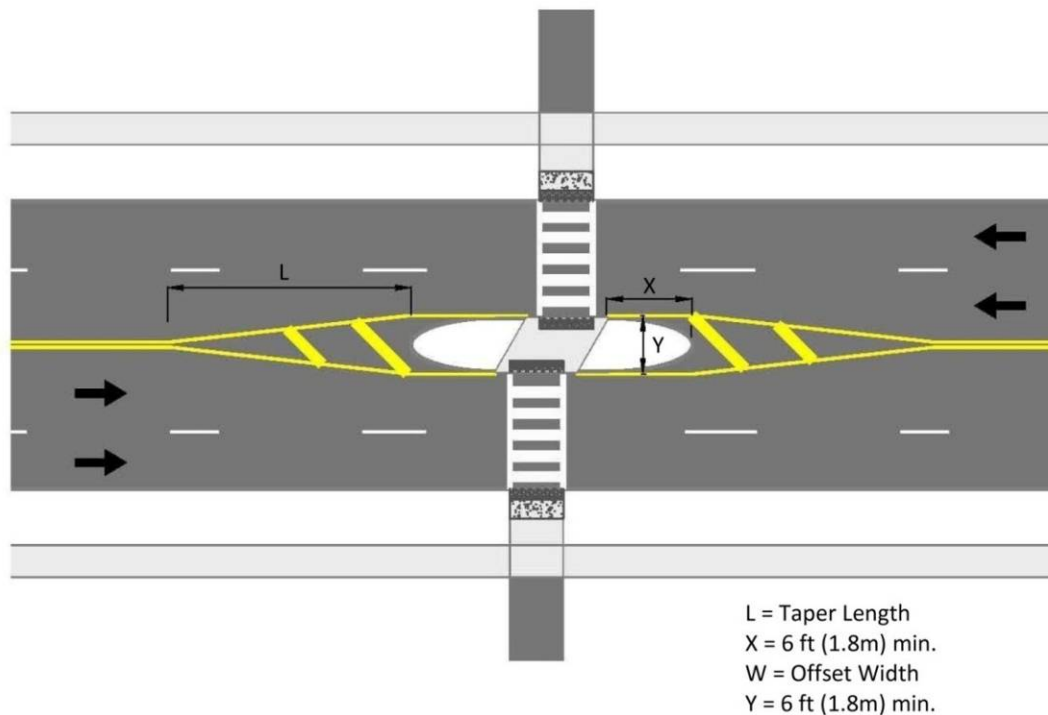
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Exhibit 5.22. Crossing Island (see Equation 5-8 to compute taper length)

US Customary	Metric
$L = \frac{WV^2}{60}, \quad \text{where } V \leq 45 \text{ mph}$ $L = WV, \quad \text{where } V \geq 45 \text{ mph}$	$L = \frac{WV^2}{155}, \quad \text{where } V \leq 70 \text{ km/h}$ $L = 0.62WV, \quad \text{where } V \geq 70 \text{ km/h}$
where: L = taper length (ft) W = offset width (ft) V = approach speed (mph)	where: L = taper length (m) W = offset width (m) V = approach speed (km/h)

Equation 5-8. Taper Length

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5.3.6. ADDITIONAL BICYCLE CROSSING CONSIDERATIONS**TRANSITION ZONES**

Where a shared use path crosses or terminates at an existing road, it is important to integrate the path into the existing system of on-road bicycle facilities and sidewalks. Care should be taken to properly design the terminus to transition the traffic into a safe merging or diverging situation. Appropriate signing is necessary to warn and direct both bicyclists and motorists regarding these transition areas.

Each roadway crossing is also an access point, and should therefore be designed to facilitate movements of path users who either enter the path from the road, or plan to exit the path and use the roadway.

TRAFFIC CALMING FOR INTERSECTIONS

At crossing locations where the speed of approaching roadway traffic is a concern, traffic calming measures may be helpful. These can include locations where roadway users are expected to yield to path users and sidepath crossings where motorists turn across the path. Slower motorist approach speeds can improve the ability of path users to judge gaps, improve motorists' preparedness to yield to path users at the crossing, and reduce the severity of injuries in the event of a collision.

Traffic calming measures that may be appropriate include a raised intersection or raised crosswalk, chicanes, curb extensions, speed cushions, crossing islands, and curb radius reduction at corners.

Traffic calming measures at path-roadway intersections should not be designed in a way that makes path access inconvenient or unsafe for bicyclists on the roadway who may wish to enter the path, or vice versa..

SHARED USE PATHS THROUGH INTERCHANGES

Where a shared use path is parallel to a roadway that intersects with a freeway, separation and continuity of the path should be provided. Users should not be required to exit the path, ride on roadways and/or sidewalks through the interchange, and then resume riding on a path.

At higher volume interchanges, a path may need grade-separated crossings to enable users to cross free-flow exit and entrance ramps with reasonable convenience and safety. An engineering analysis should be done to determine if grade separation is necessary. Away from ramps, paths can often be carried (with appropriate roadway separation or barrier) on the same structure that carries the parallel roadway through the interchange. See Section 5.2.10. for guidance on the design of structures.

DRAFT FOR AASHTO COMMITTEE REVIEW AND COMMENT**5.4. PAVEMENT MARKINGS, SIGNS, AND SIGNALS**

The MUTCD (3) regulates the design and use of all traffic control devices. Part 9 of the MUTCD presents standards and guidance for the design and use of signs, pavement markings, and signals that may be used to regulate, warn, and guide cyclists on roadways and pathways. Other parts of the MUTCD also include information relevant to shared use path operation and should be consulted as necessary.

5.4.1. PAVEMENT MARKINGS

Pavement markings can provide important guidance and information for path and roadway users. Pavement markings should be retroreflective. They should not be slippery or rise more than 0.16 in (4 mm) above the pavement.

MARKED CROSSWALKS

Marked crosswalks are recommended at intersections between shared use paths and roadways. They delineate the crossing location and can help alert roadway users to the potential conflict ahead. At a midblock location, no legally recognized crosswalk for pedestrians is present if no crosswalk is marked.

The use of high visibility (i.e. ladder or zebra) markings is recommended at shared use path crossings as these are more visible to approaching roadway users. More information on the installation of crosswalks at path-roadway intersections is provided in Section 5.3.2 of this chapter.

CENTERLINE STRIPING

A normal (4-6 inch or 100-150mm wide) yellow centerline stripe may be used to separate opposite directions of travel where passing is inadvisable. This stripe should be broken where there is adequate passing sight distance, and solid in locations where passing by path users should be discouraged. This may be particularly beneficial in the following circumstances: (1) for pathways with heavy user volumes; (2) on curves with restricted sight distance, or design speeds less than 14 mph (24 km/h); and (3) on unlit paths where nighttime riding is expected. The use of the broken centerline stripe may not be appropriate in parks or natural settings. However on paths where a centerline is not provided along the entire length of the path, appropriate locations for a solid centerline stripe should still be considered where described above.

A solid yellow centerline stripe may be used on the approach to intersections to discourage passing on the approach and departure of an intersection. If used, the centerline should be striped solid up to the stopping sight distance from edge of sidewalk (or roadway, if no sidewalk is present). A consistent approach to intersection striping can help to increase awareness of intersections.

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EDGE LINE STRIPING

The use of normal (4-6 inch or 100-150mm wide) white edge lines may be beneficial on shared use paths where bicycle traffic is expected during periods of darkness.

The use of white edge lines may be considered at approaches to intersections to alert path users of changing conditions. Where it is desirable to reduce path users' speed approaching an intersection, edge line stripes may be useful to create a perceived narrowing of the path.

If the pathway design includes a separate area for pedestrian travel, it should be separated from the bicycle travelway by a normal white line. Refer to Section 5.2.1 for more information on segregation of traffic.

APPROACH MARKINGS FOR OBSTRUCTIONS

Obstructions should not be located in the clear width of a path. Where an obstruction on the traveled portion must remain (for example in situations where bollards must be used), channelizing lines of appropriate color (yellow for centerline, otherwise white) should be used to guide path users around it. An example of a centerline treatment is given in Exhibit 5.21.

For obstructions located on the edge of the path, a solid white edgeline should be used. Approach markings should be tapered from the approach end of the obstruction to a point at least one foot (0.3 m) from the obstruction on each side, or only the inside side if the obstruction is at edge of pathway. The taper length should be a minimum length of 25 feet (7.6 m), and may be continuous for the length of the path where the obstruction exists.

PAVEMENT MARKINGS TO SUPPLEMENT INTERSECTION CONTROL

Stop and yield lines may be used to indicate the point at which a path user should stop or yield at a traffic control device. Design of stop and yield lines is described in Chapter 3B of the MUTCD. Stop or yield lines may be placed across the entire width of the path.

If used the stop or yield line should be placed a minimum of 2-feet (0.6 m) behind the nearest sidewalk or edge of roadway if a sidewalk is not present.

SUPPLEMENTAL PAVEMENT MARKINGS ON APPROACHES

Advanced pavement markings may be used on roadway or path approaches at crossings where the crossing is unexpected or where there is a history of crashes, conflicts, or complaints. If a supplemental word marking (such as HWY XING) is used, its leading edge should be located at or near the point where the approaching user passes the intersection warning sign or advance traffic control warning sign that the marking supplements. Additional markings may be placed closer to the crossing if needed, but

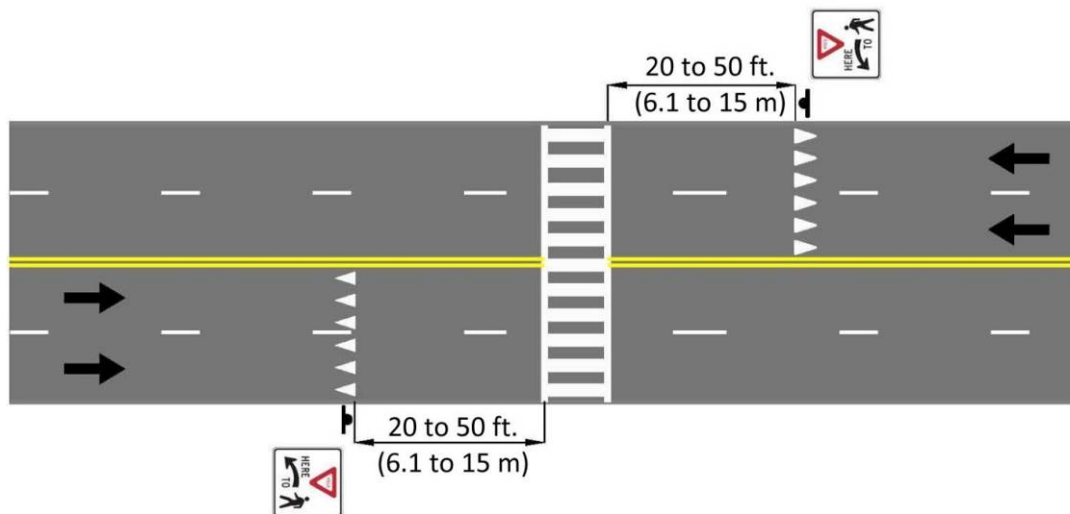
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1 should be at least 50 feet (15 m) from the crossing. Advanced pavement markings may be placed across
 2 the entire width of the path or within the approach lane. Pavement markings should not replace the
 3 appropriate signs.

4 Pavement markings may be word or symbols as described in Chapter 2 of the MUTCD (3).

5 ADVANCED STOP OR YIELD LINES

6 Advanced stop lines or yield lines should be used at multi-lane uncontrolled (or yield controlled)
 7 roadway approaches to a path crossing. The applicability of either a stop line or a yield line is governed
 8 by state law. Exhibit 5.23 shows an application of advanced yield lines. Advanced stop and yield lines
 9 reduce the potential for a multiple-threat crash between the path user and a vehicle. The advance stop
 10 or yield line provides a clearer field of vision between path users who are crossing the road and
 11 approaching vehicles in both lanes.



12

13 *Exhibit 5.23. Advanced Yield Signs and Markings*

14 5.4.2. SIGNS

15 All signs should be retroreflective and conform to the color, legend, and shape requirements described
 16 in the MUTCD. (3) Signs used along a path may be reduced in size per Table 9B-1 of the MUTCD. Signs
 17 utilized along a roadway which are visible to motorists should not be reduced in size and should
 18 conform to the sizes established in the MUTCD.

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1 Regulatory signs notify pathway (and roadway) users of location-specific regulations. Such a sign is
2 installed at or near the location where the regulation applies. Regulatory signs are generally rectangular
3 with white backgrounds and black text and symbols.

4 Warning signs are utilized to notify road and pathway users of unexpected conditions that might require
5 a reduction of speed or other action necessary for safety. A warning sign should be used, for example,
6 where pathway width must be reduced in a short section because of a constraint. However warning
7 signs should be used sparingly; use perceived as excessive or unnecessary can result in disrespect for
8 other important signs.

9 Warning signs are diamond shaped with black symbols and text. Permanent warning signs for bicycle
10 facilities should be standard yellow or fluorescent yellow-green (temporary warning signs should be
11 orange). In general, a uniform application of warning signs of the same color should be used.

12 For advance warning sign placements on shared use paths, the sign should be placed to allow adequate
13 perception-response time. The location of the sign should be based on the stopping sight distance
14 needed by the fastest expected path user; however, in no instance should the sign be located closer
15 than 100 feet (30 m) from the location warranting the advance warning. Warning signs should not be
16 placed too far in advance of the condition, such that path users tend to forget the warning because of
17 other distractions.

18 The purpose of guide and wayfinding signs is to inform path users of intersecting routes, direct them to
19 important destinations, and generally to give information that will help them along their way in the
20 most simple, direct manner possible. Guide signs are rectangular with green backgrounds and white
21 text.

22 SHARED USE PATH CROSSING WARNING SIGN ASSEMBLY

23 Roadway users may be warned of a shared use path crossing by utilizing a combined bicycle-pedestrian
24 warning sign (W11-15), per Exhibit 5.24, or a bicycle warning sign (W11-1). On a roadway approach to a
25 path crossing, placement of an intersection or advance traffic control warning sign should be at (or close
26 to) the distance recommended for the approach speed in Table 2C-4 of the MUTCD. (3) See Exhibits
27 5.17 through 5.20.

28 The assembly consists of a W11-15 or a W11-1 accompanied by a W16-7p (downward arrow) plaque
29 mounted below the warning sign. This sign should not be installed at the crossing if the roadway traffic
30 is yield-, stop-, or signal-controlled.

31 The W16-8P (path name) plaque may be mounted on the sign assembly (below the W11-15 or W11-1
32 sign) to notify approaching roadway users of the name of the shared use path being crossed.

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At path crossings that experience frequent conflicts between motorists and path users, or on multi-lane roadways where a sign on the right hand side of the roadway may not be visible to all travel lanes, an additional path crossing warning sign assembly should be installed on the opposite side of the road, or on the refuge island if there is one.

The combined bicycle-pedestrian warning sign (W11-15) or bicycle warning sign (W11-1) may be used in advance of shared use path crossings of roadways. Again, this warning sign should not be used in advance of locations where the roadway is stop-, yield-, or signal controlled. Advance warning sign assemblies may be supplemented with a W16-9p (AHEAD) plaque or W16-2P (XX FEET) plaque located below the W11-15P sign.

TRAFFIC CONTROL REGULATORY SIGNS

YIELD and STOP signs are used to assign priority at controlled but unsignalized path-roadway intersections. The choice of traffic control (if any) should be made with reference to the priority assignment guidance provided in Section 5.3.2 and in the MUTCD. The design and use of the signs is described in sections 2B and 9B of the MUTCD. (3)

INTERSECTION AND ADVANCE TRAFFIC CONTROL WARNING SIGNS

Advance traffic control warning signs announce the presence of a traffic control of the indicated type (YIELD, STOP, or signal) where the control itself is not visible for a sufficient distance on an approach for users to respond to the device. An intersection warning sign may be used in advance of an intersection to indicate the presence of the intersection and the possibility of turning or entering traffic.

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- On a shared use path approach, placement of an advance warning sign should be at a distance at least as great as the stopping sight distance of the fastest expected path user in advance of the location to which the sign applies. In no case should the advance placement distance be less than 50 feet (15 m). See Exhibits 5.17 through 5.20.
- An intersection or advance traffic control warning sign may carry a W16-8P (road or path name) plaque to identify the intersecting road or path, as appropriate for the approach.
- An advisory speed (W13-1) plaque may be added to the bottom of the sign assembly to advise the approaching user to the proper traveling speed for the available sight lines or geometric conditions.

GUIDE SIGNS

- Road name/path name signs (D3-1 and W16-8P) should be placed at all path-roadway crossings. This helps path users track their locations. At midblock crossings the D3-1 sign may be installed on the same post with a regulatory sign, above the STOP or YIELD sign.
- Guide signs to indicate directions, destinations, distances, route numbers, and names of crossing streets should be used in the same manner as on roadways and as described in Chapter 4.
- Reference location signs (also called mile markers) assist path users in estimating their progress, provide a means for identifying the location of emergency incidents, and are beneficial during maintenance activities. Section 9B-24 of the MUTCD provides guidance for the use of reference location signs.
- Where used, wayfinding signs for shared use paths should be implemented according to the principles discussed in Section 4.11. Mode-specific guide signs (D11-1a, D11-2, D11-3, and D11-4) may be used to guide different types of users to the traveled way that is intended for their respective modes (see Exhibit 5.25). If used, the signs should be installed at the point where the separate pathways diverge.
- (3)

23

**Exhibit 5.25. Mode-Specific Guide Signs**

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As discussed earlier in this chapter, it may be necessary to provide active warning or a traffic signal at some shared use path crossings of roadways. Guidance on this topic is provided in FHWA's *Safety Effects of Marked versus Unmarked Crosswalks at Uncontrolled Locations: Final Report and Recommended Guidelines* (10). The use of path user volume to determine the need for a signalized crossing may not be appropriate. In some situations the path user may not have access to another appropriate crossing location.

Signalized shared use path crossings should be operated to ensure the slowest user type likely to use the path will be accommodated. This will typically be the pedestrian.

For manually-operated signal actuation, the push button should be located in a position that is accessible from the path and in accordance with the draft PROWAG. (1) Bicyclists should not have to dismount to activate the signal. Part 9 of the MUTCD provides a variety of signs that are appropriate for these locations.

Another method of signal actuation is to provide automated detection (such as an inductive loop in the pavement); however, if the detection device is such that it does not detect pedestrians and other path users, it must be supplemented with a manually-activated signal. At signalized intersections on divided roadways, a push button should also be located in the median for those path users who may be trapped in the refuge area. Further discussion of signal design considerations is in Chapter 4.

Path crossing warning sign assemblies (W11-15) should not be used at a signal-controlled shared use path-roadway intersection.

In locations where motor vehicle traffic delay is a concern, a pedestrian hybrid beacon (popularly known as a HAWK--High-intensity Activated CrossWalk) may be considered, in accordance with MUTCD. (3) This signal is activated with a pushbutton. It controls traffic on the roadway by using a combination of red and yellow signal lenses, while the path approach is controlled by pedestrian signals.

A warning beacon is another type of crossing device that can be considered. A flashing warning beacon is a signal that displays flashing yellow indications to an approach. It is typically a single light, but can be installed in other combinations. A common application is to add a flashing amber signal to the top of a standard warning sign to bring attention to a shared use path crossing. The flashing signal may also be used on overhead signs at crosswalks. Flashing beacons are more effective if they only flash when path users are present, rather than flashing continuously, and therefore should be actuated by path users.

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WORKS CITED

1. **United States Access Board.** *Draft Public Rights-Of-Way Accessibility Guidelines*. Washington, D.C. : United States Access Board, 2005.
2. —. *Architectural Barriers Act Accessibility Guidelines for Outdoor Developed Areas*. Washington, D.C. : United States Access Board, 2007.
3. **Federal Highway Administration.** *Manual on Uniform Traffic Control Devices*. Washington, D.C. : Federal Highway Administration, U.S. Department of Transportation, 2009.
4. —. Shared Use Path Level of Service Calculator. [Online] 2006.
<http://www.tfhrc.gov/safety/pedbike/pubs/05138/>. FHWA-HRT-05-138.
5. —. *Shared Use Path Level of Service - A User's Guide*. 2006. FHWA-HRT-05-138.
6. **Landis, B.W, Petrisch, T.A and Huang, H.F.** *Characteristics of Emerging Road and Trail Users and their Safety*. McLean, VA : Federal Highway Administration, 2004.
7. **United States Access Board.** *Regulatory Negotiation Committee Recommendations for Accessibility Guidelines: Outdoor Developed Areas Final Report*. Washington, D.C. : United States Access Board, 1999.
8. **AASHTO.** *Roadway Lighting Design Guide*. Washington, D.C. : American Association of State Highway and Transportation Officials, 2005.
9. —. *Guide for the Planning, Design, and Operation of Pedestrian Facilities*. Washington, D.C. : American Association of State Highway and Transportation Officials, 2004.
10. **Zegeer, C.V., J.R. Stewart, H.H. Huang, P.A. Lagerwey, J. Feaganes and B.J. Campbell.** *Safety Effects of Marked Versus Unmarked Crosswalks at Uncontrolled Locations: Final Report and Recommended Guidelines*. s.l. : Federal Highway Administration, 2005. FHWA-HRT-04-100.
11. **AASHTO.** *A Policy on Geometric Design of Highways and Streets*. Washington, D.C. : American Association of State Highway and Transportation Officials, 2004.
12. *Sidepath Safety Model – Bicycle Sidepath Design Factors Affecting Crash Rates*. **Petritsch, T, et al.** s.l. : TRR Record, 2006, Vol. 1982/2006.
13. **Federal Highway Administration.** *DRAFT Accessibility Guidance for Bicycle and Pedestrian Facilities, Recreational Trails, and Transportation Enhancement Activities*. Washington, DC : Federal Highway Administration [On-line], 2008.

DRAFT FOR AASHTO COMMITTEE REVIEW AND COMMENT

- 1 14. **North Carolina Department of Transportation.** *North Carolina Bicycle Facilities Planning and Design*
2 *Guidelines*. Raleigh : North Carolina Department of Transportation, 1994.
- 3 15. **United States Access Board.** *Public Rights-Of-Way Accessibility Guidelines*. Washington, D.C. : United
4 States Access Board, 2005.
- 5 16. **Green, J.S, et al.** *Evaluation of Safety, Design, and Operation of Shared-Use Paths*. McLean, VA :
6 FHWA, 2006.

CHAPTER 6: BICYCLE PARKING FACILITIES

6.1. INTRODUCTION

Providing bicycle parking facilities is an essential element in a multi-modal transportation system. Unlike motor vehicles, most bicycles are not equipped with locks or anti-theft devices and do not require a key to operate. In addition, while they can be temporarily immobilized by locking a wheel to the frame, this does not prevent theft due to the bicycle's relatively light weight and small size.

In addition to helping prevent theft, installing well-designed parking in appropriate locations can contribute to a more orderly and aesthetic appearance of sidewalks and building sites. In the absence of bicycle parking or where parking facilities are inconveniently located, people may lock their bicycles to any stationary object such as a sign post, parking meter, fence, or tree. These randomly located bicycles may interfere with pedestrian movements or vehicular traffic flow, and make a sidewalk inaccessible to persons with disabilities. Providing bike parking can also be an inexpensive strategy to increase overall parking supply.

This chapter outlines recommendations for the planning and design of bicycle parking facilities that meet the needs of different types of bicycles and bicycle trips. Bicycle parking facilities should be provided at both the trip origin and trip destination. The wide variety of bicycle parking devices available is generally grouped into two classes, long-term and short-term. The needs for each differ in terms of their design and level of protection. In many locations, a combination of short- and long-term options may be appropriate.

6.2. PLANNING FOR BICYCLE PARKING

Bike parking facilities can be planned for and installed in a number of ways. Bicycle parking should be provided at all public facilities, should be incorporated into roadway and streetscape projects, and should be an integral aspect of land development and redevelopment processes. Many communities provide bike parking in the public right-of-way in response to requests from business owners or property managers. Consulting with local bicyclists can be an excellent way to determine where bike parking is needed.

Requiring bicycle parking in new development and redevelopment is a cost effective way to provide bike parking. Many communities have sought to increase the availability of bicycle parking through the local zoning and permitting process. One approach is to establish bicycle parking requirements relative to expected demand based on land use. Another approach is to require that bicycle parking spaces be

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provided in proportion (often 1:10) to the total number of automobile parking spaces. However, this approach can be problematic where there is a simultaneous effort to reduce motor vehicle parking and increase pedestrian and bicycle mode shares. The need for bicycle parking may increase over time so plans should anticipate this need for increased capacity.

Bicyclists will seek to park as close as possible to their final destination. Bicycle parking should therefore be conveniently placed in a location that is highly visible and as close to the building entrance as possible. In the event that directional signage is needed to indicate the location of bike parking, the MUTCD provides a sign that can be used for this purpose (see Exhibit 6.1). (1)



D4-3

Exhibit 6.1. Directional Signage for Bicycle Parking

Bicycle racks should be located so that they:

- Are easily accessed from the street and protected from motor vehicles.
- Are visible to passers-by to promote usage and enhance security.
- Do not impede or interfere with pedestrian traffic or routine maintenance activities.
- Do not block access to buildings, bus boarding or freight loading.
- Allow reasonable clearance for opening of passenger-side doors of parked cars.
- Are covered, if possible, where users will leave their bikes for a longer amount of time (see Section 6.4).

Bicycle parking requirements should be sufficiently detailed to address the design elements discussed in this chapter.

6.3. SHORT-TERM BICYCLE PARKING FACILITIES

Short-term parking facilities should be installed wherever people will need to leave their bicycle unattended for a short period of time. In general, bicycle parking should be considered wherever motor

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vehicle parking is provided and in areas where motor vehicle parking is not provided at individual properties, such as downtown areas or other high density locations.

Bicycle parking should be easy to locate and simple to use. Priority locations include stores, restaurants, apartment and condominium complexes, offices and public facilities such as transit stops, schools, parks and libraries. Two key components of successful short-term parking are location and facility design.

6.3.1. SITE DESIGN

When designing bike parking sites, it is important to consider the amount of space used by a fully occupied rack and the space necessary for bicyclists to access the parking area and use both sides of the rack. Below is a list of recommended dimensions for bike parking sites. Measurements should be made from an object to the nearest vertical component of rack.

DISTANCE TO OTHER RACKS

- Rack units aligned end to end should be placed a minimum of 96 inches (2.4 m) apart.
- Rack units aligned side by side should be placed a minimum of 36 inches (0.9 m) apart.

DISTANCE FROM A CURB

- Racks located perpendicular to a curb should be a minimum of 36 inches (0.9 m) from the back of curb.
- Racks located parallel to a curb should be a minimum of 24 inches (0.6 m) from the back of curb.

DISTANCE FROM A WALL

- Assuming access is needed from both sides, U-racks located perpendicular to a wall should be a minimum of 48 inches (1.2 m) from the wall.
- Racks located parallel to a wall should be a minimum of 36 inches (0.9 m) from the wall.

Well designed bike parking requires minimal maintenance. Damaged racks should be fixed or removed and replaced. Periodic removal of abandoned bikes and locks, especially at transit stations and universities, may be necessary. Abandoned bikes or bike wheels locked to racks reduce capacity and may discourage others from bicycling due to perceived risk of theft. Education may help reduce incorrect locking techniques and instruction for proper use may be placed on or near the rack. (2)

6.3.2. RACK DESIGN

One of the simplest, most effective type of short-term bike parking is the “inverted U” bike rack (see Exhibit 6.2). This rack supports the parking of two bikes simultaneously, one on each side of the rack, and can be grouped to provide additional spaces as needed. Some racks accommodate more than two bikes, although these facilities should be designed based on the principles listed below, to ensure capacity is not limited by incorrect use.

Racks should be constructed out of strong metal tubing and securely anchored to the ground unless the rack is of sufficient size and weight to prevent easy removal. If the rack is secured to a durable base, vandal- and theft-resistant hardware should be used. A crossbar (as shown in Figure 6.2) is recommended to prevent a bike from being stolen by knocking over the U-rack and slipping the lock over the end of the newly exposed post.

In all cases the parking area beneath the rack should be a concrete or asphalt surface and large enough to support bicycles locked to the rack.



Exhibit 6.2. Example of “Inverted U” Bicycle Rack (photo by Toole Design Group)

Bicycle racks should be designed so that they:

- Support the bicycle at two points above its center of gravity.
- Accommodate high security U-shaped bike locks.
- Accommodate locks securing the frame and one or both wheels (preferably without removing the front wheel from the bicycle.)
- Provide adequate distance [minimum 36 inches (0.9 m)] between spaces so that bicycles do not interfere with each other
- Do not contain protruding elements or sharp edges.

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- Do not bend wheels or damage other bicycle parts.
- Do not require the user to lift the bicycle off the ground. (2)

6.3.3. CONSIDERATIONS FOR SPECIAL TYPES OF RACKS**ART RACKS**

Artistically-inspired bike parking facilities can add a desirable element to a streetscape. If poorly designed, however, the facility may not provide the same degree of security or ease of use as other simpler designs and can contain protruding elements that may endanger pedestrians and other bicyclists. If used, artistically-inspired racks should be designed in accordance with all of the design and location guidelines described above.

WAVE RACKS

Wave racks or ribbon racks are not recommended. While they offer some perceived economic and aesthetic benefits, they are commonly used incorrectly and when used as intended do not provide adequate support or spacing.

SCHOOLYARD RACKS

Also referred to as “dish-rack” or “comb” style, these racks are not recommended and those still in use should be replaced. These racks are poorly designed as they support the bike only by the front wheel, which can bend the rim, and they do not support proper locking and thus provide inadequate theft prevention to the user.

6.4. LONG-TERM BICYCLE PARKING FACILITIES

Long-term bicycle parking facilities should provide a high degree of security and protection from the weather. They are intended for situations where the bicycle is left unattended for long periods of time, such as apartments and condominium complexes, schools, places of employment and transit stops. The simplest type of long-term parking is a structure that covers a bike parking area and offers sufficient protection from the elements. Long-term bicycle parking facilities can also include lockers, monitored bike parking areas, cages, or a dedicated space or room within a building or a parking garage. Long-term parking facilities should be well lit and accessible to provide a high degree of personal security. Signs may be necessary to direct bicyclists to long term parking.

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1 Bicycle lockers are lockable, self-contained units that can store an individual bicycle and related
2 accessories and provide a high level of security. They should be constructed from a strong, weather
3 resistant and maintenance-free material. Most bicycle locker systems require user registration and
4 public agency administration and maintenance. The effective capacity of lockers may be somewhat
5 limited as parking is only available to the registered individual. Some transit agencies are exploring the
6 use of smart cards to reduce management costs and increase security and availability. Homeland
7 security concerns should be also taken in to account and racks may be required to include a transparent
8 element to detect inappropriate use. The siting of lockers in public spaces should also be carefully
9 considered to minimize negative impacts.

10 Another strategy for long term parking is to create an access-controlled space that contains racks for
11 support and locking of individual bikes. If located outdoors, the space should be covered and well light.
12 Creating an indoor bike room is an option for residential and employment centers. Bike rooms should be
13 easy to access and if not located on the ground floor should be accessible by elevator. Rooms and cages
14 should include racks that are designed and sited according the recommendations for short term parking.

15 The use of two-tiered racks can provide increased parking capacity in areas with limited space
16 availability. Consider providing a mechanism to assist the user in lifting their bicycle onto the second
17 tier.

18 It is important that people be able to securely lock their bicycles as theft can be a problem in shared
19 spaces. Rooms should be designed so that when racks are occupied sufficient space is available in
20 between racks to access parked bicycles. If no space is available, buildings may still provide a long-term
21 parking option by permitting employees to bring their bicycles into their personal work space.

22 Some transit agencies provide staffed bicycle parking areas which offer valet parking to customers.
23 Some communities have created dedicated bicycle parking structures offering a range of amenities
24 including showers and lockers and bicycle repair service. These can provide excellent support for
25 bicycling within a community and have been very successful in areas with high levels of bicycle use. (2)

1 WORKS CITED

- 2 1. **Federal Highway Administration.** *Manual on Uniform Traffic Control Devices*. Washington, D.C. :
3 Federal Highway Administration, U.S. Department of Transportation, 2009.
- 4 2. **Association of Pedestrian and Bicycle Professionals.** *Bicycle Parking Guidelines*. Washington, DC : s.n.,
5 2002.

CHAPTER 7: MAINTENANCE AND OPERATIONS

7.1. INTRODUCTION

Bikeways are subject to surface deterioration and debris accumulation, and require maintenance to function well. Poorly maintained facilities become unusable and are hazardous; they can cause equipment damage and physical injury to bicyclists.

What may be an adequate roadway surface for automobiles can cause difficulties for cyclists who ride on narrow, high-pressure tires. Uneven longitudinal cracks and joints can divert a bicycle wheel. Gravel blown off the travel lane by traffic often accumulates in the area where bicyclists ride. Small rocks, branches and other debris can deflect a wheel, and potholes can cause wheel rims to bend, leading to spills. An accumulation of leaves can hide a pothole. Broken glass can puncture bicycle tires. A good maintenance program protects public funds invested in bikeways, so they can continue to be used safely.

7.2. RECOMMENDED MAINTENANCE PROGRAMS AND ACTIVITIES

A bikeway maintenance program is necessary to ensure adequate maintenance of facilities. Sufficient funds should be budgeted to accomplish the necessary tasks. Neighboring jurisdictions can consider joint programs for greater efficiency and reduced cost. The program should establish maintenance standards and a schedule for inspections and maintenance activities recommended below.

Road users are usually the first to experience deficiencies. Spot-improvement programs enable bicyclists to bring problems to the attention of authorities in a quick and efficient manner. An online complaint/comment submission form facilitates public input about bikeway maintenance problems. Many jurisdictions have maintenance reporting systems that can be expanded to include requests from bicyclists. Quick response from the responsible agency improves communications between the public and staff.

7.2.1. SWEEPING

Bicyclists often avoid shoulders and bike lanes filled with gravel, broken glass and other debris. Regularly scheduled maintenance helps to ensure that litter on the traveled way is regularly swept. Debris from the roadway should not be swept onto sidewalks; nor should debris from sidewalks be swept onto the roadway.

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Shared use paths can also accumulate debris that can cause difficulties for bicyclists. This is especially true for paths that are located in coastal areas, paths that extend through wooded areas, and paths along waterways that overflow during storm events.

Some jurisdictions use sand or gravel to treat roadways during snow events or icy conditions. These treatments degrade conditions for bicycling in addition to causing problems such as clogged storm drains and other long term infrastructure maintenance issues. Salt products that are more environmentally-friendly should be explored, rather than using sand or gravel. Jurisdictions that use sand or gravel should sweep bikeways as soon as practical, particularly after major storm events.

The following recommendations can help to alleviate bicycle hazards caused by debris:

- Establish a regular sweeping schedule for roadways and pathways that anticipates both routine and special sweeping needs. This may involve more frequent sweeping seasonally, and also should include periodic inspection, particularly in areas that experience frequent flooding, or in areas that have frequent vandalism. The sweeping program should be designed to respond to user requests for sweeping activities.
- Remove debris in curbed sections with maintenance vehicles that pick up the debris; on roads with flush shoulders, debris can be swept off the pavement.
- Prevent problems by paving gravel driveway approaches to reduce loose gravel on paved roadway shoulders. Also require parties responsible for debris to contain it; for example, require tarps on trucks loaded with gravel. Local ordinances often require tow-vehicle operators to remove glass after crashes, and contractors are usually required to clean up daily after construction operations that leave gravel and dirt on the roadway.

7.2.2. SURFACE REPAIRS

Cracks, potholes, bumps, and other surface defects can degrade bicycling conditions. The following recommendations apply:

- Inspect bikeways regularly for surface irregularities; after noticing or receiving notice of a surface irregularity, repair potentially hazardous conditions promptly.
- Establish a process that enables the responsible agency to respond to user complaints in a timely manner.
- Prevent the edge of a surface repair from running longitudinally through a bike lane or shoulder.
- Perform preventative maintenance periodically, such as keeping drains in operating condition and eliminating intrusive tree roots.
- Sweep a project area after repairs.

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- Develop a pavement preservation program for bikeways to minimize deterioration and cracking.
- Prevent long-term problems by building bikeways, especially paths, to a high pavement standard so they last a long time without requiring significant maintenance or expensive repair. This could include selecting a pavement material that is resistant to root damage, or selectively placing root barriers in locations where root damage is expected to be a problem.

7.2.3. PAVEMENT OVERLAYS

Pavement overlays are good opportunities to improve conditions for cyclists if done carefully: a ridge should not be left in the area where bicyclists ride (this occurs when an overlay extends part-way into a shoulder bikeway or bike lane). Overlay projects offer opportunities to widen the roadway, or to restripe the roadway with bike lanes (see Chapter 4). The following recommendations can help to ensure that pavement overlays are compatible with bicycle travel:

- Extend the overlay over the entire roadway surface, including shoulder bikeways and bike lanes, to avoid leaving an abrupt edge within the riding area. If the surface conditions are acceptable on the shoulder or bike lane, the pavement overlay can stop at the shoulder or bike lane stripe, provided no abrupt ridge remains at the stripe.
- Ensure there is no sudden drop off at the edge of pavement.
- During overlay projects, ensure that the surface of inlet grates and utility covers are maintained to within ¼ inch (6 mm) of the pavement surface (raise if necessary), and replace any that are not bicycle-friendly with those that are (see Section 4.12.8).
- Pave at least 10 feet (3 m) back on (low-volume) driveway connections, and 30 feet (9.1 m) or to the right-of-way line, whichever is less, on unpaved public road connections, can prevent gravel from spilling onto shoulders or bike lanes.
- Sweep the project area after overlay to prevent loose gravel from adhering to the freshly paved shoulder or bike lane.

7.2.4. VEGETATION

Vegetation encroaching into bikeways can be a serious hazard. Roots should be controlled to prevent surface breakup as they can undermine a path surface and create hazards for all users. Adequate clearances and sight distances should be maintained at driveways and intersections. Bicyclists should be visible to approaching motorists, not hidden by overgrown shrubs or low hanging branches, which can also obscure signs. The following recommendations apply to vegetation control and removal:

- Cut back vegetation to prevent encroachment.

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- Cut back intrusive tree roots and install root barriers where appropriate.
- Adopt local ordinances to require adjacent landowners to control vegetation and/or allow road authorities to control vegetation that originates from private property.

7.2.5. TRAFFIC SIGNAL DETECTORS

Repairs and modifications to traffic signals offer opportunities to improve their functionality for bicyclists. At traffic signals with detectors, check that a typical bicycle can trigger a response when no other vehicles are waiting at the light. The following recommendations can help to ensure traffic signals are bicycle compatible:

- Adjust detector sensitivity so the signal can be actuated by a typical bicycle.
- Place a stencil over the most sensitive part of the detector to notify cyclists where to wait to trigger a green light. (1)
- Adjust the signal phases to account for the speed of a typical bicyclist. See Chapter 4 for additional guidance on other detection technologies and evaluation and improvement of signal timing for bicycles.

7.2.6. SIGNS AND MARKINGS

New bikeway signs and markings are highly visible, but over time signs may fall into disrepair and markings may become hard to see, especially at night. Signs and markings should be kept in a readable condition, including those directed at motorists. Bicyclists depend on motorists observing the signs and markings that regulate their movements (e.g. stop signs and stop lines). The following recommendations apply to signs and markings:

- Inspect signs and markings regularly, including retroreflectivity at night.
- Replace defective or damaged signs as soon as practical.
- Replace symbol markings as needed; in high use areas this may require replacement more than once a year.

7.2.7. DRAINAGE IMPROVEMENTS

Drainage facilities often deteriorate over time. Catch basins may need to be adjusted in height or replaced to improve drainage. A bicycle-safe drainage grate flush with the pavement reduces jarring bumps that can cause loss of control. Curbs used to divert storm water into catch basins should be

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designed so they do not create a hazard for cyclists. The following recommendations apply to drainage improvements for bicycles:

- Reset catch basin grates flush with pavement.
- Modify or replace deficient drainage grates with bicycle-safe grates. A policy for replacing unsafe drainage grates during resurfacing and reconstruction is one way to accomplish this task over time.
- Repair or relocate faulty drainage at intersections where water backs up in the gutter.
- Adjust or relocate existing drainage curbs that encroach into shoulders or bike lanes.

7.2.8. CHIP SEALING

Chip seals leave a rough surface for bicycling and are strongly discouraged. Chip seals that cover the travel way and part of the shoulder area leave a ragged edge or ridge in the shoulder, degrading conditions for bicyclists. The following recommendations apply:

- Where a chip seal must be used on a roadway shared with bicyclists, use a fine mix chip seal [3/8 inch (10 mm) or finer].
- Where shoulders or bike lanes are wide enough and in good repair, apply the chip seal only to the main traveled way.
- If the shoulders or bike lanes must be chip sealed, cover the shoulder area with a well rolled, fine-textured material: 3/8 inch (10 mm) or finer for single pass, 1/4 inch (6 mm) for second pass.
- Sweep the shoulder area following chip seal operations.
- Chip seal should not be used on shared use paths.

7.2.9. PATCHING ACTIVITIES

Road graders can provide a smooth pavement patch; however, the last pass of the grader sometimes leaves a rough tire track in the middle of the shoulder or area where bicyclists ride. Loose asphalt may at times collect on the shoulder, adhering to the freshly paved surface. The following recommendations apply:

- Equip road graders with smooth tires where practicable.
- Do not place patch partway into shoulder: stop the patch at the edge of roadway, or cover the entire shoulder width.
- Roll the shoulder area after the last pass of the grader.
- Sweep loose materials off the roadway before they adhere to the fresh pavement.

DRAFT FOR AASHTO COMMITTEE REVIEW AND COMMENT**7.2.10. UTILITY CUTS**

Utility cuts can leave a rough surface for cyclists if not back-filled carefully and fully compacted. Utility cuts should be finished as smooth as new pavement. The following recommendations apply:

- Wherever feasible, place cut line in an area that will not interfere with bicycle travel, and ensure that cuts parallel to bicycle traffic don't leave a ridge or groove in the bicycle wheel track area.
- Back fill cuts in bikeways flush with the surface (humps will not get packed down by bicycle traffic).
- Ensure proper compaction to reduce or eliminate later settlement.

7.2.11. SNOW CLEARANCE

Many bicyclists ride year-round, especially for utilitarian or commute trips. Snow stored in bike lanes impedes bicycling in winter. The following recommendations apply:

- On streets with bike lanes and paved shoulders that are used by bicyclists, remove snow from all travel lanes (including bike lanes) and the shoulder, where feasible.
- Do not store snow on sidewalks where it will impede pedestrian traffic.
- Snow may be stored on sidewalk street furniture zones or landscape strips where there is sufficient width.
- Remove snow from shared use paths that are regularly used by commuters, unless there is a desire to use the facility for cross-country skiing.

7.3. OPERATING BIKEWAYS IN WORK ZONES

Transportation construction projects often disrupt the public's mobility and access. Proper planning for bicyclists through and along construction areas is as important as planning for motor vehicle traffic, especially in urban and suburban areas. The MUTCD states that the "needs and control of all road users (motorists, bicyclists, and pedestrians) through a temporary traffic control zone shall be an essential part of highway construction, utility work, maintenance operations, and the management of traffic incidents." (1) Temporary lane restrictions, detours and other traffic control measures should therefore be designed to accommodate bicyclists. The recommendations below should be incorporated into project construction plans. Workers who routinely perform maintenance and construction operations should be aware of these considerations.

Plans for the maintenance of bicycle travel should be initiated whenever the need for temporary traffic controls is being considered. At the onset of planning for temporary traffic controls, it should be

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determined how existing bicycle facilities will be maintained during construction. Options include accommodating bicycles through the work zone or providing a detour route.

Similar to other vehicular traffic, bicyclists should be protected from work zone hazards. Hazards may include road or path closures, sudden changes in elevation, construction equipment or materials, and other unexpected conditions. Accommodation in the work zone may result in the need for the construction of temporary facilities including paved surfaces, structures, signs and signals. The MUTCD includes appropriate mode-specific detour guidelines in the section on temporary traffic controls (1). Where guidelines do not adequately cover a situation specific to bicycle use, general vehicular guidelines should be applied.

7.3.1. RURAL HIGHWAY CONSTRUCTION

Construction operations on rural highways can affect long-distance commuter, touring and recreational cyclists. On low-volume roads, or through short construction zones, standard traffic control practices are usually adequate. Bicyclists are provided with access as long as a smooth, paved surface is maintained, and temporary signs, debris and other obstructions are removed from the edge of the roadway after each day's work.

On high-volume roads or through long construction zones, adequate paved roadway width should be provided for motor vehicles to safely pass bicyclists. Flaggers and pilot cars should take into account the bicyclists' lower speed. Radio messages can be relayed to other flaggers if bicyclists are coming through at the end of a platoon. On highways with very high traffic volumes and speeds, and where construction will restrict available width for a long time, a detour route should be provided for bicyclists where possible. The detour should not be overly circuitous, and M4-9 detour signs should guide cyclists along the route and back to the highway. (1)

7.3.2. URBAN ROADWAY CONSTRUCTION

In urban areas, safe and convenient passage is needed during construction for bicyclists. If a detour requires significant out-of-direction travel, the bicyclist will prefer to ride through the construction zone. It is preferable to create a passage that allows bicyclists to proceed as close to their normal route as possible. Accommodation within the construction zone is preferred. Closing a bikeway or installing signs asking cyclists to take a detour is usually ineffective, as bicyclists can share a lane over a short distance. Detour routes that require bicyclists to make two left turns across heavy traffic are also discouraged and such situations may require providing two detours, one for each direction of travel.

On longer projects, and on busy roadways, a temporary bike lane or wide outside lane may be provided. Bicyclists should not be routed onto sidewalks or onto unpaved shoulders. Debris should be swept to

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- 1 maintain a reasonably smooth and clean riding surface in the outer few feet of roadway. Advance
- 2 construction signs should not obstruct the bicyclist's path. Signs should be placed in a buffer/planter
- 3 strip, rather than in a bike lane or on a sidewalk. Where this is not possible, either raising the sign, or
- 4 placing signs half on the sidewalk and half on the roadway may be the best solution. Bike lanes and
- 5 sidewalks should not be used for storage of construction signs or materials when work is halted for the
- 6 day.

1 **WORKS CITED**

- 2 1. **Federal Highway Administration.** *Manual on Uniform Traffic Control Devices*. Washington, D.C. :
3 Federal Highway Administration, U.S. Department of Transportation, 2009.
- 4 2. *Protection of Nonmotorized Transportation Traffic*. 23 U.S.C. 109(m),